

Teck Highland Valley Copper Partnership

2017 Dam Safety Inspection Report

Highland Tailings Storage Facility





March 29, 2018

Teck Highland Valley Copper Partnership PO Box 1500 Logan Lake, British Columbia VOK 1W0

Mr. Chris Anderson Superintendent, Tailings and Water Management

Dear Mr. Anderson:

2017 Dam Safety Inspection
Highland Tailings Storage Facility

We are pleased to submit the 2017 Dam Safety Inspection report for the Highland Tailings Storage Facility. The inspection and this report were prepared to comply with Section 10.5.3 of the Health, Safety and Reclamation Code for Mines in British Columbia (the Code), Section 4.2 "Annual Tailings Facility and Dam Safety Inspection Report" of the Code Guidance Document.

Yours truly,

KLOHN CRIPPEN BERGER LTD.

Bill Chin, P.Eng. Engineer of Record

Senior Geotechnical Engineer, Principal

CW/EH:cd



Teck Highland Valley Copper Partnership

2017 Dam Safety Inspection Report

Highland Tailings Storage Facility

EXECUTIVE SUMMARY

Klohn Crippen Berger Ltd. (KCB) were engaged by Teck Highland Valley Copper Partnership (THVCP) to complete the 2017 Dam Safety Inspection (DSI) of the Highland Tailings Storage Facility (TSF) on the Highland Valley Copper (HVC) mine site in accordance with requirement of the Health, Safety and Reclamation Code for Mines in British Columbia (the Code). The visual inspection was completed by the Engineer of Record (EoR), Mr. Bill Chin, P.Eng., as a representative of KCB on September 27, 2017. Mr. Chris Anderson, P. Eng., THVCP Tailings and Water Superintendent, is the TSF Qualified Person (as defined by the Code) for the Highland TSF.

The DSI includes the L-L Dam and H-H Dam, three seepage collection ponds at the toe of the L-L Dam (Seepage Pond 1, Seepage Pond 2, Seepage Water Reclaim Pond), two active slimes ponds at the toe of the L-L Dam (Slimes Pond 1 and Slimes Pond 2), and 24 Mile Lake.

The HVC site is located near Logan Lake, approximately 45 km south of Kamloops, in the interior of British Columbia. The Highland TSF is located approximately 6.5 km northwest of the operating mill, refer to Figure 1. Tailings from ongoing mining operations are discharged into the Highland TSF. Tailings are retained in the impoundment by two containment dams (L-L Dam and H-H Dam) which are incrementally raised to meet tailings storage requirements, typically on an annual basis. 24 Mile Lake is an active containment facility which receives emergency discharge of water and tailings from the H-H Pumphouse during tailings line maintenance and operational upsets.

The L-L Dam comprises an earthfill starter dam raised via the centerline method with a low permeability vertical glacial till core supported by a downstream zone of hydraulically placed, compacted cycloned sand and an upstream cycloned sand berm/tailings beach. A sand and gravel drainage blanket extends beneath the downstream shell of the dam, with granular finger drains at selected locations which convey flow into a series of seepage recovery ponds at the toe of the dam.

The H-H Dam comprises an earthfill starter dam raised via the centerline method with a low permeability centreline vertical glacial till core, supported by random fill and tailings beach on the upstream side and variable mine waste fill on the downstream side.

24 Mile Lake is a former natural lake surrounded by waste dumps that have been placed over the life of the mine.

The L-L Dam has been assigned an "Extreme" consequence category and the H-H Dam a "Very High" consequence category, as defined by CDA (2013). The downstream seepage dams at the toe of the L-L Dam have been assigned a "Significant" consequence category. 24 Mile Lake has been assigned a "Low" consequence category, as defined by CDA (2013). There were no significant changes to the key geotechnical or hydrotechnical hazards during 2017. The most recent dam safety review (DSR) of the Highland TSF was completed by Stantec in 2017 (Stantec 2018). The Code requires a DSR be conducted at a minimum every five years for TSFs, irrespective of dam consequence classification (MEM 2016). Therefore, the next DSR is scheduled for 2022.

The Highland TSF free water pond is maintained at the western end of the impoundment. The water level in the pond varied by 6.6 m in 2017, which is greater than historical trends over the past seven



years, largely due to more inflow from a wetter freshet. THVCP manages and tracks the annual water balance for the Highland TSF. In 2017, a net gain in the TSF pond volume was estimated based on the water balance. The Highland TSF has no spillway and is designed to store the environmental design flood (EDF) and inflow design flood (IDF), which for this facility is the probable maximum flood (PMF). 24 Mile Lake can also store the IDF (100-year return period event). All of the L-L Dam seepage and slimes ponds, except the Seepage Water Reclaim Pond and Seepage Pond 2, are designed to safely route or store, with appropriate freeboard, storm events greater than or equal to the IDF required by the Code (MEM 2016). Agreed actions to bring the Seepage Water Reclaim Pond and Seepage Pond 2 back into compliance are underway and are listed in Table 2.

The Operation, Maintenance and Surveillance (OMS) manual was updated in 2018. The manual meets the intent of the Mining Association of Canada (MAC 2011) and CDA (2014) guidelines, is current and provides adequate coverage for existing conditions. An update for the Emergency Preparedness and Response Plan (EPRP) is ongoing.

Visual inspections and instrument measurements were completed by THVCP in general accordance with the prescribed frequencies during periods of the year when instrument sites are accessible. There were two event-driven inspections of the L-L Dam in 2017 in response to a pipe rupture at the cyclone house and to flooding during freshet. There were four piezometers and no inclinometer threshold exceedances noted at the L-L Dam during 2017. None of the piezometer threshold exceedances were considered a dam safety concern. There were three piezometers and two inclinometer threshold exceedances noted at the H-H Dam in 2017. These exceedances led to increased reading frequencies and review of the instrumentation data in relation to further fill placement but were not considered a dam safety concern. Further details of the actions taken are presented in Sections 5.4 and 5.5 of the report. Revised piezometric and movement thresholds have been set for 2018 to monitor deviation from the established trend. During 2017, an additional 60 vibrating wire piezometers, 15 standpipe piezometers and 11 inclinometers were installed at the L-L Dam. At the H-H Dam, an additional 26 vibrating wire piezometers, 2 standpipe piezometers, 6 Sondex settlement systems and 6 inclinometers were installed. Ten survey monuments were installed at the lock-block retaining wall upslope of the H-H Pumphouse.

Water quality downstream of the Highland TSF is monitored by HVC monthly to assess the effectiveness of the tailings facility in protecting the downstream receiving environment (ERM 2018). All permit sampling requirements and frequency were met in 2017. THVCP have implemented the Sulphate Adaptive Management Plan to mitigate influence of seepage from the Highland TSF on downstream water quality and increase water available to downstream users in Pukaist creek. This is a multi-year program which includes installation of interception wells downstream of the dam, lining of seepage collection ponds and diversion of non-contact water around the TSF and into Pukaist Creek.

The Highland TSF appeared to be in good physical condition and the observed performance during the 2017 site inspections was consistent with the expected design conditions and past performance. The status of recommendations to address deficiencies and non-conformances identified during past



DSIs are summarized in Table 1. Closed recommendations are shown in *italics*. Recommendations to address deficiencies and non-conformances identified during the 2017 DSI are summarized in Table 2.

As shown, most of the recommendations have either been completed or are included in the planned 2018 dam construction activities. For the purposes of the DSI, these recommendations are considered closed. The only outstanding recommendations are associated with upgrading the Seepage Water Reclaim Pond and the Seepage Pond 2 to bring them back into compliance with management of the IDF and freeboard, as listed in Table 2.

Table 1 Previous DSI Recommendations – Status Update

| ID No. | Recommended Action | Priority ⁽¹⁾ | Recommended Deadline (Status) |
|------------|--|-------------------------|--|
| | L-L Dam | | |
| LL-2015-01 | Downstream sand should be placed in over-steepened upper sections of the downstream dam slopes (between Sta. 1+050 m and Sta. 3+650 m) to the design (2.5H:1V) during the next construction season. Some re-work of the existing loose sand on these slopes will be required as the downstream sand is raised, to ensure these slopes meet compaction/density requirements. | 3 | End of next construction season (Outstanding. Included in construction sequence for next dam raise (2017 to 2018). EoR has reviewed stability and found acceptable factor of safety and no interim dam safety concern.) |
| LL-2015-02 | The overflow lines from the secondary cyclones were buried with un-compacted cyclone sand across the upstream S2 zone during 2015. This sand will require re-working following completion of downstream sand placement during the next construction season. | 3 | End of next construction season (Outstanding. Some of the lines were removed and trenches re-filled with S2 fill during 2017 (refer to section 3.2.2 for details). Remaining will be done as part of ongoing construction) |
| LL-2015-04 | It was noted in the Q4 dam inspection that the north side of the NBB El. 1202 m bench is over-steepened to 1V:1H. This should be re-graded to the design slope gradient to prevent instability. | 4 | During next construction season (CLOSED. This over steepened area is related to an access road cut in one of the benches and potential instability relates to local area and is not a dam safety issue. Area will be remediated during future construction works. EoR has reviewed stability and found acceptable factor of safety and no interim dam safety concern.) |
| LL-2015-06 | We recommend that key design criteria for the Highland TSF, including the probable maximum flood (PMF), freeboard requirements and seismic ground motions be reviewed during 2016. | 4 | December 2016 (Outstanding. Deferred to 2018. There is no interim dam safety concern.) |
| LL-2016-01 | Repair P10-4 to prevent inflow of surface water (suspected cause of elevated groundwater readings). Remove metal cover and investigate possible location where surface water could be entering the standpipe. If the original grout backfill to the hole has sunk, then top up with bentonite or cement to seal the standpipe. Ensure ground is sloped away from the piezo to prevent any ponding water. | 4 | May 1 2017 (CLOSED) |
| LL-2016-02 | Install seepage measurement weirs at the outflow points of the North Dam (Sta. 3+600) and South Dam (Sta. 1+100) major drains to monitor seepage rates. | 3 | December 2017 (Outstanding. Deferred until 2018 when proposed North Dam drain construction is completed) |

| ID No. | Recommended Action | Priority ⁽¹⁾ | Recommended Deadline (Status) |
|----------------|--|-------------------------|---|
| | H-H Dam | | |
| НН-2015- 01 | H-H lock-block wall - during operations routine monitoring of the retaining wall condition should be conducted. This includes an annual survey at selected points along the crest and toe of the wall, and monthly photographs taken from the same location to monitor for signs of changes to the wall alignment. | 3 | Ongoing throughout 2016 (CLOSED) |
| НН-2015- 02 | H-H lock-block wall - a piezometer should be installed behind or at the toe of the retaining wall to monitor water levels behind the retaining wall as this can affect the stability of the dam near the retaining wall | 3 | December 2016 (CLOSED, VWP installed in 2017) |
| HH-2016- 01 | H-H lock-block wall - during operations routine monitoring of the retaining wall condition should be conducted. This includes an annual survey at selected points along the crest and toe of the wall. | 3 | May 2017 (CLOSED, duplicate of HH-2015-01) |
| | 24 Mile Lake | | |
| | None. | | |
| | L-L Dam Seepage Collection and Slir | nes Ponds | |
| SP-2016-01 | In order to meet the design freeboard requirement at Seepage Pond 2, the following is recommended: Iower the existing inlet invert elevation by 0.4 m (from El. 1114.4 m to El. 1114.0 m); re-align the pipe such that the bend after the first 12 m (currently graded upwards) is removed; and maintain or lower the existing outlet invert elevation to achieve a minimum grade of 1%. | 2 | July 2017 (CLOSED, superseded by SP-2017-02) |
| SP-2016-02 | To meet the design freeboard requirement at the Seepage Water Reclaim Pond, review the following items as part of planning and prior to construction activities in 2017: the water quality and discharge requirements to determine an appropriate environmental design flood (EDF) and explore options for discharge during flood events, if permissible; the anticipated timeline for construction and operation of the proposed Slimes Pond 3 which was modeled, as designed, to discharge to Seepage Water Reclaim Pond under flood conditions; review the current and proposed operational requirements for pumping (in relation to capacity and operational elevations); and update, if necessary, the 2015 flood routing assessment based on revised assumptions. | 2 | July 2017 (CLOSED) |
| SP-2016-03 | If required, upgrade the water management plan for the Seepage Water Reclaim Pond, based on the updated flood routing assessment (action item SP-2016-02). | 2 | December 2017 (CLOSED, superseded by SP-2017-01) |
| SP-2016-04 | Repair the small tear in the liner of Seepage Pond 2 (Photo II-C-12, Appendix II). | 3 | May 2017 (CLOSED) |

Notes:

- 1. Recommendation priority guidelines, specified by Teck and assigned by KCB:
 - Priority 1: A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant risk of regulatory enforcement.
 - Priority 2: If not corrected could likely result in dam safety issues leading to injury, environmental impact or significant regulatory enforcement; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
 - Priority 3: Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
 - Priority 4: Best Management Practice Further improvements are necessary to meet industry best practices or reduce potential risks.

Table 2 2017 Recommendations for Deficiencies and Non-Conformances

| ID No. | Deficiency or Non- Conformance | Applicable Regulation or OMS Reference | Recommended Action | Priority ⁽¹⁾ | Recommended Deadline (Status) |
|--|--------------------------------------|---|---|-------------------------|-------------------------------------|
| | | | L-L Dam | | |
| | | | No new recommendations from 2017 | | |
| | | | H-H Dam | | |
| | | | No new recommendations from 2017 | | |
| | | | 24 Mile Lake | | |
| | | | No new recommendations from 2017 | | |
| | | L-L | . Dam Seepage Collection and Slimes Ponds | | |
| SP-2017- 01 Water Management Flood Routing Involving pumping to manage the IDF without off- lease discharge until upgrade works are complete. Suggested interim milestones: Design in2018, Permit and IFC in 2019 and Construction in 2020 | | | | Q4 2020 | |
| SP-2017- 02 | Water Management | Flood Routing | Regrade pipe to a consistent downward grade | 2 | Q4 2018 |

Notes:

- 1. Recommendation priority guidelines, specified by Teck and assigned by KCB:
 - Priority 1: A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant risk of regulatory enforcement.
 - Priority 2: If not corrected could likely result in dam safety issues leading to injury, environmental impact or significant regulatory enforcement; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
 - Priority 3: Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
 - Priority 4: Best Management Practice Further improvements are necessary to meet industry best practices or reduce potential risks.

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1 INTRODUCTION

Klohn Crippen Berger Ltd. (KCB) was engaged by Teck Highland Valley Copper Partnership (THVCP) to complete the 2017 dam safety inspection (DSI) of the Highland Tailings Storage Facility (TSF) on the Highland Valley Copper (HVC) mine site, as shown in Figure 1. Tailings from ongoing mining operations are discharged into the Highland TSF. Tailings are retained in the impoundment by two containment dams (L-L Dam and H-H Dam) which are incrementally raised to meet tailings storage requirements, typically on an annual basis.

The scope of this report includes: L-L Dam and the Seepage Collection and Slimes Ponds at the downstream toe; H-H Dam and 24 Mile Lake at the downstream toe of the H-H Dam.

The scope of work consisted of:

- a visual inspection of the physical conditions of the various containment facilities;
- a review of updated piezometer, inclinometer and seepage monitoring data provided by THVCP;
- a review of climate and water balance data for the site;
- a review of other relevant dam safety management documents (e.g. Operations, Maintenance & Surveillance (OMS) manual); and
- a review of the past year's construction records.

The inspection and this report were prepared to comply with Section 10.5.3 of the Health, Safety and Reclamation Code for Mines in British Columbia (the Code), Section 4.2 "Annual Tailings Facility and Dam Safety Inspection Report" of the Code Guidance Document (MEM 2016).

The visual inspection was completed by the Engineer of Record (EoR), Mr. Bill Chin, P.Eng., as a representative of KCB on September 27, 2017. The weather during the DSI was clear and sunny and did not impact the visual inspection. Mr. Chris Anderson, P. Eng., THVCP Tailings and Water Superintendent, is the TSF Qualified Person (as defined by the Code) for the Highland TSF.

The Highland TSF is operated under the following permits:

- British Columbia Ministry of Energy, Mines and Petroleum Resources (MEM) Geotechnical Permit M11 – the permit covers the entire mine life, property and the surrounding area and communities. This permit was last amended in 2011, approving the Lornex Pit expansion, the new South Side Tailings Distribution System and raised ultimate crest elevations of H-H Dam and L-L Dam. Section B 2 of the permit lists the conditions concerning dam safety at Highland TSF.
- British Columbia Ministry of Environment (MOE) Water Licenses 46527 and 46528 these licenses allow diversion and storage of water from Pukaist Creek on Crown Land.



 British Columbia MOE Effluent Permit PE-376 – this permit allows the storage of tailings and effluent in the Highland TSF.

The L-L Dam and H-H Dam have been assigned an "Extreme" consequence classification and a "Very High" consequence classification, respectively, in accordance with the Code which refers to the Canadian Dam Association (CDA) Dam Safety Guidelines (CDA 2013). These classifications are based on the latest dam consequence review hosted by THVCP on January 16, 2018, which the EoR's designate (Mr. Rick Friedel, P.Eng. of KCB) participated in via teleconference. The classifications were subsequently reviewed and agreed by the EoR (Mr. Bill Chin, P.Eng. of KCB). A summary list of the consequence classifications assigned to each of the supplementary dams downstream of the main dams is provided in Section 2.

The Code recommends a dam safety review (DSR) be undertaken every 5 years based on the "Extreme" consequence classification of L-L Dam and "Very High" consequence classification for the H-H Dam. The other dams at the facility with lower consequence classifications have previously been included in the Highland TSF DSR. The most recent DSR for the Highland TSF dams was carried out by Stantec in 2017 and is in process of documentation. The next DSR is scheduled for 2022.

2 FACILITY DESCRIPTION

2.1 General

The Highland Valley Copper Mine, owned and operated by THVCP, is located near Logan Lake, approximately 45 km southwest of Kamloops, in the interior of British Columbia, as shown in Figure 1. The Highland TSF is located approximately 6.5 km northwest of the operating mill.

The tailings are retained in the Highland TSF by the L-L Dam (west end) and H-H Dam (east end) which were constructed across either end of the Highland Valley. The free water pond is maintained at the western end of the TSF, separated from the core of the L-L Dam by a combined 126 m width of compacted sand zone and a tailings beach. Three seepage collection ponds (Seepage Pond 1, Seepage Pond 2, Seepage Water Reclaim Pond) are located at the downstream toe of the L-L Dam. Two active slimes ponds (Slimes Pond 1 and Slimes Pond 2) are also located at the downstream toe of the L-L Dam and are used to collect construction water and slimes (i.e. suspended sediment) from hydraulic sand placement on the downstream side of L-L Dam. Two inactive slimes ponds (Slimes Pond 3 and Slimes Pond 4) are also located at the downstream toe of the L-L Dam. All seepage and construction water reports to Seepage Water Reclaim Pond, where it is subsequently pumped back to the TSF pond via the seepage water reclaim pipeline. The accumulated slimes in the slimes ponds are excavated during winter months when the slimes are frozen, and hauled to the impoundment for disposal on an as needed basis. The layout of the L-L Dam structures is shown in Figure 2.

The 24 Mile Lake facility area was formally a natural lake that is now surrounded by waste dumps with no clearly defined dam structure. The facility is downstream of the H-H Dam, physically separate from the Highland TSF, and collects seepage from the H-H Dam as well as emergency tailings disposal from the H-H Pumphouse during periods of tailings line maintenance and/or operational upsets. The layout of the H-H Dam and 24 Mile Lake are shown in Figure 4. In 2017, KCB completed an assessment was initiated to review the potential for declassifying 24 Mile Lake facility as a "dam" under the Code (KCB 2018c). This work was still in progress at the time of this DSI.

Pertinent information on the dam structures at the Highland TSF are included in Table 2.1. Further descriptions for the L-L Dam, H-H Dam and 24 Mile Lake are provided in the subsequent sections of the report. A schematic of the tailings and water management system is shown on Figure 2.1.

Table 2.1 Containment Facilities at the Highland TSF

| Dam / Facility | Consequence Classification ⁽¹⁾ | Purpose | Construction | Current Approximate Dam Crest Length (m) | Current Dam Crest Elevation (m) | Current Approximate Maximum Dam Height (m) |
|-------------------------------------|--|--|--|--|---------------------------------------|--|
| L-L Dam | Extreme | Contains the Highland TSF at its downstream (western) limit. | Earthfill starter dam that has been raised using centreline method of construction with a downstream zone of compacted cycloned sand, a centreline vertical till core, and a sand and gravel drainage blanket beneath the downstream shell of the dam. | 3,000 | 1260.5 | 161 |
| H-H Dam | Very High | Contains the Highland TSF at the upstream (eastern) limit. | Earthfill starter dam that has been raised using the centreline method of construction, with a downstream zone of variable mine waste and a centreline vertical core. | 1,755 | 1271 to 1277 | 97 |
| 24 Mile Lake | Low ^(1,2) | Receives seepage from the H-H Dam and emergency tailings disposal from the H-H Pumphouse during maintenance of the tailings lines and/or operational upsets. | A former natural lake surrounded by waste dumps. | n/a | Min. 1220 (waste dumps) | 40 to 50 (waste dumps) |
| Seepage Water Reclaim Pond | Significant | Part of the reclaim system, which pumps water back to the TSF. Receives water from surface runoff, the slimes ponds and the seepage collection ponds. | Original embankment on the west side of pond is homogeneous glacial till with downstream sand and gravel filter. Pond is unlined. | 95 | 1103.2 | 5 |
| Seepage Pond 1 | n/a (decommissioned) ⁽³⁾ | Collects seepage from the toe of the VBB at the L-L Dam. Pond is unlined. | Excavated into natural ground with an embankment constructed of silty sand (possible Glacial Till) fill on the southwest side. The northeast side of the pond is formed by the sand and gravel drainage blanket to the VBB. Pond is unlined. | 500 | 1104 | 3 |

| Dam / Facility | Consequence Classification ⁽¹⁾ | Purpose | Construction | Current Approximate Dam Crest Length (m) | Current Dam Crest Elevation (m) | Current Approximate Maximum Dam Height (m) |
|-------------------|--|--|--|--|---------------------------------------|--|
| Seepage Pond 2 | Significant | Collects seepage primarily from finger drains under the NBB at the L-L Dam. | Excavated into natural ground on three sides, with a homogenous glacial till embankment on the north side of the pond. Pond is lined with geomembrane. | 80 | 1116.6 | 1.8 |
| Slimes Pond 1 | Significant | Was used to collect construction water and fines from hydraulic sand placement on the downstream side of L-L Dam prior to the construction of Slimes Pond 2. | Excavated into natural ground with homogeneous glacial till embankment on the northeast (NE) and southwest (SW) sides. Pond is unlined. | NE = 500 SW = 700 | 1103.2 | 2.5 to 3.75 m |
| Slimes Pond 2 | Significant ⁽¹⁾ | Collects construction water and fines from hydraulic sand placement on the downstream side of L-L Dam. | Excavated into natural ground on three sides, with a homogeneous glacial till embankment on the south side. Pond is lined with geomembrane. | 100 | 1126.9 | 10 |
| Slimes Pond 3 | n/a (under construction) | Once completed will collect construction water and fines from hydraulic sand placement on the downstream side of L-L Dam. | nt Under construction. | | | |
| Slimes Pond 4 | n/a (inactive) | Was used to collect construction water and fines from hydraulic sand placement on the downstream side of L-L Dam prior to the construction of Slimes Pond 2. | · | | | |

Notes:

- 1. As assigned during the Dam Classification Workshop held in January 2018.
- 2. Assessed in late 2017 for potential to be declassified as a dam under the Code (KCB 2018c).
- 3. During the 2014 review of the consequence classification, THVCP and KCB's EOR at the time concluded that Seepage Pond 1 was "decommissioned". However, annual inspection of this pond is maintained as part of the annual DSI for the Highland TSF.

Pumped water flow Tailings dam ---> Gravity water flow ---> Spillway **BOYES CREEK** WOODS CREEK Pond Seepage flow WITCHES FORGOTTEN CREEK Tailings flow BROOK Control valve (THVCP) PUMPHOUSE⁽²⁾ BURR CREEK Natural watercourse Control valve (Cooks Ferry Band) Emergency tailings dump Ż Construction material Spigot point 0 Emergencystop 3 6 Tailings Lines 2, 3, 4, 6, 7 PONDS н-н MOP PUMP 7 Construction HOUSE Ε 2 Water / Slimes **TAILINGS** DAM 7 Day 1 POND Н-Н SEEPAGE HIGHLAND TSE Pond Deep PONDS SAMP measures PUKAIST CREEK Wells 12 24 MILE RECLAIM LAKE BARGES 8 SS1 (13 SFEPAGE 6 SS2 WATER SS3 RECLAIM 17 POND **B19 SURFACE** FRED BROOK RAW WATER (16)->To Highland RUNOFF NO. 2 CYCLONE (15) RESERVOIR^(1,2) BOOSTER HOUSE (11) SPATSUM PUMPHOUSE Notes: Ż See the Raw Water Reservoir, Calling Lake, Laura Lake, and Jim Black Lake process flow diagrams for details. JIM BLACK CALLING Not all flow paths and controls are shown herein LAURA LAKE(1,2) 10 LAKE(1,2) IAKF(1,2) Seepage flow pathway from L-L Dam to Pukaist Creek is currently under investigation as part of SAMP. Roman Road Pipeline 1x 20" HDPE pipeline Operational 2 Dewatering to H-H Pumphouse 1x 12" HDPE pipeline Operational System consists of: i) riprap-lined collection pond below Highway 97c; 1) hip Pumphouse and motor control center buildings; iii) ply Norm HDPE pipeline HDPE 3 Woods Creek Diversion Operational 4 Woods Creek Diversion to Pukaist Creek Operational 5 Woods Creek Diversion to SWRP 1x 500 mm dia. HDPE pipeline with knife gate valve Operationa 6 Construction Water / Slimes to Slimes Ponds Gravity flow of water and slimes from one or more hydraulic cyclone sand cells during construction. Operational during construction Slimes Ponds to Seepage Ponds Slimes Pond 1 drains to SWRP, and Slimes Pond 2 drains to Seepage Pond 2. Operational Seepage Pond 2 Outlet $1x36^{\prime\prime}$ dia. HDPE pipe graded upward for the first 12.1 m, then downward for the next 44 m to the SWRP with control value $1x36^{\prime\prime}$ dia. Operationa Low-Level Outlet and Spillway rel outlet: 12" dia. HDPE pipe with control valve and submerged intake trash rack Operational (Calling Lake) Spillway: 12' wide channel Diversion Outlet (Laura Lake) 10 12" dia. HDPE pipe with control valve and intake trash rack Operational 11 Jim Black Diversion to SWRP Open channel with U/S y-valve Operationa 12 SWRP to TSF 2x permanent pumps in the Seepage Water Reclaim Pond pumphouse feeding 2x 18" dia. HDPE pipelines to the L-L Pond Operational 13 Reclaim Barges #1 and #2 Reclaim Barges on the south side of the L-L Pond Operational 14 Bypass via Reclaim Isolation Valve Bypass line controlled by hydraulic valve Operational 15 Reclaim to Raw Water 2x 36" dia. HDPE pipelines from No. 2 Booster to Raw Water Reservoir Operational Consists of Low-Level Outlet at East Dam (2x HDPE pipelines with control valves) and #3, #4 Reclaim (2x HDPE pipelines with 16 Raw Water to Mill Operational

Figure 2.1 Process Flow Diagram for Highland TSF



South Side Tailings Gland Water System

1x 10" HDPE pipelin

2.2 L-L Dam

The layout of the L-L Dam and associated structures is shown in Figure 2. For context and reference, selected design drawings and typical cross sections are included in Appendix III. Since there was no construction in 2017 other than the start of foundation preparation for the 2018 dam raise, the construction record drawings issued in the 2015 construction summary report (KCB 2015) remain applicable for the purposes of this report. General information regarding the dam is as follows:

- As of the end of 2017, the crest elevation of L-L Dam is 1260.5 m, corresponding to a maximum height (crest to toe of Valley Buttress Berm at El. 1107 m) of about 155 m. The currently permitted ultimate crest of the L-L Dam is El. 1279 m, which corresponds to a maximum height of 172 m at the same location.
- The starter dam construction records were reported by Bechtel (1977). Construction records
 of subsequent raises are contained in various construction summary reports (1978 to 2015).
- The L-L Dam foundation characterization is documented in the KCB report entitled "L-L Dam Geological and Geotechnical Site Characterization Report" (KCB 2016o), which should be referenced for details. The typical stratigraphic sequence beneath the L-L Dam, starting from surface, is as follows:
 - Upper Sand and Gravel;
 - Lacustrine Unit:
 - Unit has been excavated from the dam footprint downstream of the starter dam.
 - Upper Glaciolacustrine Unit;
 - Glacial Till;
 - Lower Glaciolacustrine Unit;
 - Lower Sand and Gravel;
 - Lower Glacial Till / Gravel; and
 - Bedrock (Kamloops Group volcanic and sedimentary units, and Guichon Creek granodiorite).
- The dam comprises the following main elements (KCB 2010):
 - An initial earthfill starter dam, raised via the centerline method with a 15.2 m wide low permeability vertical glacial till core supported by a downstream zone of hydraulically placed, compacted cycloned sand and an upstream compacted cycloned sand berm and tailings beach.
 - A sand and gravel drainage blanket beneath the downstream shell of the dam, with granular finger drains at selected locations.
 - Downstream slopes ranging from 2H:1V to 3H:1V, with three buttress berms constructed
 of hydraulically placed, compacted cycloned sand, designated as the South Buttress Berm
 (SBB), the Valley Buttress Berm (VBB) and the North Buttress Berm (NBB). With these
 buttress berms in place, the current overall equivalent slopes at these locations are
 approximately 3H:1V, 3.5H:1V, and 7H:1V respectively.



- The foundation blanket and foundation drains direct seepage to Seepage Pond 1, Seepage
 Pond 2 and the Seepage Water Reclaim Pond.
- Construction water and fines are collected in Slimes Pond 1 and Slimes Pond 2, with the water being decanted into the Seepage Water Reclaim Pond.
- Seepage and construction water from the toe of the dam is pumped back into the impoundment from the Seepage Water Reclaim Pond via a 460 mm (18 inch) pipeline which crosses the dam crest at the south abutment.

2.3 H-H Dam

The layout of the H-H Dam is shown in Figure 4. Refer to Appendix III for selected design drawings and typical cross sections. The dam was raised late 2017, to be documented in the construction record drawings in the 2017 construction summary report (in progress). General information regarding the dam is as follows:

- As of December 2017, the crest elevation of H-H Dam varies from 1271 m (west abutment) to 1277 m (east abutment), with a maximum downstream slope height of 97 m. The current permitted ultimate crest of the H-H Dam is El. 1292.7 m, which corresponds to a maximum height of 113 m.
- The starter dam construction records were reported by Bechtel (1972). Construction records of subsequent raises are contained in various construction summary reports (1978 to 2015).
- The foundation conditions at the H-H Dam are documented in the KCB report entitled "H-H Dam Performance Review Report" (KCB 2016b), which should be referenced for details. The typical stratigraphic sequence beneath the H-H Dam, starting at surface, is as follows:
 - Upper Sand and Gravel;
 - Glacial Till;
 - Sand and gravel with interlayered silt and clay; and
 - Bedrock (Guichon Creek granodiorite).
- The dam comprises the following main elements (KCB 2010):
 - An earthfill starter dam, raised via the centerline method with a low permeability centreline vertical glacial till core, supported by random fill and tailings beach on the upstream side and variable mine waste fill on the downstream side.
 - Downstream slope of approximately 2H:1V, with a buttress of varying heights along the dam alignment, constructed of mine waste.
 - An approximately 6.8 m high lock-block retaining wall constructed at the east of the dam immediately adjacent to the H-H Pumphouse.
- Seepage from the H-H Dam reports to 24 Mile Lake.



2.4 24 Mile Lake

The layout of 24 Mile Lake is shown in Figure 4. There are no design drawings available. General information regarding 24 Mile Lake is as follows:

- The facility is a former natural lake surrounded by waste dumps that have been placed over the life of the mine. There are no construction records for the facility.
- AMEC (2014a) indicated that the two dumps formed on the valley side slopes downstream of the lake consist of overburden stockpiled from pit stripping operations. AMEC (2014a) also indicated that some of these dumps contain free dumped high plastic clay which may be in excess of 40 m thick above the outlet haul road level, and that a portion of these dumps are capped with waste rock for armouring purposes.
- AMEC (2014b) indicated that the foundation conditions include waste fills (as described above), glaciofluvial sand and gravels, glacial till, and interglacial silts and sands. The latter deposits contain glaciolacustrine units, including those designated as 10A, 10B, and 10C, which are prevalent in the valley mine pit and contain slickensided shear surfaces. No information is provided by AMEC as to the depth and extent of the glaciolacustrine units at the facility.
- 24 Mile Lake is an active containment facility which receives emergency discharge of water and tailings from the H-H Pumphouse during tailings line maintenance and operational upsets via an inlet located in the northeast corner of the facility.
- The water in 24 Mile Lake is pumped back to the H-H Pumphouse, via a pump located on the east side of the facility, maintaining a relatively constant pond volume throughout the year.
- The current depth of tailings is unknown, but is estimated to be at least 30 m based on a 1970 survey of the original lake edge.

2.5 Seepage Collection and Slimes Ponds

Slimes, seepage collection and water reclaim ponds were constructed and modified as required to accommodate the expanding footprint of the L-L Dam and meet construction requirements. The design and construction records for most of these ponds are not available.

- Seepage Pond 2 was designed by THVCP.
- Slimes Pond 2 and Slimes Pond 3 were designed by KCB (KCB 2014d, KCB 2015d):
 - Slimes Pond 2 was constructed in 2014 and commissioned in 2015.
 - Slimes Pond 3 construction began in 2015 and has not been completed.



3 HISTORY AND RECENT ACTIVITY

3.1 General

Construction of the Highland TSF began in 1971 with the J-J Starter Dam, which was located approximately half-way between the H-H Dam and the L-L Dam. In 1972 the H-H Starter Dam was constructed, followed by the L-L Starter Dam between 1976 and 1979. By 1991 the J-J Dam was buried by tailings.

Over the 45-year life of the facility, there has been five major design updates, along with some minor design revisions, to reflect changes in the mine plan. The current design for the facility is documented in the KCB report entitled "Highland Valley Tailings Storage Facility, Design Update for L-L Dam Crest Elevation 1279 m" (KCB 2010).

3.2 L-L Dam

3.2.1 Historical Overview

A brief history of the construction and operations at the L-L Dam is summarized below. This information was based on the construction summary reports prepared from 1978 to 2015. No construction took place at the L-L Dam in 2015 and 2016.

- The starter dam was constructed to El. 1155 m between 1976 and 1979 (Bechtel 1977, KL 1988). Since then, the dam has been raised on a regular basis (typically annually).
- The first major redesign occurred with the excavation and replacement of the downstream lacustrine deposits followed by the extension of the VBB during construction of the starter dam in 1978 (KL 1988).
- The Seepage Water Reclaim Pond was constructed in the 1980's.
- In the 1990s, the NBB was added to the section of the dam between about Sta. 2+300 and 2+800 where potentially weaker sedimentary layers were noted in the underlying bedrock (KL 1992, KCB 1999).
- Slimes Pond 1 and Seepage Pond 1 were constructed in the early 1990's.
- The SBB, was also added in the 1990's to support an upstream bend in the dam alignment (KC 1995).
- In 2001, a Low-Level Outlet (LLO) was constructed at the south abutment of the dam (KCB 2014a). In addition, an upstream compacted cycloned sand zone was incorporated into the design to provide additional support to the glacial till core in the event of earthquake induced liquefaction of the tailings beach. This upstream cycloned sand zone extends 126 m upstream and the centreline and is raised concurrently with the till core.
- Seepage Pond 2 was constructed in 2012 (KCB 2013).



- In 2013 the LLO was decommissioned by backfilling with concrete (KCB 2014a), and in 2015 an
 inverted filter drain was constructed at the outlet of the decommissioned LLO to mitigate
 piping risks (KCB 2015c).
- Also in 2013, the pumps in Seepage Pond 1, which controlled outflow to the Seepage Water Reclaim Pond, were removed and replaced by the installation of a gravity outlet pipe at the south end of the pond (KCB 2014b).
- In 2014, construction of the Valley Buttress Berm Extension (VBBE) commenced, which connects the NBB to the VBB.
- Slimes Pond 2 was constructed in 2014 (KCB 2015).
- In 2015, Slimes Pond 3 construction commenced but is not yet complete (KCB 2016c).

3.2.2 2017 Construction Activities

During 2017, THVCP entered into a new construction contract (Highland TSF 2017-2019 Construction) for the planned three-year sequence of annual raises at the L-L Dam and H-H Dam. Delays in construction startup limited the amount of work completed at the L-L Dam in 2017, as described below. However, upstream beach construction continued as in previous years. The next scheduled milestone is to raise the crest to El. 1262.5 m by November 30, 2018 which is an additional 2 m above the current crest at El. 1260.5 m.

The construction activities listed below were completed in 2017 and will be reported in greater detail in the 2017 construction summary report.

Upstream S2 Fill Zone – Underflow Pipe Grade and Excavation/Removal of Secondary Cyclone Pipelines at South Dam

In preparation for hydraulic cycloned sand placement as part of 2018 construction activities, THVCP engaged a Contractor to remove existing secondary cyclone pipelines from their location (per end of 2015 construction season) and construct a new underflow pipe grade within the upstream S2 fill zone. From June 28, 2017 to July 8, 2017, the following dam construction works to support pipe placement took place:

- Widen the pipe grade downstream of the existing overflow pipeline by excavating part of the S2 fill between the South Abutment (Sta. 0+800) and Sta. 2+000;
- Place excavated fill within the S2 zone (between Sta. 0+800 and Sta. 2+000);
- Excavate and remove four of the eight pipes within the S2 zone between Sta. 1+400 and Sta.
 2+000 from the secondary cyclones to the tailings beach; and
- Backfill the excavated pipe trenches (2 m to 3 m deep), using the S2 fill from the excavations.



Fill placement was carried out per the mechanical placement specifications from the 2017 construction package, summarized as:

- Fill placed in horizontal lifts (with dozer or excavator);
- Maximum 0.3 m loose lift thickness; and
- Compacted to minimum 97% of standard Proctor maximum dry density.

THVCP dam inspectors provided quality control (QC) during construction which included initial full time / later part time monitoring, completing daily checklists to document compliance with specifications, taking daily photographs, and conducting pre-/post-construction as-built surveys. KCB provided intermittent quality assurance (QA) coverage including two (minimum) nuclear densometer tests per day and daily photographs of the work area.

Based on available QA/QC records, the completed works are in compliance with specifications.

Foundation Preparation at North and South Dam

From November 19, 2017 to December 20, 2017, the Contractor undertook foundation preparation activities at two segments of L-L Dam: North Dam (Sta. 2+800 to Sta. 3+600) and South Dam (Sta. 1+100 to Sta. 1+200). The work consisted of:

- removal of unsuitable material down to competent foundation where the GLU is absent, or
 just above the top of GLU where present (in these areas, excavation of the GLU will be carried
 out at a later date in 2018 in close coordination with progressive backfill as the excavation
 progresses); and
- placement of approximately 7,700 m³ of SG1 drainage blanket fill at the North Dam (Sta. 3+000 to Sta. 3+200).

The construction activities were carried out in general accordance with the specifications in the 2017 construction documents. Temporary works and temporary excavations are the responsibility of the contractor, but are reviewed by KCB from a dam safety related perspective.

Beach Construction

Beach construction via open ended discharge of Primary cyclone underflow and overflow tailings from the crest of the L-L Dam was carried out between April and September 2017, between Sta. 1+600 and Sta. 2+900.

3.3 H-H Dam

3.3.1 Historical Overview

A brief history of the construction and operations at the H-H Dam is summarized below. This information was based on the construction summary reports prepared from 1978 to 2015. In general, up to 1993, the H-H Dam was constructed by Contractors using local borrow materials. From 1993



onwards, the portion of dam downstream of the core and downstream filter zone (SG2 fill zone) is constructed by THVCP, employing its own equipment and workforce, using select waste material from the Valley Pit. This was carried out in 2016 with intermittent placement of downstream RF2 fill out to the ultimate dam profile (between Sta. 0+540 and Sta. 1+600, up to approximately El. 1271 m).

- The starter dam was constructed in 1972 to El. 1193.6 m (Bechtel 1972). Following completion
 of the starter dam, the dam has been raised on a regular basis (typically annually).
- After about 1992, the downstream shell was raised by end-dumping sand and gravel (random fill) material in up to 10 m thick lifts with only traffic compaction by construction equipment (i.e., haul trucks). A 10 m wide "transition zone" immediately downstream of the glacial till core was typically compacted in thinner (2 m) lifts (KCB 2016b).
- From about 2003 to 2007, a waste dump was constructed downstream of the dam which raised the original ground at the toe from El. 1195 m to about El. 1224 m to El. 1227 m (KCB 2016b). The waste dump was constructed in about 10 m thick lifts with the top of each lift compacted by construction equipment traffic.
- Between about 2008 and 2009, the downstream side of the dam was further raised to about El. 1255.5 m across most of the dam length using Bethsaida sand (sand and gravel fill) with similar construction procedures as per the random fill:
 - Over approximately the same period, a portion of the waste dump along the western end of H-H Dam (i.e. from about Sta. 0+600 to Sta. 1+200) was raised as high as El. 1290 m.
 - Along the eastern end of H-H Dam (i.e. from about Sta. 1+500 to Sta. 1+950), and immediately upstream of 24 Mile Lake, the waste dump was maintained at about El. 1224 m.
- In 2010, a filter zone was added between the glacial till core and the downstream fill zones (KCB 2010).
- A 6.8 m high lock block retaining wall was designed by Sadé Engineering Associates (SEA) and constructed by THVCP during 2012 at the toe of the dam at Sta. 2+020.
- In 2014, a rockfill buffer zone was added along the upstream edge of the dam to mitigate erosion by tailings slurry (KCB 2014).

3.3.2 2017 Construction Activities

During 2017, THVCP entered into a new construction contract (Highland TSF 2017-2019 Construction) for a planned three-year sequence of annual raises at the L-L Dam and H-H Dam. Delays in 2017 meant not all work was carried out to design lines and specifications (deviations summarized below). However, the minimum 2017 Till Core raise was reviewed based on updated depositional modelling, and the horizontal crest milestone at El. 1273 m was revised from to a sloping crest from El. 1271 m (West Abutment) to El. 1277 m (East Abutment). The next milestone crest raise is planned to El. 1276 m by November 30, 2018 which will be reviewed in 2018.

The construction activities listed below were completed in 2017 and will be reported in greater detail in the construction record report for the ongoing raise.

Annual Dam Raise

A brief summary of the construction activities at the H-H Dam is provided below:

- The downstream RF2 Zone (downstream of the glacial till core and transition/filter zone) was raised on two separate occasions to El. 1277 m (between Sta. 0+540 and Sta. 1+600) and El. 1271 m (between Sta. 1+500 and Sta. 1+700).
- The upstream RF1/RF2 Zone was raised 2 m from West to East Abutment, 1 m higher than the adjacent Till Core Zone to facilitate construction of a pipe grade which was planned to be used to discharge tailings onto the beach and also act as a safety berm to maintain minimum 1 m buffer height (in compliance with the OMS).
- The SG2 Zone was raised between 2 m (east abutment, to El. 1277 m) and 1 m (west abutment, to El. 1271 m) while maintaining the 0.6% grade from Sta. 1+400 to the east abutment, consistent with the Till Core Zone raise.
- The Till Core Zone was raised between 2 m (East Abutment, to El. 1277 m) and 1 m (West Abutment, to El. 1271 m) while maintaining the 0.6% grade from Sta. 1+400 to the east abutment. Due to inclement weather late in the year, till was placed per temporary requirements (i.e. not to full IFC specification) as agreed upon between KCB and THVCP. This material is part of the Till Core Zone buffer (1 m) and will be removed and replaced with fill placed to specifications during the next Till Core raise in 2018.

Relocation of Tailings Pipelines

Prior to November 2017, tailings deposition at the H-H Dam consisted of discharge through six lines at three locations: SS1 (Line 5 by West Abutment), Sta. 2+200 (Lines 2, 3, 4), and East Abutment (Lines 6, 7 which are ~100 m northwest of the dam abutment). Lines 6, 7 are preferentially used because discharge at SS1 and Sta. 2+200 have been known to cause erosion and sudden rises of the tailings beach against the upstream slope of H-H Dam. However, in early 2017, Lines 6, 7 were briefly inundated by build-up of tailings at the pipeline outfalls, and THVCP began to develop plans to raise the two lines and explore alternative deposition strategies from the crest of the H-H Dam.

A summary of construction activities related to the relocation of tailings pipelines is given in Table 3.1.



Table 3.1 Construction Activities for Relocation of Tailings Pipelines at H-H Dam

| Plan Component | Construction Activity | Completed in 2017? |
|--|---|---|
| Pipe grade for Lines 3, 4 across the upstream edge of the H-H Dam | Construct an RF2 bench on top (where needed for grade) of the 2017 as-built upstream RF1/RF2 zone: Sta. 2+200 to Sta. 1+300: 8 m wide Sta. 1+300 to ~Sta. 1+000: <8 m wide to avoid placing within the Till Core Zone | Y |
| Relocation of Lines 3, 4 across the upstream edge of the H-H Dam crest | Move discharge points from Sta. 2+200 to Sta. 1+600 and Sta. 1+300. | N (pending review of deposition strategy) |
| Raise and extension of Lines 6, 7 | Construct new right of way for pipelines from East Abutment to ~150 m northwest of the dam abutment. Relocate Lines 6, 7 to new right of way. | Y |

THVCP plan to review the tailings deposition strategy for the Highland TSF as a whole, and work with KCB to determine long-term sequencing.

The new right of way for Lines 6,7 was constructed across the H-H dam at the east abutment. The fill placed for this right of way is considered temporary and will be removed during future raises of the dam crest.

4 WATER MANAGEMENT

4.1 Overview

Water management at each structure and how they interact with each other is summarized below. Figure or drawing references for key operating water management structures, where available, are summarized in Table 4.1.

Table 4.1 References for Operational Water Management Structures for Highland Facility

| Facility | Structure Name | Drawing or Figure Reference (Appendix III) |
|--------------------------------|--------------------------------|---|
| L-L Dam / H-H Dam | Reclaim barges 1 and 2 | Figure 17 |
| Soonage Dond 2 | Outflow pipe | No design drawing available |
| Seepage Pond 2 | Temporary pump | No design drawing available |
| Slimes Pond 1 Pumping pipeline | | No design drawing available |
| Slimes Pond 2 | Outflow pipe | D-74001 |
| Silities Porid 2 | Underdrain pump | D-74001 |
| Seepage Water Reclaim Pond | Seepage water reclaim pipeline | 111-803-140 |
| 24 Mile Lake Portable pump | | No design drawing available |

Highland TSF

- Inflows include water from tailings slurry (discharged either upstream of the H-H Dam or upstream of the L-L Dam), precipitation on the pond, seepage return water pumped from the Seepage Water Reclaim Pond, seepage return water pumped from 24 Mile Lake, surface runoff from surrounding catchments, and inflows from Boyes Creek, Burr Creek, and Forgotten Creek all located on the northern side of the TSF:
 - Flows from Woods Creek, which previously reported to the TSF, are now diverted into Pukiast Creek downstream of the TSF via the Woods Creek Diversion Scheme, which is part of the Pukiast Creek Sulphate Adaptive Management Plan (THVCP 2017a).
- The free water pond is maintained at the western end of the impoundment. The water level in the pond varied by 6.6 m in 2017, which is greater than historical trends over the past six years (~3.7 m). This is largely attributed to the runoff resulting from rapid melt of a larger than average snowpack during freshet, discussed further in Section 4.2.
- Outflows include seepage from both dams, reclaim water from two reclaim barges located on the south side of the impoundment near the L-L Dam which is pumped to the Raw Water Reservoir for reclaim to the Highland Mill and evaporation. The Highland TSF has no spillway and is designed to store the environmental design flood (EDF) and inflow design flood (IDF); refer to Section 4.4.

Seepage Pond 2

• Inflows include seepage from northern sections of the L-L Dam (via Finger Drain #1 which channels seepage from beneath the North Dam and North Buttress Berm), precipitation on the pond, runoff from upstream catchments, and decant water from Slimes Pond 2.

- The water level is not regularly surveyed by THVCP. Based on a visual estimate, the pond level
 was visually estimated to be 1.5 m below the dam crest during the 2017 DSI site visit.
- Outflows include gravity flow through a 900 mm (36") outflow pipe located in the southeast corner of the pond and evaporation. The outflow pipe is graded upward at 4.2% for the first 9.5 m and 0.6% for the next 2.6 m, then downward at 3.5% for the next 44 m to the outlet. A flow control valve is located along the upward 0.6% segment of pipe. A temporary pump can also be used to increase outflow capacity during active hydraulic sand placement when decant water was flowing from Slimes Pond 2. Outflows from the pond report to the Seepage Water Reclaim Pond. There is no spillway for this pond.

Slimes Pond 1

- Inflows include seepage from the L-L Dam, seepage from Seepage Pond 1, precipitation on the pond, and runoff from upstream catchments.
- The water level is not regularly surveyed by THVCP. Based on a visual estimate, the pond level was 5 m below the dam crest during the 2017 DSI site visit.
- Outflows include seepage, which flows into the adjacent Seepage Water Reclaim Pond, and sporadic pumping by THVCP to drawdown water levels in advance of slimes excavation. There is no spillway for this pond.

Slimes Pond 2

- Inflows include construction water from hydraulic sand placement on the downstream side of L-L Dam, precipitation on the pond, and runoff from upstream catchments.
- The water level is not regularly surveyed by THVCP; and the pond was dry during the 2017 site inspection.
- Outflows include gravity flow through a 900 mm (36") outflow pipe which is directed into Seepage Pond 2, evaporation and discharge via an underdrain pump which can be used to manually draw down the water level in the pond prior to slimes excavation. There is no spillway for this pond.

Seepage Water Reclaim Pond

- Inflows include seepage from the L-L Dam (via the UVD/LVD drain), seepage from Slimes Pond 1, outflow from Seepage Pond 2, outflow from Seepage Pond 1, outflow from Jim Black Lake, precipitation on the pond, diverted flows from Woods Creek, and runoff from upstream catchments.
- The water level is regularly surveyed by THVCP. The water level varied seasonally up to 1.2 m in 2017 and the minimum freeboard measured was 2.3 m.
- Outflows include seepage, reclaim pumping back to the impoundment via the seepage water reclaim pipeline which crosses the L-L Dam crest at the south abutment. The pumpback system is comprised of two Peerless 6-Stage 16HXB vertical turbine pumps, each with a rated discharge of 3,000 USGPM (0.19 m³/s) for a total capacity of 0.38 m³/s. There is no spillway for this pond which relies on pumping to manage the IDF, as discussed in Section 4.4.

24 Mile Lake

- Inflows include water from tailings slurry from the H-H Pumphouse, seepage from the H-H Dam, precipitation on the pond, and runoff from upstream catchments.
- The water level is regularly surveyed by THVCP. The water level varies seasonally, up to 3.4 m in 2017. The minimum freeboard measured during 2017 was 20 m.
- Outflows include seepage that discharges to the Valley Pit, and reclaim water pumped to the
 H-H Pumphouse via a portable pump located on the east side of the facility. There is no
 spillway for this facility.

4.2 Climate

Climate data was collected throughout the year from the L-L Dam weather station (El. 1122 m) and summarised on Table 4.2and

Figure 4.1. Climate normals (1981 to 2010) from the Highland Valley Lornex Station (Environment Canada Station No. 1123469) are shown on the same figure for comparison. This climate station was located near the Highland Mill, and had the longest running record for the mine site from 1971 until being decommissioned in November 2011.

Seasonal snowpack depth is not measured at the L-L Dam weather station. Instead, monthly measurements at the Highland Valley snow survey station (Station No. 1C09A) near the Trojan TSF are used to track the changes in snowpack. The measurements are sorted by survey period (the first of January through May) to compare snowpack depths (in snow-water equivalent (SWE) around the same time each year. Historical average and 2017 snowpack depths based on available records are summarized in Table 4.3.

The following observations were noted for 2017:

- May through August appear noticeably drier than average. No data was missing during this time
- On an annual basis, precipitation at the L-L Dam weather station was 28% lower than normal (at Lornex).
- Snowpack depths were not measured for the January 1st or February 1st survey periods. The March 1st, April 1st, and May 1st snowpack depths (in SWE) were 30%, 50%, and 271% greater than average, respectively.

During freshet, a period of rainfall followed by a sudden increase in temperature (Figure 4.2) triggered greater than normal surface runoff on site and in the region starting May 5, 2017. Available records also show the snowpack depth (in SWE) near the Trojan TSF was 3.7 times greater (relative percent difference = +271%) than average for that time of year. The combination of available snowpack and rapid melt-inducing changes led to a more severe freshet in 2017 than normal. Observations and actions in response are discussed in the relevant sections of this report.



Table 4.2 Monthly Precipitation in 2017

| Month | Precipitation (mm) | | | | |
|--------------|----------------------|----------------|--|--|--|
| ivionth | L-L Dam – Unadjusted | Lornex Normals | | | |
| January | 13.5 | 27.5 | | | |
| February | 37.1 | 21.0 | | | |
| March | 21.2 | 16.7 | | | |
| April | 34.6 | 21.3 | | | |
| May | 15.6 | 41.3 | | | |
| June | 5.9 | 47.9 | | | |
| July | 2.8 | 43.5 | | | |
| August | 10.8 | 31.7 | | | |
| September | 28.8 | 31.2 | | | |
| October | 33.5 | 30.0 | | | |
| November | 46.2 | 40.4 | | | |
| December | 34.8 | 40.8 | | | |
| Annual Total | 284.7 | 393.3 | | | |

Figure 4.1 Monthly Precipitation in 2017 and Climate Normals

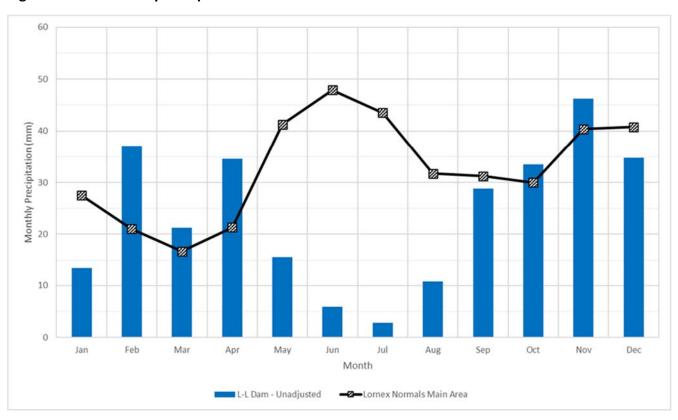


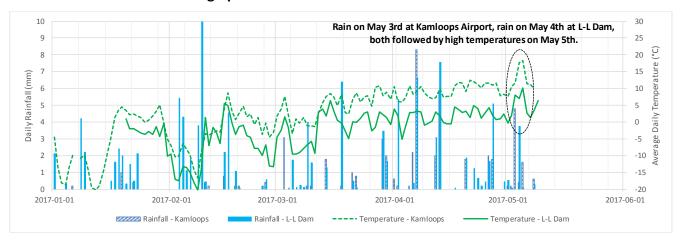
Table 4.3 Historical Average and 2017 Snowpack Depths

| Survey Period | Years of Record ⁽¹⁾ | Historic Average Snowpack Depth ⁽²⁾ (mm SWE ⁽³⁾) | 2017 Snowpack Depth (mm SWE ⁽³⁾) | Relative Percent Difference (%) |
|--------------------------|-----------------------------------|---|---|---------------------------------|
| January 1st | 11 | 50.2 | Not surveyed | N/A |
| February 1 st | 25 | 83.5 | Not surveyed | N/A |
| March 1 st | 51 | 89.5 | 116(4) | +30% |
| April 1st | 51 | 96.3 | 144 ⁽⁴⁾ | +50% |
| May 1 st | 48 | 27.3 | 101 ⁽⁴⁾ | +271% |

Notes:

- 1. At the Highland Valley snow survey station (Station No. 1C09A) near the Bethlehem TSF. Data prior to 1966 was not included as the station was moved to its current location in 1965.
- 2. Calculated based on available period on record.
- 3. SWE = snow water equivalent.
- 4. The March 1st survey was conducted on March 4, 2017. The April 1st survey was conducted on April 3, 2017. The May 1st survey was conducted on May 3, 2017.

Figure 4.2 Daily Rainfall and Average Temperature at Kamloops Airport and L-L Dam Climate Stations Leading up to Freshet



4.3 Water Balance

THVCP manages and tracks the annual water balance for the Highland TSF. Table 4.4 is a summary of annual inflows and outflows, provided by THVCP. The water balance is based on simplified modelling results and therefore the values should be treated as indicative only. As a check, THVCP also calculated the difference in pond volume at the end of 2017 and 2016 using weekly pond elevations combined with a stage-storage curve (Table 4.5). Both modelled and measured values indicate the Highland TSF was operated with a net gain for the 2017 calendar year; however, there is a 0.2 Mm³ (approximate) difference between the two values. This discrepancy can be partially attributed to the challenges of capturing seasonal trends which are dependent on climatic conditions from previous months (e.g. highs and lows of temperature or precipitation in a year) when modelling over a calendar versus a hydrologic year. This effect can be exacerbated during "extreme" climatic periods such as the more severe, melt-induced freshet in 2017.

Table 4.4 Annual Water Balance for Highland TSF

| Item | Volume in 2017 (Mm³) | | | | |
|--|-------------------------|--|--|--|--|
| Inflows | | | | | |
| Runoff ⁽¹⁾ | 5.67 | | | | |
| Consolidation water | 2.87 | | | | |
| Water in tailings through H-H | 94.30 | | | | |
| Outflow from Jim Black Lake | 0.82 | | | | |
| Groundwater | 0.64 | | | | |
| Spatsum | 0.58 | | | | |
| Total inflow: | 104.88 | | | | |
| Outflows | | | | | |
| Evaporation ⁽²⁾ | 7.40 | | | | |
| Seepage to Pukaist Creek | 0.77 | | | | |
| Entrainment | 17.24 | | | | |
| Reclaim to Highland Mill | 78.32 | | | | |
| Total outflow: | 103.73 | | | | |
| Balance | | | | | |
| Water gained from impoundment (inflow minus outflow) from end of 2016 to end of 2017 | 1.15 | | | | |

Notes:

Evaporation was assumed to be

Table 4.5 Change in Pond Volume in 2017 for Highland TSF

| | Volume (m³) |
|---|----------------|
| Pond volume on Dec 27, 2016 | 13,586,069 |
| Pond volume on Dec 21, 2017 | 13,041,160 |
| Change in pond volume (2017 minus 2016) | -544,909 |
| Volume correction ⁽¹⁾ | 1,916,813 |
| Change in pond volume (with correction) | 1,371,904 |

Notes:

4.4 Flood Management

The summary of flood management structures and the applicable design criteria and details for the seven facilities are given in Table 4.6 below with the following discussion points noted:

- The Highland TSF is designed to contain the required return period storm volume with a storm duration (5-day) greater than the minimum required (72-hour) by the Code.
- The remaining water containment structures, except the Seepage Water Reclaim Pond and Seepage Pond 2, are designed to safely route or store, with appropriate freeboard, storm

Adjusted precipitation values from Shula Flats weather station used in water balance model.2.
 570 mm/year.

^{1.} The bathymetry used to generate the stage-storage curve for the Highland TSF was updated in September which resulted in a 1,916,813 m³ difference compared to the previous stage-storage curve. This difference was added to the change in pond volume.

^{2.} The discrepancy between modeled and observed volume changes (i.e. between Table 4.4 and 4.5) can be partially attributed to the challenges of modeling over the calendar year rather than the hydrological year.

events greater than or equal to the IDF required by the Code and recommended by CDA (2013).

- Slimes Pond 1 design event return period is equal to the IDF recommended under the Code; however, the duration (24-hour) is less than the minimum recommended if the IDF is to be stored rather than routed (72-hour). Slimes removal and backfilling of the pond is ongoing as part of construction and the pond will be decommissioned in 2018; therefore, no further action is required.
- The Seepage Water Reclaim Pond relies on pumping to maintain water levels year-round, and in its current state, has insufficient capacity to store the IDF (KCB 2018b). If the Seepage Water Reclaim Pond capacity is exceeded it would initially flood the nearby area potentially impacting THVCP operations but not initially result in an off-lease discharge. KCB had recommended that THVCP should advance the design of preferred options described in the 2018 flood routing assessment (KCB 2018b), or suitable alternative. KCB understands that this work is ongoing. Measures would include:
 - Maximizing diversion of surface runoff from undisturbed catchments as part of normal operating protocol by:
 - Closing the Jim Black Lake outlets that report to SWRP during freshet and periods of sustained or extreme rainfall.
 - Diverting flow within the Woods Creek Diversion system from the SWRP during freshet and periods of sustained or extreme rainfall.
 - Increasing the pumpback capacity from 0.38 m³/s to sufficient capacity to manage the IDF (~0.8 m³/s), and provide redundancy.
 - As part of future L-L Dam seepage collection and slimes ponds expansion for ultimate conditions, consider:
 - Increasing the storage capacity of the SWRP through construction of larger seepage ponds (in-place or upstream) – to be operated as surge ponds with low-level outlets and low operating pond levels, as recommended in KCB (2018c).
 - Opportunities to divert non-contact runoff from undisturbed catchments north of Highway 97C and Slimes Pond 3.

Given the scope of work required, THVCP and KCB set a timeline and milestones for undertaking the upgrade works: design in 2018; permit and IFC Drawings in 2019 and construction in 2020. In the interim, THVCP should develop and implement an interim plan involving pumping to manage the IDF without off-lease discharge until upgrade works are complete. This plan will be incorporated into the OMS manual.

In its current state, Seepage Pond 2 does not meet the minimum freeboard requirement (0.5 m) during the IDF (KCB 2018b). In 2016, KCB recommended THVCP commission works to lower the outlet pipe intake and re-grade the pipe to achieve a consistent downward grade from inlet to outlet. KCB understands this work was not completed in 2017. Per the 2018 flood

routing assessment (KCB 2018b), by maintaining the existing inlet invert elevation (1114.4 m), the system has capacity to route the IDF with sufficient freeboard provided the water level is not "perched" at the top of the existing bend (El. 1114.8 m), and the hydraulic capacity of the pipe is not compromised by the initial upward segment. KCB recommends this work be carried out as part of 2018 construction activities at the L-L Dam.

Table 4.6 Inflow Design Flood Requirements for Highland Facility

| | Outfall Type | Consequence Classification | Inflow Design Flood | Design Event (Precipitation Depth, Design Flow/Volume) | IDF stored or routed? | Reference |
|-------------------------------|-----------------|-------------------------------|--|--|-----------------------|--------------|
| L-L Dam | None | Extreme | PMF ⁽¹⁾ | 5-day PMF (423 mm ⁽²⁾ , 49.4 Mm ³) | Stored | (KCB 2010) |
| H-H Dam | None | Very high | 2/3 between 1000- year and PMF ⁽¹⁾ | 5-day PMF (423 mm ⁽²⁾ , 49.4 Mm ³) | Stored | (KCB 2010) |
| Seepage Pond 2 | Pipe | Significant | Between 100-year and 1000-year ⁽³⁾ | 100-year 24-hour (75.5 mm ⁽⁸⁾ , 1.9 m ³ /s) | Routed (Note 9) | (KCB 2018b) |
| Slimes Pond 1 | None | Significant | Between 100-year and 1000-year ⁽³⁾ | 100-year 24-hour (59 mm ⁽⁴⁾ , 14,300 m ³) | Stored | (KCB 2015a) |
| Slimes Pond 2 | Pipe | Significant ⁽⁵⁾ | Between 100-year and 1000-year ⁽³⁾ | 100-year 24-hour (75.5 mm ⁽⁸⁾ , 0.2 m ³ /s) | Routed | (KCB 2018b) |
| Seepage Water Reclaim Pond | Pump | Significant | Between 100-year and 1000-year ⁽³⁾ | 100-year 24-hour (75.5 mm ⁽⁸⁾ , Note 7) | Routed (Pumped) | (KCB 2018b) |
| 24 Mile Lake | None | Low | 100-year ⁽³⁾ | 24-hour PMF (210 mm ⁽⁶⁾ , 2.2 Mm³) | Stored | (AMEC 2014a) |

Notes:

- 1. Per the Code (MEM 2016) for tailings dams.
- 2. Includes 367 mm of probable maximum precipitation (PMP) from the Environment Canada Highland Valley Lornex climate station (Station No. 1123469), and 56 mm of average annual snowmelt runoff estimated using average annual runoff from nearby streamflow stations.
- 3. Per the Code (MEM 2016) for water dams.
- 4. Based on data from the Environment Canada Highland Valley Lornex climate station (Station No. 1123469) with adjustment factor applied (KCB 2015a).
- $5. \quad \text{As assigned during the Dam Classification Workshop held in February 2015}.$
- 6. Based on data from the Environment Canada Highland Valley Lornex climate station (Station No. 1123469) (AMEC 2014b).
- 7. Refer to discussion in Section 4.4 above.
- 8. Based on Golder (2016).
- 9. Seepage Pond 2 does not have sufficient capacity to route the IDF in its current state. See discussion above for recommendations to meet minimum freeboard requirements.

4.5 Freeboard

Where available, the minimum freeboard¹ measured during 2017 based on either the DSI site visit observations and/or the latest survey, are estimated in Table 4.7. The Code specifies that an evaluation of available freeboard in excess of the peak design flood level is required but defers to CDA (2013) for freeboard design standards. The following items are of note:

There is no permanent pond at the H-H Dam and the crest is more than 10 m higher than L-L
 Dam; therefore, overtopping is not a credible failure mode. The minimum freeboard, or

¹ The vertical distance between the pond level and the low point of the dam crest.



"buffer" for the H-H Dam reported in Table 4.7 refers to the vertical distance between the top of the tailings and the dam crest (KCB 2017b).

- No minimum freeboard assessment has been completed for 24 Mile Lake based on CDA (2013). However, the available freeboard is expected to exceed the minimum requirement because the available storage (11.8 Mm³ based on the May 2017 bathymetric survey) is far in excess of the estimated 24-hour PMF volume of 2.2 Mm³ (AMEC 2014a) and, therefore, is also far in excess of the IDF volume corresponding to the 100-year.
- Seepage Water Reclaim Pond is projected to be overtopped during the IDF and therefore does not meet minimum freeboard requirements (KCB 2018b). Seepage Pond 2 is projected to not meet minimum freeboard requirements. Refer to Section 4.4 for KCB recommendations.
- All other facilities meet freeboard requirements during the IDF.

Table 4.7 Minimum Available Freeboard in 2017

| Dam | Required Minimum Freeboard ⁽¹⁾ | Minimum Available Freeboard in 2017 | Freeboard Surveyed/Visually Estimated | |
|----------------------------|--|--|---|--|
| L-L Dam | 2 m | 12.5 m | Annual minimum from surveys | |
| H-H Dam | 1 m ⁽²⁾ | 1 m ⁽²⁾ | Annual minimum from surveys at Sta. 0+600 | |
| Seepage Pond 2 | 0.5 m | 1.5 m | At time of site inspection | |
| Slimes Pond 1 | 0.5 m | 5 m | At time of site inspection | |
| Slimes Pond 2 | 0.5 m | Pond was empty | At time of site inspection | |
| Seepage Water Reclaim Pond | 0.5 m | 2.3 m | Annual minimum from surveys | |
| 24 Mile Lake | Note 3 | 20 m | Annual minimum from surveys | |

Notes:

^{1.} The vertical distance between the peak pond level during a flood event and the low point of the dam crest to accommodate wave runup and setup per CDA (2013).

^{2. &}quot;Buffer" measured between top of tailings surface and effective H-H Dam crest.

^{3.} No minimum freeboard assessment completed; see discussion above for details.

5 REVIEW OF MONITORING RECORDS AND DOCUMENTS

5.1 Monitoring Plan

The Operation, Maintenance and Surveillance (OMS) manual, including the Emergency Preparedness and Response Plan (EPRP), was updated in 2018 (THVCP 2018) and will be issued by THVCP to MEM. The 2018 update will supersede the versions submitted to MEM in December 2016 (THVCP 2016) and included the recommended items from the 2016 DSI.

- Review structure of report and include any data which is updated on an annual basis as an appendix which can easily be updated, rather than text in the body of the report (e.g. tailings production schedule, threshold levels, groundwater chemistry);
- Include a list of named individuals for each of the main roles of responsibility as an appendix to the OMS instead of in the main body of the text, to make it easier to update on a yearly basis;
- Include a plan(s) showing the location of all the facilities associated with the Highland TSF (seepage ponds, slimes ponds, inflows, outflows etc.);
- Review and update recommendations on minimum beach widths;
- Review and update the failure mode assessment;
- Update definition of event-driven dam inspections to include the following:
 - Exceedance of piezometric and/or dam movement instrumentation thresholds as discussed in Sections 5.4 to Section 5.6;
 - Earthquake greater than magnitude 5, within 100 km of the site; and
 - Rainfall event greater than the 10-year, 24-hour duration storm.

The 2018 manual meets the intent of the Mining Association of Canada (MAC 2011) and CDA (2014) guidelines, is current and provides adequate coverage for existing conditions.

5.2 Inspections

The Highland TSF monitoring program includes the following visual inspections:

- Annual DSI (this report) completed by the EoR to comply with Section 10.5.3 of the Code and submitted to MEM.
- Routine routine monitoring frequencies are summarized in Table 5.1. Summaries of dam inspection observations are provided to the EoR typically on a monthly basis and have been reviewed as part of this DSI. Inspections by THVCP staff have been completed at the prescribed frequencies during periods of the year when dams are accessible. KCB carried out quarterly dam inspections of the L-L Dam and H-H Dam on March 30th, May 28th and



December 20th/21st (checklists from these quarterly inspections are included in Appendix I-E). The annual dam safety inspection in September constitutes one of the quarterly inspections.

- Event-driven completed by THVCP staff in response to unusual conditions and the threshold exceedances listed below (included in the 2018 OMS manual). EoR participation is determined on a case-by-case basis. Details of exceedances and/or unusual conditions, including trigger, response, and remediation / actions taken, are documented in event-driven monitoring review memorandums (MRMs) for record:
 - Exceedance of piezometric and/or dam movement instrumentation thresholds as discussed in Sections 5.4 to Section 5.6;
 - Earthquake greater than magnitude 5, within 100 km of the site; and
 - Rainfall event greater than the 10-year, 24-hour duration storm.

Threshold exceedances are summarized in the appropriate sections (5.3 to 5.5). In 2017, the following event-driven inspections were triggered:

April 2017:

- On April 8 around 4:30 PM, a leak developed in one of the L-L Cyclone House gland water lines, resulting in localized ponding on the till core and erosion of the downstream fill. The affected area was limited to the south abutment, from approximately Sta. 1+000 to Sta. 1+030. The incident was identified by a THVCP dam inspector during a routine inspection who notified appropriate departments. Mill Operations mobilized to repair the leak by evening of the same day, and a THVCP dam inspector monitored the ponding/erosion.
- On April 9, the THVCP dam inspector visually estimated the single erosion gully to be 4 m at the widest and 2 m at the deepest. KCB reviewed the erosion and determined there was no significant impact to dam safety, and the area was repaired.
- In response to this incident, THVCP upgraded surface drainage to divert similar flows into the impoundment rather than to the dam.

May 2017:

- On May 4 and 5, severe freshet conditions (water levels approaching discharge / Level 1 thresholds, localized flooding) triggered increased site-wide monitoring efforts.
- On May 5, Mr. Darryl Pongracz of KCB inspected an area of ponding against the north side of the NBB. Historically, ponding has been observed in the same area due to blockage of a culvert connecting the toe seepage collection ditch to a major finger drain under the NBB. Inflows that exceed the capacity of the major finger drain also tend to pond and draw down over time, which is likely the case during this inspection during freshet. No erosion damage was noted on the slopes of the NBB immediately following freshet, nor during the DSI.



 On May 6, Mr. Rick Friedel (EoR designate) of KCB accompanied THVCP to inspect the Highland TSF, amongst other structures, via helicopter fly-over. No immediate dam safety concerns were noted during this fly-over.

Table 5.1 Routine Monitoring Frequency

| Dam | Inspection by THVCP | Inspection by KCB (EoR or designate) |
|------------------------------------|---------------------|--------------------------------------|
| L-L Dam | Weekly | Quarterly |
| H-H Dam | Weekly | Quarterly |
| 24 Mile Lake | Quarterly | Quarterly |
| L-L Dam Seepage Pond 2 | Monthly | Quarterly |
| L-L Dam Slimes Pond 1 | Monthly | Quarterly |
| L-L Dam Slimes Pond 2 | Monthly | Quarterly |
| L-L Dam Seepage Water Reclaim Pond | Monthly | Quarterly |

5.3 Reservoir Levels / Tailings Buffer

5.3.1 Tailings Pond at L-L Dam

The Highland TSF pond elevation was measured weekly (minimum) during 2017 in compliance with the OMS manual. Pond elevations are shown in the piezometric figures included in Appendix V.

During 2017, the pond elevation varied from a minimum of El. 1241.4 m in March to a maximum of El. 1248.0 m in June. The seasonal changes in the pond elevation in 2017 are approximately 78% greater than previous years which can be attributed to wetter than normal conditions during freshet. Since 2011, the typical pond elevation throughout the year has increased by around 1 m per year. Typically, pond levels remain relatively constant from June to November, decrease from December to February, and rises during freshet from mid-February to June. In 2017, the same general trend was observed but with a much higher rise in pond level during the freshet period due to excessive snowmelt observed that year. There were no required changes to the water management plan in 2017.

5.3.2 Tailings Beach at H-H Dam

There is no permanent free water pond against H-H Dam during normal operating conditions and hence, there are no specific freeboard reporting requirements defined in the M-11 and PE-376 permits. However, the 2018 OMS update specifies that the tailings beach at the H-H Dam needs to be maintained a minimum of 1 m below the effective dam crest to prevent erosion and overrunning of tailings onto the dam crest during active tailings deposition. This "1 m buffer zone" applies to a situation that can be controlled by operational changes (e.g. relocating discharge points) and is not the same as conventional freeboard requirements associated with a free water pond for protection against wave run-up and flood surcharge. The "effective dam crest" is measured to the top of confinement for tailings, whether that is provided by the top of the Glacial Till core, rockfill or any berms that are along the upstream edge of the crest.

In 2017, the tailings beach was surveyed weekly, per the OMS manual. In June 2017, the tailings beach at Lines 6, 7 (discharge points ~450 m northwest of the East Abutment) inundated the tailings pipelines, which required re-routing the discharge to the lines at Sta. 2+200 m. This resulted in a relatively rapid rise of the tailings surface at that location to within ~1.1 m of the dam crest at this East Abutment. The EoR was notified and a decision was made in conjunction with THVCP to construct an approximately 1 m high berm of Zone RF1 fill on the upstream edge of the crest to increase the available buffer until the planned 2017 raise of the dam could be completed.

At the same time, KCB recommended that THVCP commission studies to increase the flexibility and capability of the tailings deposition system to accommodate similar operational upsets in the future. These studies are ongoing.

5.3.3 24 Mile Lake

The current available storage volume is far in excess of the estimated IDF volume (see Section 4.5). In 2017, the lake level was surveyed weekly which meets the minimum required per the 2018 OMS update.

5.3.4 L-L Dam Seepage Collection and Slimes Ponds

Of the five seepage collection and slimes ponds at the toe of the L-L Dam, only Seepage Water Reclaim Pond is routinely surveyed to track pond levels. In 2017, surveys were conducted weekly which meets the minimum frequency required in the OMS manual. The remaining seepage collection and slimes ponds are visually inspected monthly at which time the dam inspector estimates the available freeboard.

The 2018 OMS update specifies minimum freeboard required for each pond under flood conditions, as summarized in Table 4.7. KCB recommends expanding the routine pond level survey activities to include Seepage Pond 2, Slimes Pond 2, and Slimes Pond 3 (when its construction is expected to be completed) on a monthly basis, per the 2018 OMS update.

5.4 Piezometers

5.4.1 Overview

THVCP's Tailings and Water team are responsible for collecting, processing, documenting and distributing instrumentation readings. Piezometric readings are compared to threshold values included in the OMS manual. Information is provided to KCB for routine monthly reviews, or as required (e.g. threshold exceedances).

All piezometers at the L-L and H-H Dams are required to be monitored by THVCP on a monthly basis, when accessible. This requirement was generally met during 2017. Several instruments located on the slope of the dam are unsafe to access during winter conditions, therefore monthly reading frequency is not required and these instruments were read when safely accessible. In 2016, THVCP began to implement a program of automated reading collection and remote monitoring of vibrating wire piezometers at the L-L and H-H Dams using the Navstar GeoExplorer.



KCB reviews the piezometer data provided by THVCP on a monthly basis and summarizes the observations in a Routine Monitoring Review (RMR) memo. The primary objective of these RMRs is to provide a succinct summary of the surveillance results for the period under consideration. Nine RMR's were issued in 2017 (two of the issued RMR's included review of multiple months of data).

In addition to RMRs, any piezometer threshold exceedances are reviewed by KCB and reported in Monitoring Review Memos (MRMs) which document the occurrence, response and recommendations associated with specific threshold exceedances in instrument readings. During 2017, KCB issued 11 MRM's which covered piezometer threshold exceedances (discussed further in section 5.4.2 and 5.4.3 below).

5.4.2 L-L Dam

There are currently 215 piezometers (combination of standpipes and vibrating wire piezometers) at the L-L Dam (as shown on Figure V-1 in Appendix V), of which:

- 26 of the standpipe piezometers are currently plugged;
- 28 of the standpipe piezometers are dry;
- 13 are located in the vicinity of the seepage ponds and slimes ponds at the toe of the L-L Dam;
 and
- 60 vibrating wire piezometers and 15 standpipe piezometers were recently installed during two separate drilling programs throughout 2017. The first program (Highland TSF Phase 1) was completed in 2017. The second program (Highland TSF Phase 2/3) began in 2017 and will be completed in 2018.

Figures of piezometric levels are included in Appendix V. The figures are grouped by dam segment and stratigraphy, as shown in Figure V-1. Each figure is further subdivided into 'a' and 'b', whereby 'a' illustrates the long term piezometric trends for the dam segment, and 'b' illustrates the piezometric trend over the past five years.

Cross sections through the dam, showing geology, pond elevations, piezometric readings and cumulative displacement profiles from inclinometers are included in Appendix VI.

For the purposes of this 2017 DSI, the 2017 threshold levels have been applied for review of the data. Updated threshold levels have been issued for use during the upcoming 2018 construction season (KCB 2017c).

Pore pressure trends in relation to threshold levels for piezometers within each dam segment are discussed below. In 2017, there were six Level 1 exceedances (documented in MRMs). Typical actions following Level 1 exceedances included increasing reading frequency, visual inspection by THVCP dam inspectors, review of data by the EoR or designate, and updating the threshold values as deemed appropriate.



North Abutment (North of Sta. 3+400 m)

- Figure V-2 to Figure V-4 in Appendix V show piezometric levels at the North Abutment over time.
- Figure VI-2 in Appendix VI shows a cross section of piezometric levels through the dam at Sta. 3+630.
- In general, the piezometers both upstream and downstream at the North Abutment show a historic trend of fluctuating elevations with changes in pond elevation, with occasional spikes which are generally attributed to hydraulic cyclone sand placement. Levels recorded in 2017 are consistent with this historic trend; with some of the downstream and upstream piezometers showing a response to beach construction at the end of 2017.

North Dam (Sta. 2+800 m to 3+400 m)

- Figure V-5 to Figure V-6 in Appendix V show piezometric levels at the North Flank over time.
- Figure VI-3 in Appendix VI shows a cross section of piezometric levels through the dam at Sta. 3+300.
- In general, upstream piezometers at the North Dam show a historic muted trend of fluctuating elevations with changes in pond elevation, whereas downstream piezometers have shown minimal change in elevation since 2012, with occasional spikes which are generally attributed to hydraulic cyclone sand placement. Levels recorded in 2017 are consistent with this historic trend, with the exception of the following:
 - One Level 1 exceedance was triggered in P04-2 during freshet in early May. KCB reviewed
 the exceedance and concluded that it was related to ponding due to freshet runoff. The
 threshold exceedance was not considered a dam safety concern and details on the
 response and KCB's recommendations were recorded in an MRM.

North Buttress Berm (Sta. 2+400 m to 2+800 m)

- Figure V-7 to Figure V-10 in Appendix V show piezometric levels at the NBB over time.
- Figure VI-4 in Appendix VI shows a cross section of piezometric levels through the dam at Sta. 2+564.
- In general, piezometers upstream at the North Buttress show a historic muted trend of fluctuating elevations with changes in pond elevation, whereas downstream piezometers have shown minimal change in elevation since the 1990's, with occasional spikes which are generally attributed to hydraulic cyclone sand placement. Levels recorded in 2017 are consistent with this historic trend.
- There were no threshold exceedances in piezometers at the NBB during 2017.

Valley Buttress Berm Extension (Sta. 2+100 m to 2+400 m)

- Figure V-8 to Figure V-12 in Appendix V show piezometric levels at the VBBE over time.
- Figure VI-5 in Appendix VI shows a cross section of piezometric levels through the dam at Sta. 2+250.
- In general, downstream piezometers at the VBBE have shows minimal change in elevation since the mid 1990's, with occasional spikes which are generally attributed to hydraulic cyclone sand placement. Levels recorded in 2017 are consistent with this historic trend, with the exception of the following:
 - One Level 1 exceedance was triggered in standpipe piezometer P92-1B. A review of the data indicated a large discrepancy between the readings in P92-1B and readings in two other piezometers (P92-1A and P92-1C) which are located within the same sedimentary bed in the same drillhole. Therefore, KCB concluded there was no dam safety concern and recommended that this piezometer be abandoned. KCB's recommendations were recorded in an MRM.

Valley Buttress Berm (Sta. 1+500 m to 2+100 m)

- Figure V-13 to Figure V-16 in Appendix V show piezometric levels at the VBB over time.
- Figure VI-6 in Appendix VI shows a cross section of piezometric levels through the dam at Sta. 1+850.
- In general, upstream piezometers at the VBB show a historic trend of fluctuating elevations with changes in pond elevation, while downstream instruments have historically remained constant since the late 1980's. Levels recorded in 2017 are consistent with this historic trend, with the exception of the following:
 - One Level 1 exceedance was triggered in P10-4. Following a review of available monitoring data, KCB concluded that the falling head test carried out in August 2016 cleaned out the piezometer screen and it is now responding to piezometric levels in the Glacial Till, as levels are consistent with other nearby piezometers installed in the Glacial Till. The threshold exceedance was not considered a dam safety concern and details on the response and KCB's recommendations were recorded in an MRM.
 - A number of new instruments have been installed in the VBB since 2016, and as such, long-term trends in these instruments are not yet developed.

South Dam (Sta. 0+800 m to 1+500 m)

- Figure V-14 to Figure V-18 in Appendix V show piezometric levels at the South Dam over time.
- Figure VI-7 in Appendix VI shows a cross section of piezometric levels through the dam at Sta. 1+200.
- In general, upstream piezometers at South Dam show a historic trend of fluctuating elevations with changes in pond elevation, while downstream instruments have historically remained

constant since the late 1980's. Levels recorded in 2017 are consistent with this historic trend, with the exception of the following:

 P02-1 P02-2 and P02-3, all installed in Glacial Till fill within a trench cut into the granodiorite bedrock, have shown a consistent rise in piezometric elevation since 2009.
 The quality of these instrument installations was considered questionable, and therefore a new piezometer, VWP17-03A, was installed in 2017 at this location for validation.

5.4.3 H-H Dam

There are currently 38 piezometers at the H-H Dam (as shown in Figure V-19), of which:

- 5 standpipe piezometers are currently plugged;
- 3 standpipe piezometers are dry; and
- 2 standpipes and 26 vibrating wire piezometers were recently installed throughout 2017 at the H-H Dam (KCB 2018). Fourteen of the 28 new instruments have not been read since installation late in the year, and therefore are not shown in Appendix V.

Figures of piezometric levels are included in Appendix V. Cross sections through the dam, showing geology, pond elevations, piezometric readings and cumulative displacement profiles from inclinometers are included in Appendix VI.

Updated threshold levels for 2017 were issued for the piezometers at the H-H Dam in 2016 (KCB 2016j), based on a review of the performance history of the dam (KCB 2016k). These levels, with the exception of VWP15-23B, will be reviewed and updated, where necessary, during the H-H Dam Design Review, scheduled for completion mid-2018. The threshold for VWP15-23B was revised in November 2017 to reflect the historical trend based on the increased period of record and three exceedances during fill placement over the year. For the purposes of this 2017 review and moving forward for 2018 monitoring, the 2016 threshold levels (defined for 2017) are considered appropriate given the lack of noted exceedances, and have been applied.

In 2017, there were three Level 1 exceedances, all at VWP15-23B in relation to fill placement and/or upstream tailings deposition activities, as summarized below:

- January to 2017 March 2017:
 - Between November 2016 and March 11, 2017, THVCP placed fill on the H-H Dam to raise the RF2 fill zone (located downstream of the core) to elevation 1277 m between the right (southwest) abutment and Sta. 1+600. In early December 2016, readings in piezometer VWP15-23 B, installed in a glaciolacustrine unit in the foundation exceeded the Level 1 thresholds.
 - Based on KCB's review of the available data, KCB and THVCP agreed on December 6, 2016
 to suspend fill placement until the instrument readings indicated stabilizing and/or
 attenuating trends. Regular reviews of the instrument readings collected by THVCP
 following cessation of fill placement, were carried out by KCB.

- Fill placement re-started on January 18, 2017 and continued throughout February and March under close stewardship through frequent communications between THVCP and KCB as well as regular instrumentation data reviews.
- Placement of the final 3 m lift of fill to EL. 1277 m was completed on March 11, 2017.
- By March 25, 2017, piezometric levels in VWP15-23B had dropped below the Level 1 threshold.
- KCB concluded that the threshold exceedances were in response to RF2 fill placement, and were not considered an immediate dam safety concern. However due to the lack of performance history data available for the H-H Dam, a precautionary approach was taken to ongoing fill placement.
- July to October 2017:
 - From late May 2017, the piezometric level in VWP15-23 B began to steadily increase again and exceeded the Level 1 threshold on July 3rd, 2017.
 - VWP15-22, which is installed upstream of HHVWP15-23B within the dam fill sand and gravel recorded a rise in piezometric level of almost 5 m over the same time period as the response seen in VWP15-23 B.
 - KCB reviewed the data and concluded that the response in both instruments most likely reflected active tailings beach deposition.
- October to November 2017:
 - During October 2017, the piezometric level in VWP15-23 B began to steadily increase again and exceeded the Level 1 threshold.
 - The timing of the piezometric level rise coincided with the start of dam crest raise works at the H-H Dam in mid October 2017.
 - Following this latest threshold exceedance, and based on a review of its response to the
 past year's fill placement activities, KCB updated the Level 1 threshold from El. 1205 m to
 El. 1207 m.

The threshold exceedances at VWP15-23 B were not considered a dam safety concern and details on the response and KCB's recommendations were recorded in a series of MRM's.

5.4.4 24 Mile Lake

There are no piezometers installed at 24 Mile Lake.

5.5 Inclinometers

5.5.1 Overview

THVCP's Tailings and Water team are responsible for collecting, processing, documenting and distributing instrumentation readings. Inclinometer readings are compared to threshold values

included in the OMS manual. Information is provided to KCB for routine monthly reviews, or as required (e.g. threshold exceedances).

All inclinometers at the L-L Dam and H-H Dam are required to be monitored by THVCP on a monthly basis, weather permitting. This was generally completed during 2017. Several instruments located on the slope of the dam are unsafe to access during winter conditions, therefore monthly reading frequency is not required and these instruments were read when safely accessible.

KCB reviews the inclinometer data provided by THVCP on a monthly basis and summarizes the main observations a Routine Monitoring Review (RMR) memo. The primary objective of these RMRs is to provide a succinct summary of the surveillance results for the period under consideration. During 2017, KCB issued nine RMR's (two of the issued RMR's included review of multiple months of data).

In addition to RMRs, any inclinometer threshold exceedances are reviewed by KCB and reported in monitoring review memos (MRMs) which document the occurrence, response and recommendations associated with specific threshold exceedances in instrument readings. During 2017, KCB issued three MRM's which covered inclinometer threshold exceedances (discussed further in section 5.5.3 below).

5.5.2 L-L Dam

There are currently 30 inclinometers at the L-L Dam, as shown in Figure IV-1 in Appendix IV. During 2017, 11 inclinometers were installed during two separate drilling programs. The first program (Highland TSF Phase 1) was completed in 2017. The second program (Highland TSF Phase 2/3) began in 2017 and will be completed in 2018. Of the 11 new inclinometers, three have not been read since installation late in the year, and therefore are not shown in Appendix IV.

Figures of cumulative displacement for each inclinometer are included in Appendix IV (Figure IV-1 to Figure IV-20). In addition, Figure IV-21 and Figure IV-22 show rates of displacement in selected key geological units.

Cross sections through the dam, showing geology, pond elevations, piezometric readings and cumulative displacement profiles from inclinometers are included in Appendix VI.

For the purposes of the current DSI review, the 2017 threshold levels have been applied. Updated threshold levels have been issued for use during the upcoming 2018 construction season (KCB 2017c).

In 2017, there were no threshold exceedances recorded in the inclinometers at the L-L Dam. Readings are consistent with historical trends.

5.5.3 H-H Dam

There are currently eight inclinometers and six Sondex settlement systems installed at the H-H Dam, as shown in Figure IV-23. Of these, six inclinometers and six Sondex settlement systems were recently installed during two separate drilling programs. The first program (Highland TSF Phase 1) was completed in 2017. The second program (Highland TSF Phase 2/3) began in 2017 and will be completed in 2018. Three inclinometers and three Sondex settlement systems have not been read since installation late in the year, and therefore are not shown in Appendix IV.



Figures of cumulative displacement for each inclinometer are included in Appendix IV (Figure IV-24 to Figure IV-26). In addition, Figure IV-27 and Figure IV-28 show rates of displacement in key geological units. Cross sections through the dam, showing geology, pond elevations, piezometric readings and cumulative displacement profiles from inclinometers are included in Appendix VI.

Updated threshold levels for 2017 were issued for the piezometers at the H-H Dam in 2016 (KCB 2016j), based on a review of the performance history of the dam (KCB 2016k). These levels will be reviewed and updated, where necessary, during the H-H Dam Design Review, scheduled for completion mid-2018. For the purposes of this 2017 review, the 2016 threshold levels have been applied.

Between January and March 2017, Level 1 exceedances were recorded at HHSAA12-1 and HHI12-1 in response to RF2 fill placement, as follows:

- Between November 2016 and March 11, 2017, THVCP placed fill on the H-H Dam to raise the RF2 fill zone (located downstream of the core) to elevation 1277 m between the right (southwest) abutment and Sta. 1+600. In early December 2016, readings in inclinometers HHSAA12-1 and HHI12-1, exceeded their respective Level 1 thresholds. All of the movements measured in the inclinometers were within the dam fill.
- Based on KCB's review of the available data, KCB and THVCP agreed on December 6, 2016 to suspend fill placement until the instrument readings indicated stabilizing and/or attenuating trends. Regular reviews of the instrument readings collected by THVCP following cessation of fill placement, were carried out by KCB.
- Fill placement re-started on January 18, 2017 and continued throughout February and March under close stewardship through frequent communications between THVCP and KCB as well as regular instrumentation data reviews.
- Placement of the final 3 m lift of fill to EL. 1277 m was completed on March 11, 2017.
- By March 25, 2017, horizontal displacement rates in HHSAA12-1 and HHI15-23 were below the Level 1 threshold.
- KCB concluded that the threshold exceedances in HHSAA12-1 and HHI15-23 were in response to RF2 fill placement, and were not considered an immediate dam safety concern. However due to the lack of performance history data available for the H-H Dam, a precautionary approach was taken to ongoing fill placement.
- Details on the response and KCB's recommendations were recorded in a series of MRM's.

5.5.4 24 Mile Lake, L-L Dam Seepage Collection and Slimes Ponds

There are no inclinometers located at 24 Mile Lake, Seepage Ponds or the Slimes Ponds.



5.6 Seepage

5.6.1 L-L Dam

Seepage is recorded, when accessible, on a weekly basis from four weirs at the toe of the L-L Dam:

- LL—FS-01 records seepage flows from Finger Drain 1 which collects seepage from the west side of the NBB and seepage from the North Abutment. It is recommended that a new weir be installed at the North Flank to enable separate measurement of seepage flow rates from the North Abutment.
- LL—FS-02 records seepage flows from the UVD/LVD drains which collects seepage from the volcanic units beneath the eastern side of the NBB.
- LL—FS-03 records combined flows from seepage at the toe of the VBB (outflowing from the south end of Seepage Pond 1) as well as outflows from Jim Black Lake. Flows from Jim Black Lake can be subtracted, based on measurements at LL-JB-FS-01.
- KCB3 records seepage flows from the VBB. This weir was choked throughout 2017, and no readings were taken by THVCP.

In addition, THVCP measures flows in Pukaist Creek, approximately 3 km southwest of the Seepage Water Reclaim Pond. Flow measurement locations are shown in Figure VII-1, and flow rates are shown in Figure VII-2, in Appendix VII. Seepage rates in 2017 were generally comparable to 2016 rates which are lower than previous years, likely due to the lack of downstream hydraulic cycloned sand construction at the dam in both 2016 and 2017. The sudden increases in May 2017 are attributed to surface runoff during freshet flood conditions.

There is currently no seepage measurement weir at the North Dam, or South Dam major drains. We recommend that weirs be installed at these locations to monitor seepage rates.

5.6.2 H-H Dam

There are no seepage measurement points at the H-H Dam.

5.6.3 24 Mile Lake, L-L Dam Seepage Collection and Slimes Ponds

There are no seepage measurement points at 24 Mile Lake, or at the L-L Dam Seepage Ponds or Slimes Ponds.

5.7 Survey Monuments

Based on a 2016 DSI recommendation, THVCP installed ten survey prisms within the lock-block retaining wall located behind the H-H Pumphouse at the H-H Dam, during 2017. Monitoring of the prisms began on October 4, 2017.

Required monitoring frequency for the prisms is monthly during periods of no construction, increasing to weekly when construction is active between Sta. 1+800 and Sta. 2+200. This was completed between October and November 21, 2017.



KCB understands that since November 21, 2017, the pedestal at the toe of the retaining wall has moved, which has compromised the subsequent readings, but that THVCP plans to modify the method of survey during 2018 to mitigate this issue going forward.

The location of the prisms and displacement plots for 2017 are included in Appendix V-III. The data indicates no clear trend of movement within the wall over the monitoring period. Vertical and horizontal displacement rates were generally < 0.2 mm per day.

5.8 Water Quality

Water quality downstream of the Highland TSF is monitored by HVC monthly to assess the effectiveness of the tailings facility in protecting the downstream receiving environment. A copy of the HVC 2017 Annual Water Quality Monitoring Report (ERM 2018) was provided to KCB for review as part of the DSI. Select observations and findings from the monitoring report are summarized as follows:

- There are six permitted surface water quality monitoring sites in the L-L Dam/Thompson Valley area, as shown on the site monitoring plan in Appendix IX. There are no surface monitoring points in the immediate area downstream of H-H Dam however there are two ground water wells sampled quarterly
- All permit sampling requirements and frequency were met in 2017.
- The 2017 monitoring results were screened against applicable BC Water Quality Guidelines (WQG). However, it should be noted that the WQG are not prescribed performance targets in permit PE-376.
- The receiving environment downstream of the dam indicated some WQG exceedances in 2017 which may be attributed in part to the following factors:
 - Influence from naturally occurring high background levels
 - Particulate transport during the unusually high freshet
 - Seepage from LL Dam and unlined seepage collection ponds
- Longer term trends in the receiving environment downstream of the dam indicate that Sulphate, Fe, Mn, and Mo have been increasing over time, which may be attributed to seepage from the LL Dam and unlined seepage collection ponds.

Further discussion on specific WQG exceedances and water quality trends observed during 2017 can be found in the 2017 Annual Water Quality Monitoring Report (ERM 2018).

THVCP have implemented the Sulphate Adaptive Management Plan to mitigate influence of seepage from the Highland TSF on downstream water quality and increase water available to downstream users in Pukaist creek. This is a multi-year program which includes installation of interception wells downstream of the dam, lining of seepage collection ponds and diversion of non-contact water around the TSF and into Pukaist Creek.



6 VISUAL OBSERVATIONS AND PHOTOGRAPHS

The visual observations made during the DSI site visit are summarized below. Copies of the inspection forms are included in Appendix I. The referenced site inspection photos are included in Appendix II.

L-L Dam

- Crest: Good physical condition. (Photo II-A-11 and Photo II-A-12)
- South Abutment: Good physical condition. (Photo II-A-2, Photo II-A-2 and Photo II-A-22)
- North Abutment: Good physical condition. (Photo II-A-1)
- Upstream Slope: Overall good physical condition. Tailings discharge during beach construction has undercut the upstream slope in a few locations (e.g. Sta. 2+000). The undercutting was observed not to have encroached on the 126 m wide compacted S2 sand berm, therefore it is not a dam safety concern. These occurrences were noted and monitored as part of weekly inspections. (Photo II-A-3 to Photo II-A-7).
- **Downstream Slope:** Overall good physical condition. In 2014, KCB recommended that the locally steepened (upper ~20 m) part of the slope between Sta. 1+050 m and Sta. 3+650 m be re-graded to design (2.5H:1V) during the next (2018) construction season. In most areas, the overall slope angle (from crest to toe) is shallower than 2.5H:1V, and the resultant factor of safety for slope stability is > 1.5, therefore this is not considered a dam safety concern. (Photo II-A-8)
- North Dam: Good physical condition. The North Dam Major Drain was flowing clear at the time of inspection. (Photo II-A-13 to Photo II-A-15)
- NBB: Good physical condition. The area of ponding against the north side of the NBB during freshet was dry at the time of inspection. No significant scour or erosion was observed. (Photo II-A-16 to Photo II-A-17)
- VBB/VBBE: Good physical condition. (Photo II-A-23)
- SBB: Good physical condition.
- South Dam: Overall good physical condition. In April, a ruptured water pipe at the L-L cyclone house led to flow over the downstream slope at the south abutment contact. The resultant erosional features in the downstream slope will be addressed as part of downstream fill placement in 2018. (Photo II-A-9). The Major Drain was flowing clear at less than 1 L/min at the time of inspection, which is consistent with past inspections.
- Tailings Beach: Good physical condition. Some erosion gullies are evident from overflow discharge pipes. These features do not compromise freeboard.
- Pond: At the time of inspection, the pond was 13 m below the crest of the dam (El. 1247.3 m). (Photo II-A-2)
- Toe Collection Ditches: Good physical condition, with no obstructions. In 2016, the ditch upstream of LL—FS-03 which was choked with sediment from erosion of the soft lacustrine



- soils which line the ditch and had filled the base of the weir. This was not cleared during 2017 and will be impacting flow readings in the weir.
- **Seepage:** All toe seepage water was clear and free of sediment. Point A weir (KCB3) was overgrown by vegetation and is no longer functional. THVCP have no plans to re-instate this weir as the area will be cleared as part of foundation preparation in 2018. (Photo II-A-24 to Photo II-A-28).

H-H Dam

- Crest: Overall good physical condition (Photo II-B-1 and Photo II-B-3 to Photo II-B-6):
 - Minor vegetation (grasses) was noted on the crest of the dam in the RF1 and Till Core zones (Photo II-B-3). Vegetation removal should be carried out as part of regular dam maintenance works. This is not considered to be a dam safety issue.
 - Longitudinal cracking up to 20 cm deep was observed in the Till Core Zone along the
 centerline from Sta. 1+200 to Sta. 1+400 (not continuous). This is not considered to be a
 dam safety issue at this time, but should continue to be monitored as part of routine
 inspections, in accordance with the OMS manual. The area of cracking was flagged and
 unsuitable material stripped off prior to fill placement during 2017 construction activities.
 (Photo II-B-5).
 - A pre-existing longitudinal crack (originally observed by THVCP and reported to KCB on April 5, 2017) was observed at the downstream edge of the SG2 zone between ~Sta. 1+500 and Sta. 1+550. No signs of fresh movement or vertical displacement were observed across the crack. Crack is >30 cm deep and up to 20 cm wide with another hairline crack ~ 1 m upstream of the main crack. Details of the history, surveillance and ongoing monitoring of the cracking were previously reported in MRM-2017-004. The cracking was repaired (by digging down and re-compacting the SG2 fill) later in the year during the annual crest raise at the H-H Dam.
- West Abutment: Good physical condition. (Photo II-B-1)
- East Abutment: Good physical condition. The foundation at the abutment has been exposed since the end of 2015 when fill placement was halted at the end of the year. (Photo II-B-2)
- Upstream Slope: Good physical condition. (Photo II-B-7 and Photo II-B-8)
- Downstream Slope: Good physical condition. (Photo II-B-9 to Photo II-B-12 and Photo II-B-14)
- Tailings Beach: At the time of inspection, the available buffer was approximately 1.2 m at its lowest point (Sta. 2+100).
- Seepage: No seepage was observed at the toe of the H-H Dam.
- Lock-Block Retaining Wall: Generally good physical condition. In 2017, survey prisms were installed at the wall. In addition, 1x VWP, 1x inclinometer and 1x Sondex settlement system were installed immediately above the wall. These instruments were installed to monitor potential movements and pore pressures, the data from which will be used in ongoing reviews



of global dam stability by KCB. Local stability of the wall, similar to the original design, is by Others. (Photo II-B-13 and Photo II-B-15)

24 Mile Lake

- Upstream Slopes: Overall good physical condition. Multiple erosion gullies were observed on the west slopes, which should be repaired as part of routine maintenance per the OMS manual. (Photo II-D-1 and Photo II-D-2)
- Outlet Pump: Present but not operational at time of inspection. Pump was not inspected or tested. (Photo II-D-3)
- Inlet: Good condition. Neither water nor tailings inflows were active during the inspection. (Photo II-D-6 and Photo II-D-7)

L-L Dam Seepage Collection and Slimes Ponds

- Seepage Pond 1: Good physical condition. (Photo II-C-4 and Photo II-C-5)
- Seepage Pond 2: Good physical condition. The tear in the pond liner on the east slope was repaired in 2017. It is understood that the outlet pipe re-alignment recommended in 2016 is on hold pending completion of the SWRP flood routing review. (Photo II-C-9 to Photo II-C-12)
- Slimes Pond 1: Good physical condition. (Photo II-C-2 and Photo II-C-3)
- Slimes Pond 2: Good physical condition. Slimes were removed/excavated down to pond bottom in 2017. The protective gravel layer on top of the liner was not observed to have been compromised. (Photo II-C-6 to Photo II-C-8)
- Seepage Water Reclaim Pond: Good physical condition. Algae continues to collect on the surface of the pond and on the screen to the outlet pumps. The Woods Creek Diversion was not flowing into SWRP at the time of inspection. (Photo II-C-13 and Photo II-C-14)



7 ASSESSMENT OF DAM SAFETY

7.1 Dam Classification Review

The consequence classification of each of the dams included in the Highland TSF (as summarised in Table 2.1) is reviewed annually by THVCP and KCB's EoR or designate representative at the time. The 2017 review was performed on January 16, 2018. No change in consequence classification was recommended.

As previously indicated, an assessment was initiated in 2017 to review the potential for declassifying the 24 Mile Lake facility as a "dam" under the Code (KCB 2018c). This work is in progress at the time of this DSI.

7.2 Failure Mode Review

7.2.1 Overview

Based on the DSI and review of available documents regarding the L-L Dam and the H-H Dam, the potential failure modes included in the CDA Dam Safety Guidelines (CDA 2013) were reviewed and discussed below. KCB is aware, and participated in, a more recent Failure Modes and Effects Analysis (FMEA) which is underway by the Wood Group, the results of which will be reviewed as part of the 2018 DSI.

It should be noted that the design, construction and operations of the L-L Dam and the H-H Dam are an ongoing process, and are closely integrated through the application of the Observational Method.

7.2.2 L-L Dam

Overtopping

The Highland TSF is designed to contain the flood volume from the PMF (49.4 Mm³) resulting from the average annual snowmelt runoff plus the 5-day PMP, which is a duration greater than required by the Code. For reference, the minimum measured freeboard during 2017 was 12.5 m, which is greater than the operating freeboard limit of approximately 6.1 m determined for 2017. Crest elevation milestones in the construction schedule are designed to maintain storage capacity for a minimum of one full year in advance. Therefore, under current construction and operating conditions, the likelihood of overtopping is considered negligible.

Internal Erosion and Piping

Based on a 2015 review of filter adequacy (KCB 2015b), the risk of piping related failure through the dam developing at this stage is very low. The dam design incorporates a large upstream cycloned sand berm and a long tailings beach, and a vertical glacial till core supported by a downstream cyclone sand zone that is filter compatible with the core fill. Piezometric levels indicate low hydraulic gradients across the dam filter zones.



At the North Abutment, the glacial till core zone only provides a partial cutoff to seepage flow, due to the deep elevation of impermeable Glacial Till. A review of the risks of internal erosion and piping at the North Abutment was carried out in 2016 (KCB 2017). A review of particle size distributions against measured and predicted hydraulic gradients for existing and ultimate conditions indicated that internal erosion based mechanisms in the North Abutment foundation should not be a concern (KCB 2017, KCB 2017c).

A decommissioned Low-Level Outlet (LLO) remains close to the South Abutment of the dam. The downstream outlet of the LLO was surrounded with filter compatible fills in 2015, in order to mitigate the risks of piping. Therefore, the likelihood of a piping related failure developing around the LLO is very low.

Contaminated Seepage

Release of contaminated seepage may occur if the seepage collection system downstream of the L-L Dam were to fail. This is dependent on the operation and design of the seepage and slimes collection ponds, and is not amenable to the Observational Method. Currently, the system is a closed loop with all outflows from the individual ponds directed to the Seepage Water Reclaim Pond (SWRP) where it is then pumped back to the TSF. To date, this system has performed satisfactorily to meet the range of expected seepage rates through the L-L Dam. The Seepage Water Reclaim Pond relies on pumping to maintain water levels year-round, and in its current state, has insufficient capacity to store the IDF (KCB 2018b). KCB recommend THVCP advance design of preferred options described in the 2018 flood routing assessment, and implement measures during the 2018 construction season to provide sufficient flood management capacity within the system. It is understood that this work is ongoing.

A second pathway for contaminated seepage to leave the TSF is via groundwater flowpaths bypassing the seepage collection system. This failure mode is being assessed separately for the design of seepage mitigation options in support of THVCP's Sulphate Adaptive Management Plan (SAMP).

Stability

The dam is founded on a mix of Lacustrine clays (beneath the Starter Dam in the centre of the valley only), Glacial Till with interbedded Glaciolacustrine deposits, and a mix of granodiorite, volcanic and sedimentary bedrock units. The design of the L-L Dam accounts for the variability of the foundation conditions along its alignment, including buttresses where required by the localized foundation conditions.

As previously indicated, stability analyses are carried out on a regular basis, and especially during planning of each construction raise. In 2017, the influence of the Lower Glaciolacustrine Unit (L-GLU) on the design and performance of the VBB were reviewed considering Most Likely Case (MLC) and Reasonably Worst Case (RWC) conditions (KCB 2017c). The analysis using the MLC parameters represent the Design Case and indicate that the minimum static safety factors are acceptable. The analysis using the RWC parameters represent the contingency case, which is used to size the stabilizing buttress required in the event performance monitoring indicates that there is a potential for RWC conditions to be mobilized in the field. The results have been formerly incorporated into the Observational Method and therefore the likelihood of a dam instability is considered low.



Surface Erosion

The downstream slope of the dam ranges from 1.5H:1V (in localised areas such as access roads) to 3H:1V. Where the slopes are steeper than 2.5H:1V, there is a risk of surface erosion, particularly during snow melt conditions, and erosion features are not uncommon during freshet. Progressive erosion that develops over time or multiple events are managed through routine and event-driven monitoring and ongoing maintenance. The likelihood of surface erosion over the downstream slope resulting in a failure from a single event is negligible.

Earthquakes

Slope stability and deformation of the dam under seismic loading and post-earthquake conditions were assessed in 2010 stability analyses (KCB 2010). The design earthquake assumed for the analysis was a magnitude M6.5 with a peak ground acceleration of 0.32 g corresponding to a 10,000-year return period event. This seismic event is in accordance with the minimum earthquake design ground motion (EDGM) required (10,000-year or maximum credible earthquake (MCE)) under the Code. The likelihood of a seismic-related failure is considered low.

7.2.3 H-H Dam

Overtopping

Overtopping of the H-H Dam is not a plausible failure mode in the current configuration because the crest is 10.5 m higher than the L-L Dam crest on the far side of the impoundment. Therefore, the L-L Dam would be overtopped before the pond reached the H-H Dam crest.

Internal Erosion and Piping

Based on a 2015 review of filter adequacy (KCB 2015b), the likelihood of piping related failure through the dam developing at this stage is low. Of particular relevance, there is no free water pond adjacent to H-H Dam.

Stability

The dam is founded on a mix of Sand and Gravel, Glacial Till with interbedded Glaciolacustrine deposits, and granodiorite bedrock. KCB (2016n) indicated that, although Glaciolacustrine lenses have been identified within the Glacial Till, they are not a concern to stability because of their depth and deep confining overburden layers.

Stability analyses were carried out in 2015 as part of construction staging for the 2016 dam crest of El. 1270 m and calculated static safety factors met the Code requirements (KCB 2015c). This process was repeated in 2017 for the 2017-2019 construction milestones. In November 2017, the impact of the planned 2 m raise on local stability at the lock-block wall (previously not assessed) was reviewed. The stability analysis found the static safety factors to meet minimum Code requirements. Moreover, to date, the instrumentation data has indicated generally satisfactory performance. Therefore, the likelihood of a dam instability is considered low.



Surface Erosion

The downstream slope of the dam is approximately 2H:1V. The majority of the downstream face of the dam comprises rockfill, therefore it is not at risk of erosion. Some rilling of the surface is visible in the lower slopes where sand and gravel fills are exposed in the face. Progressive erosion that develops over time or multiple events are managed through routine and event-driven monitoring and ongoing maintenance. The likelihood of surface erosion over the downstream slope resulting in a failure from a single event is low.

Earthquakes

Slope stability and deformation of the dam under seismic loading and post-earthquake conditions were assessed in 2010 stability analyses (KCB 2010). The design earthquake assumed for the analysis was a magnitude M6.5 with a peak ground acceleration of 0.24 g corresponding to the 5,000-year return period event. The minimum EDGM required (2/3rd between 2,475-year and 10,000 year or MCE) under the Code is 0.26 g, which is not significantly higher than the design earthquake. The likelihood of a seismic-related failure is low.

7.3 Emergency Preparedness and Response

The emergency preparedness and response plan (EPRP) for the Highland TSF forms a part of the OMS manual. KCB understands the 2018 update is in progress and as such, the following discussion will be in reference to the 2016 EPRP.

Training of THVCP staff and contractors who work near the dams is provided by a video presentation which outlines dam safety warning signs that all staff should be aware of and report if any are observed during their work.

In the case of an emergency an incident command center would be established on site to coordinate with regional emergency response organizations and local authorities. The roles and responsibilities of key team members are well defined, along with reporting structures and who is responsible for declaring an emergency and starting the incident response. The EPRP also outlines strategies that could be implemented in the event of several types of dam emergencies.

Training and testing of the EPRP currently is done using desktop scenarios. Along with testing of the system, offsite emergency response resources are contacted regularly to ensure that contact information is still up to date. The emergency reporting contact list is also reviewed and updated as required. A table top exercise to review and update the Emergency Preparedness Response Plan for the HVC site was hosted by THVCP and attended by the EoR on November 20, 2017. The main findings and recommendations have been incorporated into the latest version attached to the 2018 OMS update.

8 SUMMARY

The Highland TSF appears in good physical condition and the observed performance during the 2017 site inspections is consistent with the expected design conditions and past performance. The status of recommendations to address deficiencies and non-conformances identified during past DSIs are summarized in Table 8.1. Closed recommendations are shown in *italics*. Recommendations to address deficiencies and non-conformances identified during the 2017 DSI are summarized in Table 8.2.

As shown, most of the recommendations have either been completed or are included in the planned 2018 dam construction activities. For the purposes of the DSI, these recommendations are considered closed. The only outstanding recommendations are associated with upgrading the Seepage Water Reclaim Pond and the Seepage Pond 2 to bring them back into compliance with management of the IDF and freeboard, as listed in Table 8.2.

Table 8.1 Status of 2016 Recommendations for Deficiencies and Non-Conformances

| ID No. | Recommended Action | Priority ⁽¹⁾ | Recommended Deadline (Status) | | | |
|------------|---|-------------------------|--|--|--|--|
| | L-L Dam | | | | | |
| LL-2015-01 | Downstream sand should be placed in over-steepened upper sections of the downstream dam slopes (between Sta. 1+050 m and Sta. 3+650 m) to the design (2.5H:1V) during the next construction season. Some re-work of the existing loose sand on these slopes will be required as the downstream sand is raised, to ensure these slopes meet compaction/density requirements. | 3 | End of next construction season (Outstanding. Included in construction sequence for next dam raise (2017 to 2018). EoR has reviewed stability and found acceptable factor of safety and no interim dam safety concern.) | | | |
| LL-2015-02 | The overflow lines from the secondary cyclones were buried with un-compacted cyclone sand across the upstream S2 zone during 2015. This sand will require reworking following completion of downstream sand placement during the next construction season. | 3 | End of next construction season (Outstanding. Some of the lines were removed and trenches re-filled with S2 fill during 2017 (refer to section 3.2.2 for details). Remaining will be done as part of ongoing construction) | | | |
| LL-2015-04 | It was noted in the Q4 dam inspection that the north side of the NBB El. 1202 m bench is over-steepened to 1V:1H. This should be re-graded to the design slope gradient to prevent instability. | 4 | During next construction season (CLOSED. This over steepened area is related to an access road cut in one of the benches and potential instability relates to local area and is not a dam safety issue. Area will be remediated during future construction works. EoR has reviewed stability and found acceptable factor of safety and no interim dam safety concern.) | | | |
| LL-2015-06 | We recommend that key design criteria for the Highland TSF, including the probable maximum flood (PMF), freeboard requirements and seismic ground motions be reviewed during 2016. | 4 | December 2016 (Outstanding. Deferred to 2018. There is no interim dam safety concern.) | | | |

| ID No. | Recommended Action | Priority ⁽¹⁾ | Recommended Deadline (Status) |
|------------|--|-------------------------|---|
| LL-2016-01 | Repair P10-4 to prevent inflow of surface water (suspected cause of elevated groundwater readings). Remove metal cover and investigate possible location where surface water could be entering the standpipe. If the original grout backfill to the hole has sunk, then top up with bentonite or cement to seal the standpipe. Ensure ground is sloped away from the piezo to prevent any ponding water. | 4 | May 1, 2017 (CLOSED) |
| LL-2016-02 | Install seepage measurement weirs at the outflow points of the North Dam (Sta. 3+600) and South Dam (Sta. 1+100) major drains to monitor seepage rates. | 3 | December 2017 (Outstanding. Deferred until 2018 when proposed North Dam drain construction is completed) |
| | H-H Dam | | |
| HH-2015-01 | H-H lock-block wall - during operations routine monitoring of the retaining wall condition should be conducted. This includes an annual survey at selected points along the crest and toe of the wall, and monthly photographs taken from the same location to monitor for signs of changes to the wall alignment. | 3 | Ongoing throughout 2016 (CLOSED) |
| HH-2015-02 | H-H lock-block wall - a piezometer should be installed | | December 2016 (CLOSED, VWP installed in 2017) |
| HH-2016-01 | H-H lock-block wall - during operations routine monitoring of the retaining wall condition should be | | May 2017 (CLOSED, duplicate of HH-2015-01) |
| | 24 Mile Lake | | |
| | None. | | |
| | L-L Dam Seepage Collection and S | limes Ponds | |
| SP-2016-01 | In order to meet the design freeboard requirement at Seepage Pond 2, the following is recommended: Iower the existing inlet invert elevation by 0.4 m (from El. 1114.4 m to El. 1114.0 m); re-align the pipe such that the bend after the first 12 m (currently graded upwards) is removed; and maintain or lower the existing outlet invert elevation to achieve a minimum grade of 1%. | 2 | July 2017 (CLOSED, superseded by SP-2017-02) |
| SP-2016-02 | To meet the design freeboard requirement at the Seepage Water Reclaim Pond, review the following items as part of planning and prior to construction activities in 2017: the water quality and discharge requirements to determine an appropriate environmental design flood (EDF) and explore options for discharge during flood events, if permissible; the anticipated timeline for construction and operation of the proposed Slimes Pond 3 which was modeled, as designed, to discharge to Seepage Water Reclaim Pond under flood conditions; | 2 | July 2017 (CLOSED) |

| ID No. | Recommended Action | | Recommended Deadline (Status) |
|------------|--|---|---|
| | review the current and proposed operational requirements for pumping (in relation to capacity and operational elevations); and update, if necessary, the 2015 flood routing assessment based on revised assumptions | | |
| SP-2016-03 | If required, upgrade the water management plan for the Seepage Water Reclaim Pond, based on the updated flood routing assessment (action item SP-2016-02). | 2 | December 2017 (CLOSED, superseded by SP-2017-01) |
| SP-2016-04 | Repair the small tear in the liner of Seepage Pond 2 (Photo II-C-12, Appendix II). | 3 | May 2017 (CLOSED) |

Notes:

- 1. Recommendation priority guidelines, specified by Teck and assigned by KCB:
 - Priority 1: A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant risk of regulatory enforcement.
 - Priority 2: If not corrected could likely result in dam safety issues leading to injury, environmental impact or significant regulatory enforcement; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
 - Priority 3: Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
 - Priority 4: Best Management Practice Further improvements are necessary to meet industry best practices or reduce potential risks.

Table 8.2 2017 Recommendations for Deficiencies and Non-Conformances

| ID No. | Deficiency or Non- Conformance | Applicable Regulation or OMS Reference | Recommended Action | Priority ⁽¹⁾ | Recommended Deadline (Status) |
|------------|--|---|---|-------------------------|-------------------------------------|
| | | | L-L Dam | | |
| | | No n | ew recommendations from 2017 | | |
| | | | H-H Dam | | |
| | | No n | ew recommendations from 2017 | | |
| | | | 24 Mile Lake | | |
| | | No n | ew recommendations from 2017 | | |
| | | L-L Dam S | Seepage Collection and Slimes Ponds | | |
| SP-2017-01 | SP-2017-01 Water Management Water Management Water Sp-2017-01 Water Management Flood Routing Flood Routin | | | | |
| SP-2017-02 | Water Management | Flood Routing | Regrade pipe to a consistent downward grade | 2 | Q4 2018 |

Notes:

- 1. Recommendation priority guidelines, specified by Teck and assigned by KCB:
 - Priority 1: A high probability or actual dam safety issue considered immediately dangerous to life, health or the environment, or a significant risk of regulatory enforcement.
 - Priority 2: If not corrected could likely result in dam safety issues leading to injury, environmental impact or significant regulatory enforcement; or, a repetitive deficiency that demonstrates a systematic breakdown of procedures.
 - Priority 3: Single occurrences of deficiencies or non-conformances that alone would not be expected to result in dam safety issues.
 - Priority 4: Best Management Practice Further improvements are necessary to meet industry best practices or reduce potential risks.

9 CLOSING

This report is an instrument of service of Klohn Crippen Berger Ltd. The report has been prepared for the exclusive use of Teck Highland Valley Copper Partnership (Client) for the specific application to the Highland Valley Copper Project. The report's contents may not be relied upon by any other party without the express written permission of Klohn Crippen Berger. In this report, Klohn Crippen Berger has endeavoured to comply with generally-accepted professional practice common to the local area. Klohn Crippen Berger makes no warranty, express or implied.

KLOHN CRIPPEN BERGER LTD.

Bill Chin, P.Eng.

Engineer of Record

Senior Geotechnical Engineer, Principal

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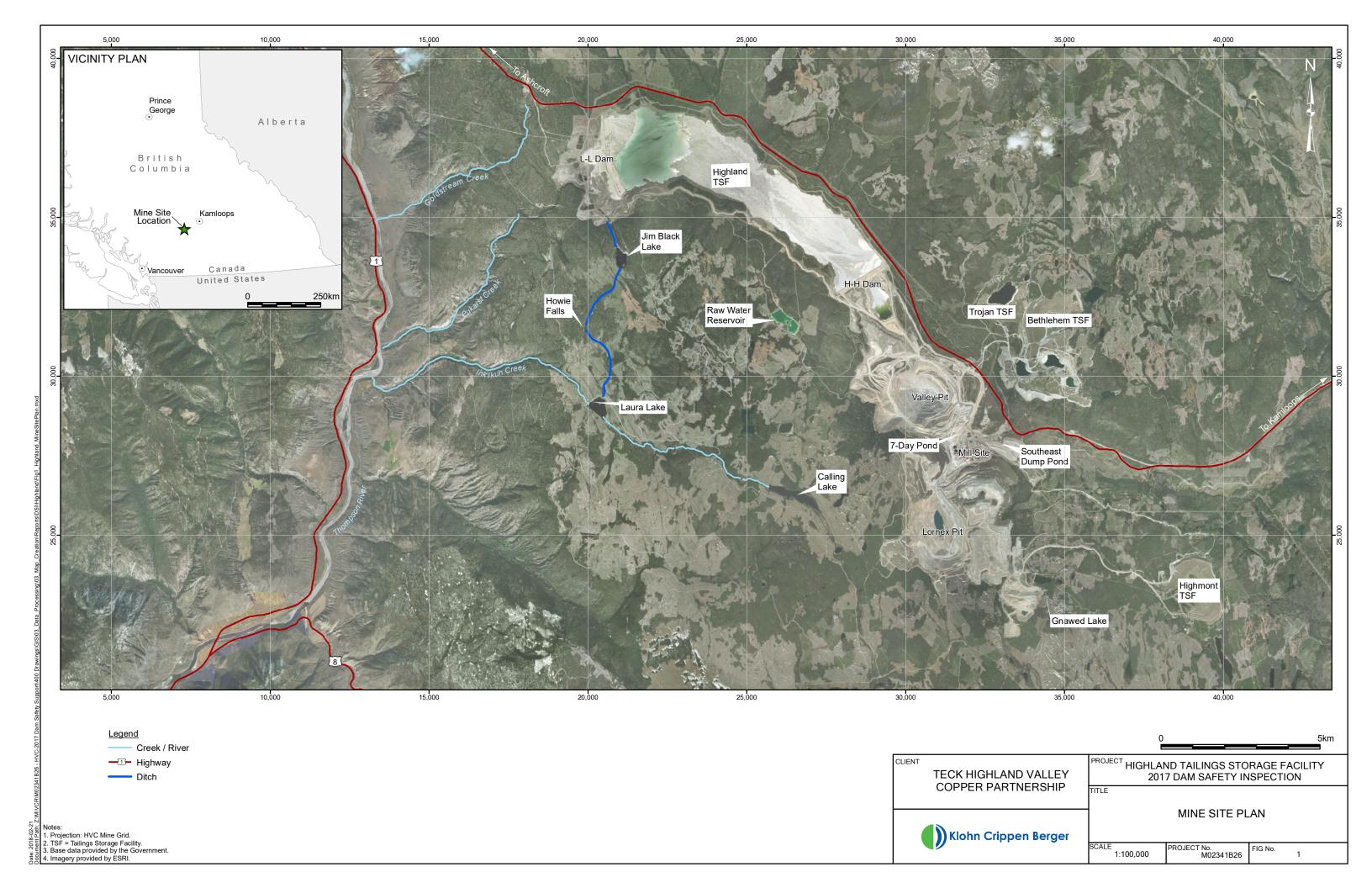
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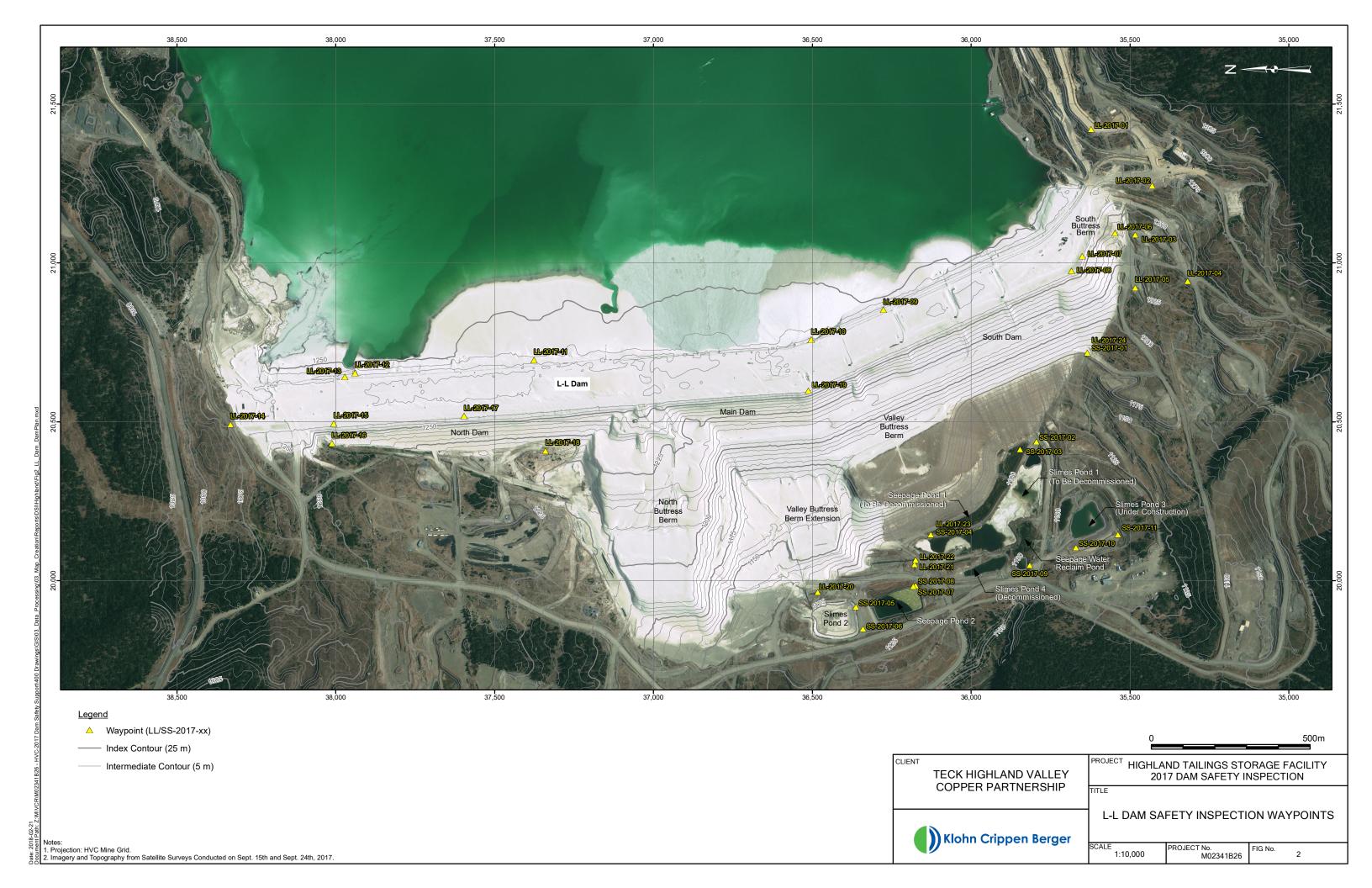


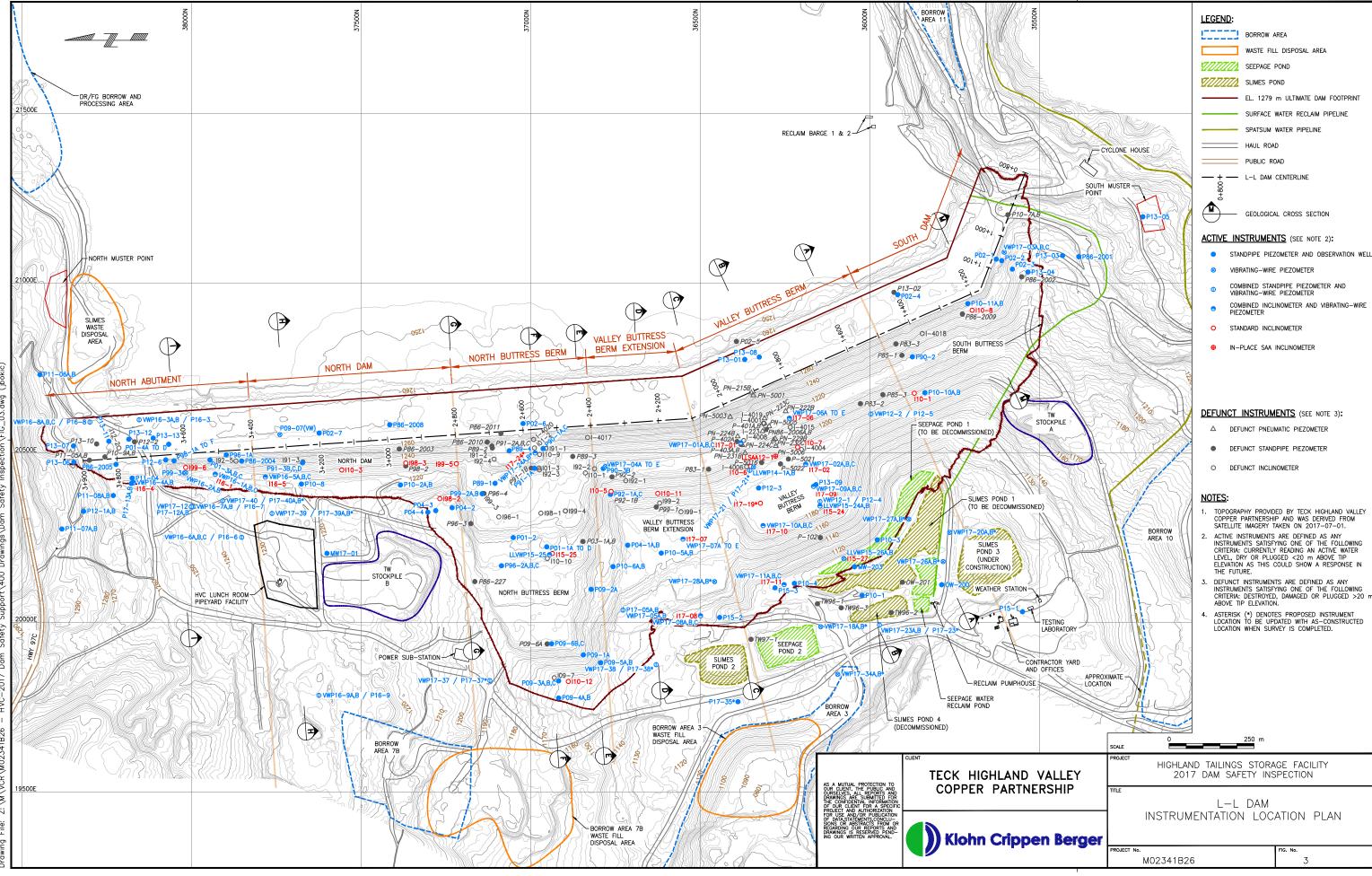
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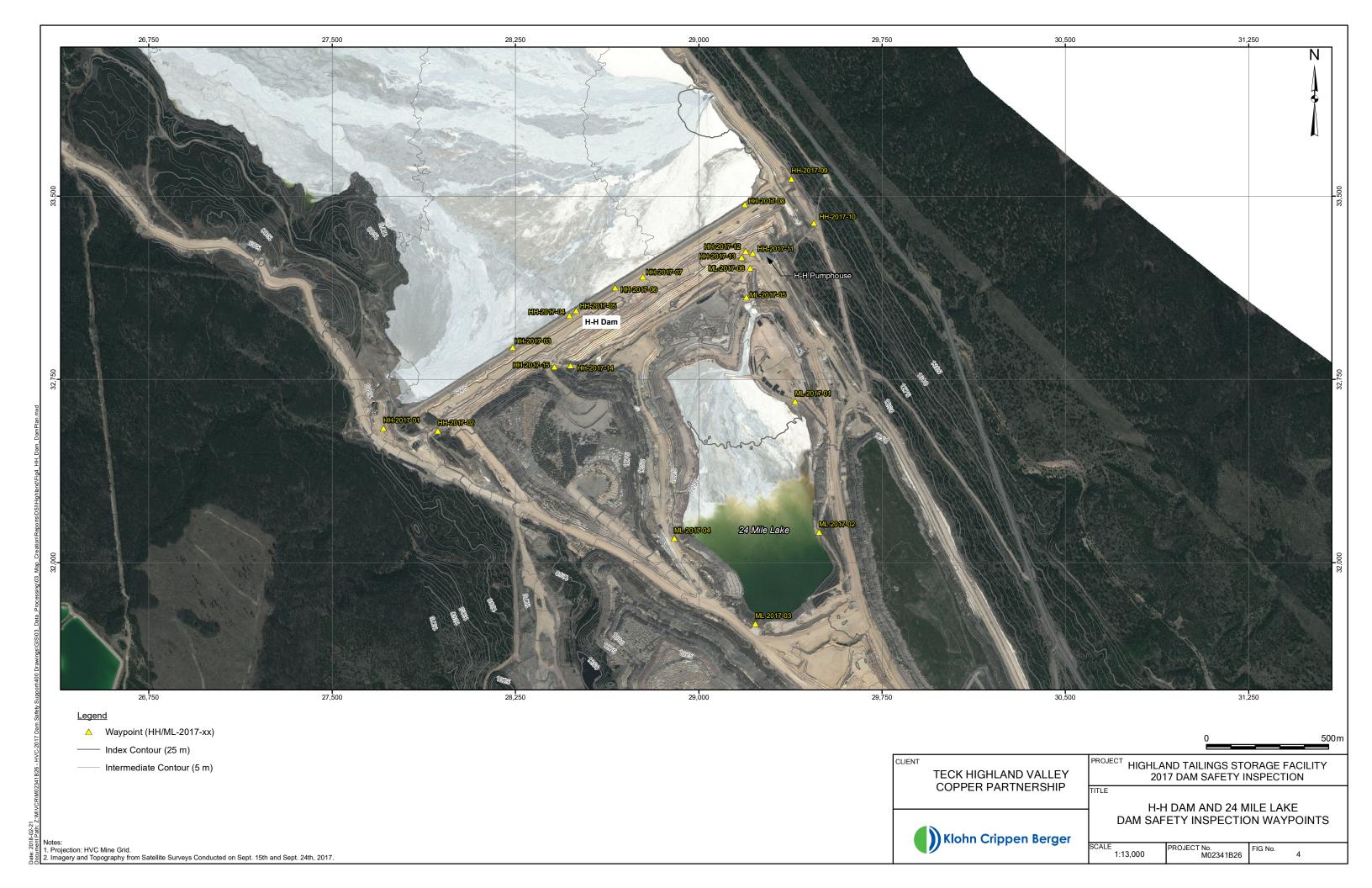
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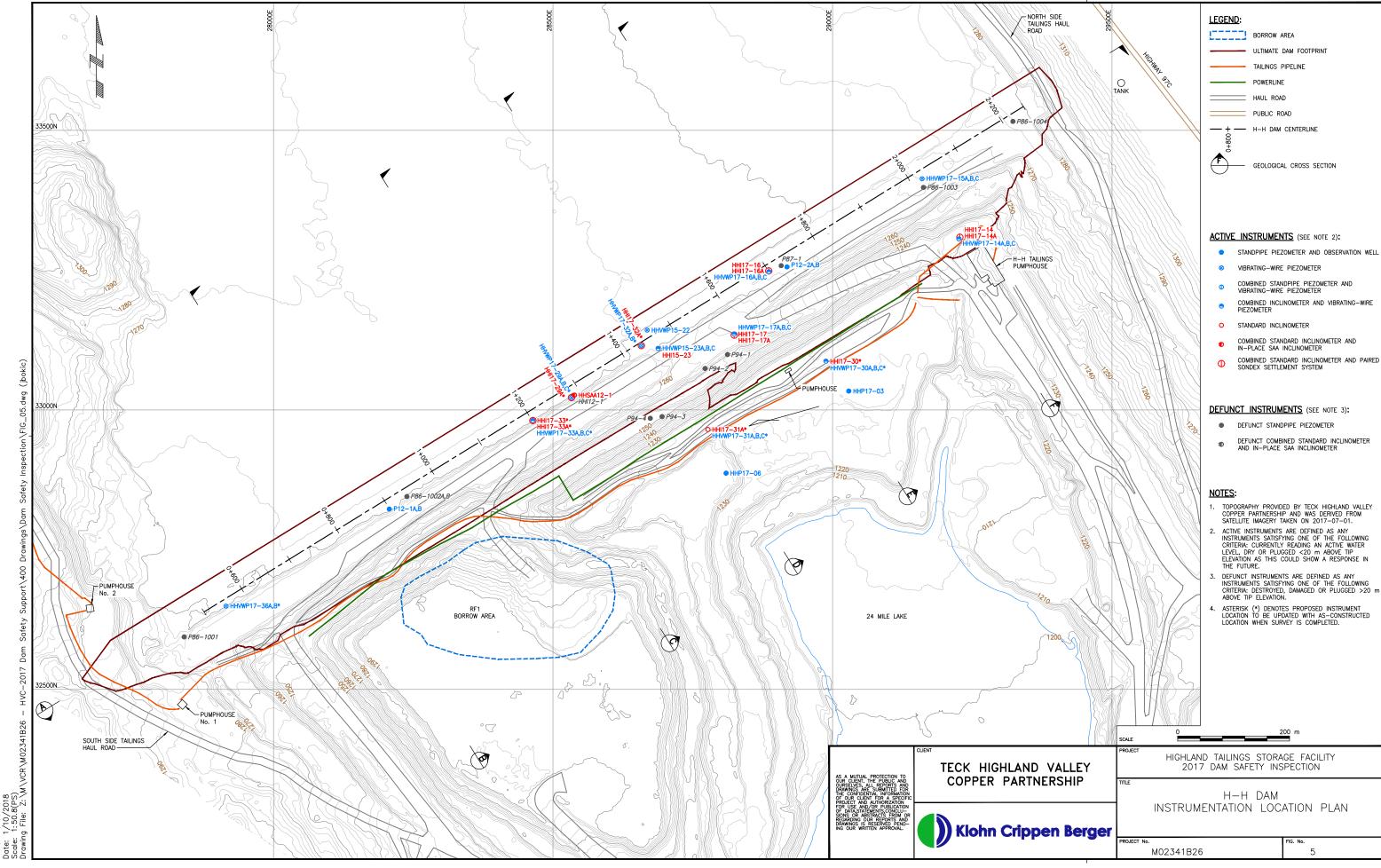






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APPENDIX I

Dam Safety Inspection Checklist

APPENDIX I-A

L-L Dam Safety Inspection Checklist

2017 DAM SAFETY INSPECTION CHECKLIST



| Facility: | L-L Dam | Inspection Date: | September 27, 2017 | |
|-----------------------------------|--------------------|------------------|--------------------|--|
| Consequence Classification | Extreme | | | |
| Weather: | Clear, sunny, 24°C | Inspector(s): | E. Hill, B. Chin | |

| Tailings Pond (m) | | | | |
|--|---|--|--|--|
| Pond El. (Survey data) | 1247.61 (THVCP survey completed on Sept. 28, 2017) | | | |
| Current Min. Dam Crest El. (Survey data) | 1260.2 m | | | |
| Freeboard | 12.89 m (based on HVC pond survey completed on Sept. 28, 2017) | | | |
| Required 2017 Crest El. (PMF + 2 m) 1259.9 m | | | | |

Are the following in **SATISFACTORY CONDITION?**

| North Dam | Yes/No | North Buttress Berm | Yes/No |
|-----------------------|------------|-----------------------|------------|
| U/S (S2 zone) | ⊠ Yes □ No | U/S (S2 zone) | ⊠ Yes □ No |
| Crest (Till zone) | ⊠ Yes □ No | Crest (Till zone) | ⊠ Yes □ No |
| D/S Slope (Sand zone) | ⊠ Yes □ No | D/S Slope (Sand zone) | ⊠ Yes □ No |
| Toe | ⊠ Yes □ No | Toe | ⊠ Yes □ No |
| Drains | ⊠ Yes □ No | Drains | ⊠ Yes □ No |

| Main Dam | Yes/No | Valley Buttress Berm | Yes/No |
|-----------------------|--------|-----------------------|------------|
| U/S (S2 zone) | | U/S (S2 zone) | ⊠ Yes □ No |
| Crest (Till zone) | | Crest (Till zone) | ⊠ Yes □ No |
| D/S Slope (Sand zone) | | D/S Slope (Sand zone) | ⊠ Yes □ No |
| Toe | | Toe | ⊠ Yes □ No |
| Drains | | Drains | ⊠ Yes □ No |

| South Dam | Yes/No |
|-----------------------|------------|
| U/S (S2 zone) | |
| Crest (Till zone) | |
| D/S Slope (Sand zone) | ⊠ Yes □ No |
| Toe | |

Were any POTENTIAL PROBLEM INDICATORS found?

| INDICATOR | North Dam | North Buttress Berm | Main Dam | Valley Buttress Berm | South Dam |
|-------------------------|------------|------------------------|------------|-------------------------|------------|
| Piping | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Sinkholes | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ☒ No |
| Seepage | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Sloughing/Slides | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Animal Activity | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ☒ No |
| Excessive Growth | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Excessive Debris | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Over-steepened Slope | ⊠ Yes □ No | ☐ Yes ⊠ No | ⊠ Yes □ No | ⊠ Yes □ No | ☐ Yes ⊠ No |

Deficiencies:

• The downstream upper 20 m is over-steepened to ~ 1H:1V. These areas are monitored for signs of instability on a regular basis. No immediate dam safety concern (FoS > 1.5).

Comments:

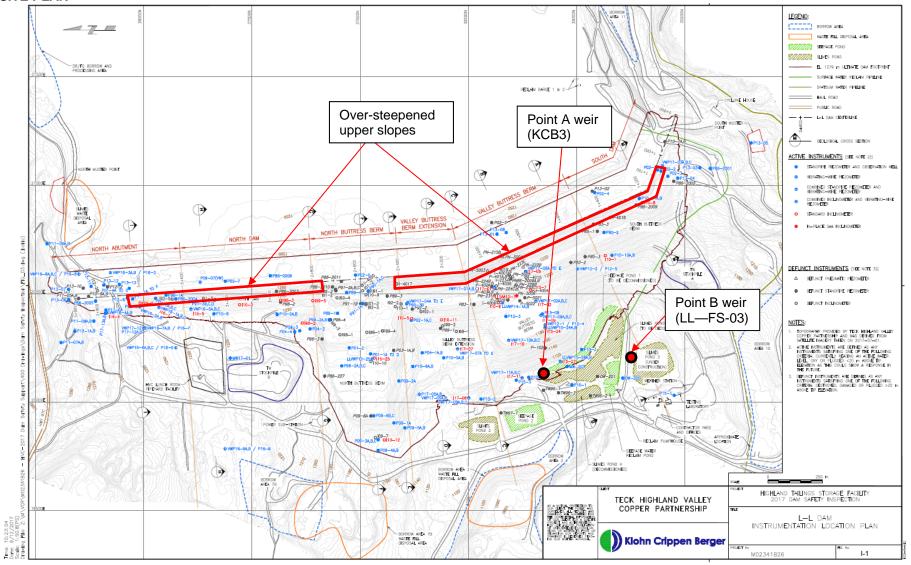
- Point A weir (KCB3) overgrown by vegetation. Plans to decommission as part of 2018 foundation preparation activities.
- Point B weir (LL—FS-03) full of sediment which is affecting flow readings.

Dam Classification:

- Classification: Extreme
- Date of latest dam classification: January 2017
- By: Teck/AMEC/KCB

Does the Dam meet design requirements?

- Stability: Yes
- Flood Storage: Yes



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APPENDIX I-B

H-H Dam Safety Inspection Checklist



| Facility: | H-H Dam | Inspection Date: | September 27, 2017 |
|---|--------------------|------------------|--------------------|
| Weather: | Clear, sunny, 15°C | Inspector(s): | E. Hill, B.Chin |
| Minimum buffer (top of tailings to crest) | 1.2 m @ Sta. 2+100 | | |

Are the following in **SATISFACTORY CONDITION?**

| DAM | Yes/No |
|-----------|--------|
| U/S Slope | |
| Crest | |
| D/S Slope | |
| D/S Toe | |

Were any POTENTIAL PROBLEM INDICATORS found?

| INDICATOR | DAM |
|------------------|------------|
| Piping | ☐ Yes ☒ No |
| Sinkholes | ☐ Yes ⊠ No |
| Seepage | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No |
| Sloughing/Slides | ☐ Yes ☒ No |
| Animal Activity | ☐ Yes ⊠ No |
| Excessive Growth | ☐ Yes ⊠ No |
| Excessive Debris | ☐ Yes ☒ No |

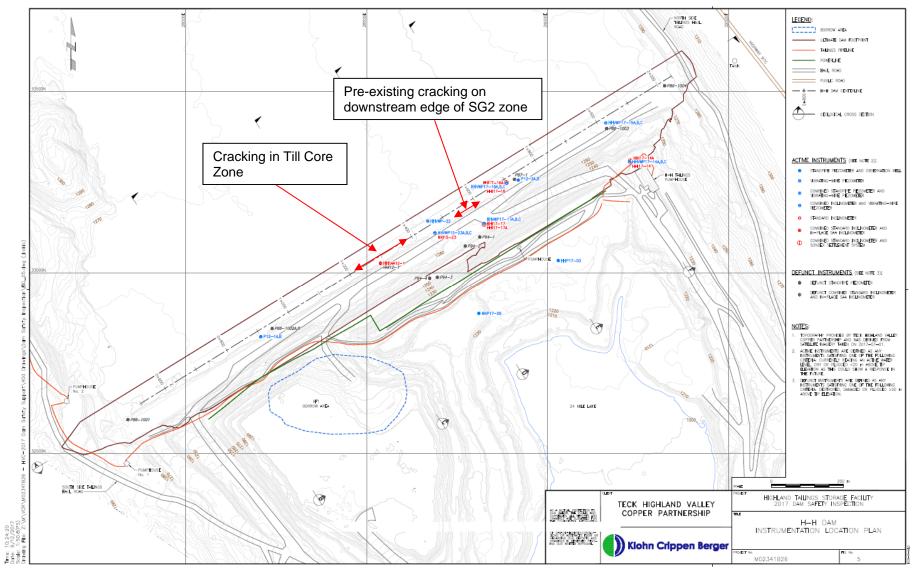
Deficiencies:

None noted.

Comments:

- Vegetation noted on the crest of the dam (Till Core and RF1 Zones) which should be removed as part
 of regular dam maintenance works.
- Longitudinal cracking noted in Till Core Zone from Sta. 1+200 to Sta. 1+400 (non-continuous). Material will be stripped as part of subgrade preparation during 2017 fill placement.
- Pre-existing longitudinal crack noted at the downstream edge of the SG2 zone at WP1608. No signs of
 fresh movement or vertical displacement across crack. Crack is >30 cm deep and up to 20 cm wide
 with another hairline crack approx. 1 m upstream of the main crack. HVC is monitoring the extent
 (depth / width / length) of the cracking during regular site inspections. Continue to monitor. Plan to
 repair with dozer blade, scarify and re-compact.
- Upstream RF2 material placed on Till Core Zone near HHVWP15-22 to be pushed back during 2017 dam raise.
- RF2 material overbuilt onto SG2 zone from Sta. 0+540 to Sta. 1+600 to be scaled back during 2017 dam raise.

Site Plan



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APPENDIX I-C

Highland TSF Safety Inspection Checklist

APPENDIX I-C-1

L-L Seepage Pond 1 Safety Inspection Checklist



| Facility: | L-L Dam Seepage Pond 1 | Inspection Date: | September 27, 2017 |
|-----------|------------------------|------------------|--------------------|
| Weather: | Clear, sunny, 24°C | Inspector(s): | E. Hill, B. Chin |

| Condition | Outlet | | |
|---------------------------------------|---|--|--|
| Was outlet flowing? | ⊠ Yes □ No □ N/A | | |
| Flow rate at Point A Weir (KCB3): | No flow, overgrown by vegetation and no longer functional | | |
| Flow rate at Point B Weir (LL—FS-03): | Not recorded | | |

Are the following in <u>SATISFACTORY CONDITION</u>?

| WEST EMBANKMENT | Yes/No |
|-----------------|------------|
| U/S slope | ⊠ Yes □ No |
| Crest | ⊠ Yes □ No |
| D/S Slope | ⊠ Yes □ No |
| D/S Toe | ⊠ Yes □ No |

Were POTENTIAL PROBLEM INDICATORS found?

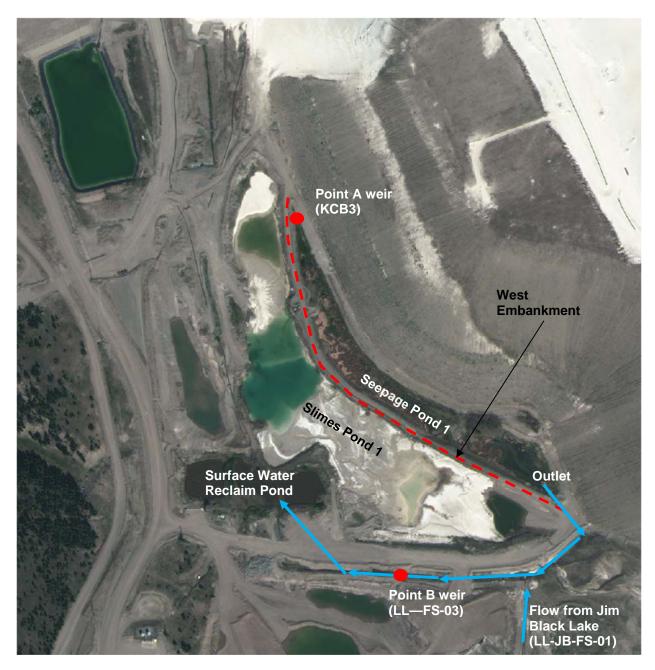
| INDICATOR | WEST EMBANKMENT |
|------------------|-----------------|
| Piping | ☐ Yes ⊠ No |
| Sinkholes | ☐ Yes ⊠ No |
| Seepage | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No |
| Sloughing/Slides | ☐ Yes ⊠ No |
| Animal Activity | ☐ Yes ⊠ No |
| Excessive Growth | ☐ Yes ⊠ No |
| Excessive Debris | ☐ Yes ☒ No |

Deficiencies:

• None noted.

Comments:

- Outlet flow is combined with flows from Jim Black Lake prior to measurement at Point B weir (LL—FS-03)– flow should be measured separately.
- Point B weir (LL—FS-03) is checked with sediment and should be cleaned out to ensure accurate measurements



= measuring weir

APPENDIX I-C-2

L-L Seepage Pond 2 Safety Inspection Checklist



| Facility: L-L Dam Seepage Pond 2 | | Inspection Date: | September 27, 2017 | |
|----------------------------------|--------------------|------------------|--------------------|--|
| Weather: | Clear, sunny, 24°C | Inspector(s): | E. Hill, B.Chin | |
| Freeboard: | ~1.5 m | | | |

| Condition | Outlet | | |
|---|------------------|--|--|
| Was outlet flowing? | ⊠ Yes □ No □ N/A | | |
| Inflow rate at Finger Drain 1 weir (LL—FS-01): | 475 L/min | | |
| Outflow rate at Seepage Pond 2 Weir (LL—FS-04): | 490 L/min | | |

Are the following in <u>SATISFACTORY CONDITION</u>?

| EMBANKMENT | Yes/No | OUTLET | Yes/No |
|------------|------------|-----------------|------------|
| U/S slope | ⊠ Yes □ No | Outlet Pipe | ⊠ Yes □ No |
| Crest | ⊠ Yes □ No | Outlet Channel | ⊠ Yes □ No |
| D/S Slope | | Outlet Controls | |
| D/S Toe | ⊠ Yes □ No | | |

Were POTENTIAL PROBLEM INDICATORS found?

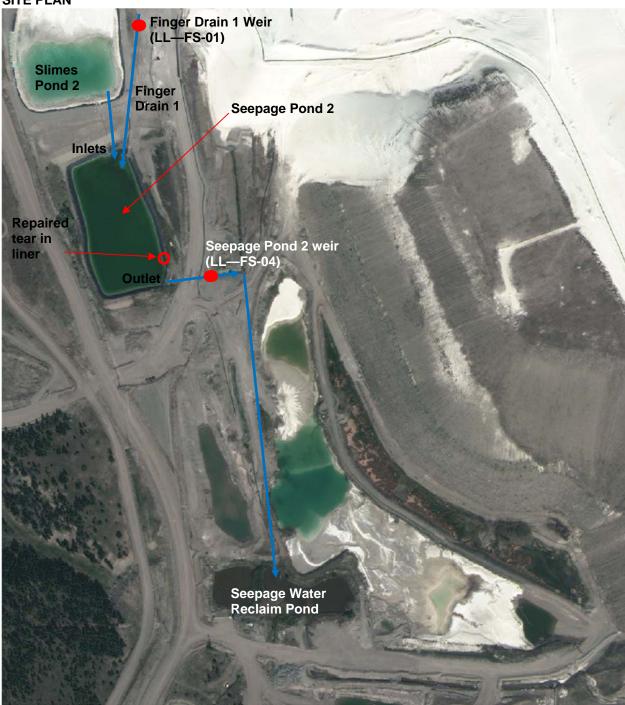
| INDICATOR | EMBANKMENT | OUTLET CHANNEL |
|------------------|------------|----------------|
| Piping | ☐ Yes ⊠ No | |
| Sinkholes | ☐ Yes ⊠ No | |
| Seepage | ☐ Yes ⊠ No | |
| External Erosion | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No | |
| Sloughing/Slides | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Animal Activity | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Excessive Growth | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Excessive Debris | ☐ Yes ☒ No | ☐ Yes ☒ No |

Deficiencies:

None.

Comments:

- 1. Capacity of outlet pipe under review to determine available freeboard during the IDF.
- 2. No inflow from Slimes Pond 2 at time of inspection.
- 3. Small tear in pond liner (~30 cm wide) close to outlet pipe (approx. 1m above water level) has been repaired.



= measuring weir

APPENDIX I-C-3

L-L Slimes Pond 1 Safety Inspection Checklist

Deficiencies:

Comments:

None noted.

None noted.



| Facility: | L-L Dam Slimes P | ond 1 | Inspection [| Date: | September 27, 2017 | |
|--|-------------------------|--------------|---------------|-------|--------------------|--|
| Weather: | Clear, sunny, 24°C | | Inspector(s): | | E. Hill, B.Chin | |
| Are the followin | g in <u>SATISFACTOR</u> | Y CONDITION? | | | | |
| | AREA | EAST EMBANI | KMENT | W | EST EMBANKMENT | |
| U/S slope | | ⊠ Yes □ No | | | □ No | |
| Crest | | | | | □ No | |
| D/S Slope | | | | | | |
| Were POTENTIAL PROBLEM INDICATORS found? | | | | | | |
| IND | ICATOR | EAST EMBANI | KMENT | W | EST EMBANKMENT | |
| Piping | | ☐ Yes ⊠ | No | | ☐ Yes ⊠ No | |
| Sinkholes | | ☐ Yes ⊠ | No | | ☐ Yes ⊠ No | |
| Seepage | | ☐ Yes 🏻 | No | | ☐ Yes ⊠ No | |
| External Erosio | n | ☐ Yes 🏻 | No | | ☐ Yes ⊠ No | |
| Cracks | | ☐ Yes 🏻 | No | | ☐ Yes ⊠ No | |
| Settlement | | ☐ Yes 🛚 | No | | ☐ Yes ☐ No | |
| Sloughing/Slide | es | ☐ Yes ⊠ | No | | ☐ Yes ☐ No | |
| Animal Activity | | ☐ Yes 🏻 | No | | ☐ Yes ⊠ No | |
| Excessive Grov | wth | ☐ Yes 🏻 | No | | ☐ Yes ⊠ No | |
| Excessive Deb | ris | ☐ Yes ⊠ | No | | ☐ Yes ⊠ No | |
| * the dam is breached | <u> </u> | | | | | |



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APPENDIX I-C-4

L-L Slimes Pond 2 Safety Inspection Checklist



| Facility: | L-L Dam Slimes Pond 2 | Inspection Date: | September 27, 2017 |
|------------|------------------------------|------------------|--------------------|
| Weather: | Clear, sunny, 24°C | Inspector(s): | E. Hill, B.Chin |
| Freeboard: | n/a – pond was dry and empty | | |

| Condition | Outlet |
|---------------------|------------------|
| Was outlet flowing? | ☐ Yes ☑ No ☐ N/A |
| Inflow rate: | 0 L/min |
| Outflow rate: | 0 L/min |

Are the following in **SATISFACTORY CONDITION**?

| AREA | EMBANKMENT |
|-----------|------------|
| U/S slope | ⊠ Yes □ No |
| Crest | ⊠ Yes □ No |
| D/S Slope | ⊠ Yes □ No |

Were POTENTIAL PROBLEM INDICATORS found?

| INDICATOR | EMBANKMENT |
|------------------|------------|
| Piping | ☐ Yes ⊠ No |
| Sinkholes | ☐ Yes ⊠ No |
| Seepage | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No |
| Sloughing/Slides | ☐ Yes ⊠ No |
| Animal Activity | ☐ Yes ⊠ No |
| Excessive Growth | ☐ Yes ⊠ No |
| Excessive Debris | ☐ Yes ⊠ No |

Deficiencies:

None.

Comments:

• Slimes were removed/excavated to pond bottom in 2017. The protective gravel layer above the liner was not compromised.



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APPENDIX I-C-5

Highland Seepage Water Pond Safety Inspection Checklist



| Facility: | L-L Dam Seepage Water Reclaim Pond | Inspection Date: | September 27, 2017 |
|------------|---|------------------|--------------------|
| Weather: | Sunny, clear, 24°C | Inspector(s): | E. Hill, B.Chin |
| Freeboard: | 3.25 m (based on HVC pond survey 28-Sep-17) | | |

| Condition | Outlet pumps | Inlet #1 | Inlet #2 |
|-----------------|--------------|--------------|--------------|
| Was it flowing? | ⊠ Yes □ No | ⊠ Yes □ No | ⊠ Yes □ No |
| Flow rate: | Not recorded | Not recorded | Not recorded |

Inlet#1=from Jim Black Lake / Seepage Pond 1, Inlet #2=from Seepage Pond II / UVD/LVD weir

Are the following in <u>SATISFACTORY CONDITION</u>?

| EAST EMBANKMENT* | Yes/No |
|------------------|------------|
| U/S slope | ⊠ Yes □ No |
| Crest | ⊠ Yes □ No |
| D/S Slope | ⊠ Yes □ No |
| D/S Toe | ⊠ Yes □ No |

^{*} This refers to the embankment separating the Surface Water Reclaim Pond and Slimes Pond 1

| WEST DAM | Yes/No | OUTLET | Yes/No |
|-----------|------------|-----------------|------------|
| U/S slope | ⊠ Yes □ No | Outlet Pump | ⊠ Yes □ No |
| Crest | | Outlet Controls | ⊠ Yes □ No |

| INLETS | Inlet #1 | Inlet #2 |
|--------------------|------------|------------|
| Inlet pipe / ditch | ⊠ Yes □ No | ⊠ Yes □ No |

Inlet#1=from Jim Black Lake / Seepage Pond 1, Inlet #2=from Seepage Pond II / UVD/LVD weir,

Were POTENTIAL PROBLEM INDICATORS found?

| INDICATOR | EAST EMBANKMENT | WEST DAM |
|------------------|-----------------|------------|
| Piping | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Sinkholes | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Seepage | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Sloughing/Slides | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Animal Activity | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Excessive Growth | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Excessive Debris | ☐ Yes ⊠ No | ☐ Yes ⊠ No |

| Deficiencies: None | |
|-----------------------|--|
| Comments: | |

Highland Valley Copper Dam Inspection Checklist – L-L Dam Surface Water Reclaim Pond



APPENDIX I-D

24 Mile Lake Dam Safety Inspection Checklist



| Facility: | 24 Mile Lake | Inspection Date: | September 27, 2017 |
|----------------------------|---|------------------|---|
| Weather: | Clear, sunny, 14°C | Inspector(s): | E. Hill, B.Chin |
| Crest Elevation: | 1220 m | | |
| Current Pond Elevation: | 1196.8 m (THVCP survey completed on Sept. 28, 2017) | Freeboard: | > 10 m (field measurement on 27-Sep-17) 14.2 m (based on THVCP pond survey completed on Sept. 28, 2017) |

Are the following in <u>SATISFACTORY CONDITION</u>?

| WASTE DUMP WALLS | Yes/No |
|------------------|--------|
| U/S Slope | |
| Crest | |

Were POTENTIAL PROBLEM INDICATORS found?

| INDICATOR | EMBANKMENT |
|------------------|------------|
| Piping | ☐ Yes ⊠ No |
| Sinkholes | ☐ Yes ⊠ No |
| Seepage | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ☒ No |
| Cracks | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No |
| Sloughing/Slides | ⊠ Yes □ No |
| Animal Activity | ☐ Yes ⊠ No |
| Excessive Growth | ☐ Yes ⊠ No |
| Excessive Debris | ☐ Yes ⊠ No |

Deficiencies:

None noted.

Comments:

- Some erosion gullies evident on west side of pond in waste dump walls. No dam safety concern at this time.
- Evidence of recent erosion at southeast corner of pond on reclaimed slope. Haul road could be impacted if erosion progresses.

APPENDIX I-E

Quarterly Dam Safety Inspection Checklist

APPENDIX I-E-1

Quarter 2 H-H Dam Safety Inspection Checklist

Q2 QUARTERLY DAM INSPECTION CHECKLIST



| Facility: | H-H Dam | | Inspection Date: | 2017-05-28 |
|--|--------------------|--------------|------------------|------------|
| Weather: | Clear / sunny | | Inspector(s): | E.Hill |
| Minimum observed bu level to dam crest): | ffer height (delta | 1.5 m at Sta | a. 0+490 | |

Are the following components of your dam in <u>SATISFACTORY CONDITION</u>? (check one if applicable)

| EMBANKMENT | Yes/No |
|------------|--------|
| U/S slope | |
| Crest | |
| D/S Slope | |
| D/S Toe | |

Were any of the following <u>POTENTIAL PROBLEM INDICATORS</u> found?

| INDICATOR | EMBANKMENT INDICATOR | | EMBANKMENT |
|------------------|----------------------|------------------|------------|
| Piping | ☐ Yes 🖾 No | Settlement | ☐ Yes ⊠ No |
| Sinkholes | ☐ Yes 🖾 No | Sloughing/Slides | ☐ Yes ⊠ No |
| Seepage | ☐ Yes ☒ No | Animal Activity | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ☒ No | Excessive Growth | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ☒ No | Excessive Debris | ☐ Yes ⊠ No |

List and describe any deficiencies (all deficiencies require assessment and/or repair):

New and Outstanding deficiencies:

None

Comments / Notes:

- 1. Current discharge at west abutment spigot point.
- 2. Some cracking at the seam between RF1 zone and the upstream (u/s) RF2 zone along the length of the dam due to settlement of the u/s RF2, most prevalent west of Sta.0+800. The cracking was not characterized as a significant dam safety concern as similar cracks have been recorded historically during fill placement, and have been attributed to settlement of the RF1 fill which is placed directly on the tailings. (Photo 1 and 2)
- 3. Observed cracking on the downstream (d/s) edge of SG2 zone between Sta. 1+497 and Sta. 1+562 (based on HVC survey April 5, 2017), as shown on attached Figure 1. Crack dimensions are being monitored by THVCP on a weekly basis. Cracking is located in an area where the d/s RF2 zone is approximately 4m below the elevation of the SG2 zone. Therefore the source of cracking is likely settlement of the loose safety berm material. Refer to KCB MRM-2017-004 for further details.

Figure 1 - SITE PLAN

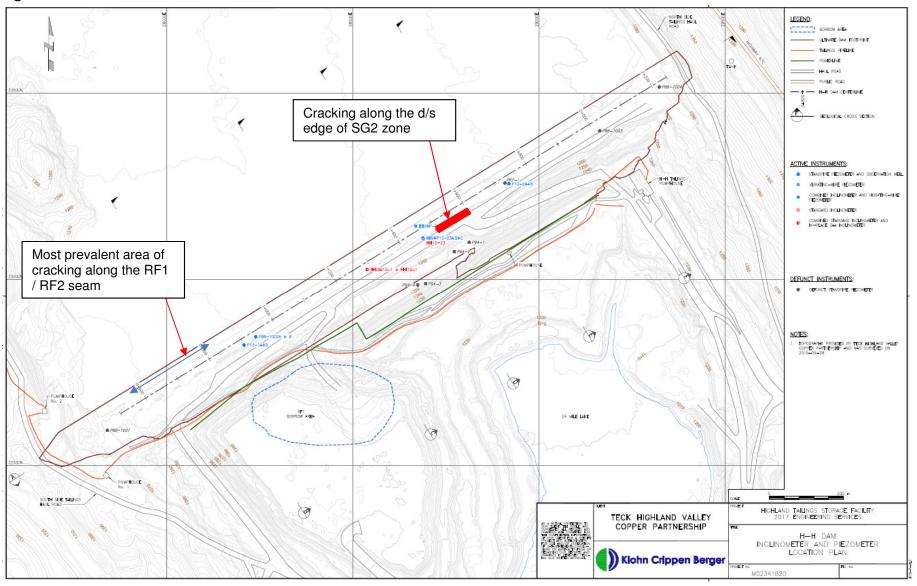




Photo 1 – Cracking at the seam between RF1 and U/S RF2 at approx. Sta. 0+800



Photo 2 - Cracking at the seam between RF1 and U/S RF2 at approx. Sta. 0+800



Photo 3 – Cracking on the u/s edge of SG2 zone at approx. Sta. 1+550 (facing west)



Photo 4 - Cracking on the u/s edge of SG2 zone at approx. Sta. 1+550



Photo 5 – Overview of cracking area on the u/s edge of SG2 zone showing height of loose safety berm (~ 4m)

APPENDIX I-E-2

Quarter 2 L-L Dam Safety Inspection Checklist

Q2 QUARTERLY DAM INSPECTION CHECKLIST



| Facility: | L-L Dam | Inspection Date: | 2017-05-28 |
|-----------|---------------|------------------|------------|
| Weather: | Clear / sunny | Inspector(s): | E.Hill |

| Tailings Pond | | Seepage Flows | L / min | Comments |
|---------------------------------------|---------|------------------|---------|---|
| Pond El. (m) | 1247.51 | Finger Drain 1 | 372 | Clear |
| Min. Dam Crest El. (m) | 1260.5 | UVD/LVD | 830 | Clear |
| Freeboard (m) | 13 | Point B weir | 6,662 | Weir and channel are silted-up. Readings are not accurate. |
| Required 2017 Crest El (PMF + 2m) (m) | 1260.5 | | | |

Are the following components in <u>SATISFACTORY CONDITION?</u>

| South Dam | Valley Buttress Berm | |
|---------------------|----------------------|------------|
| U/S slope of Zone D | U/S slope of Zone D | |
| Crest (Zone B) | Crest (Zone B) | |
| D/S Slope | D/S Slope | |
| Toe | Toe | |
| | Drains | ⊠ Yes □ No |

| Main Dam | North Buttress Berm | |
|---------------------|---------------------|--|
| U/S slope of Zone D | U/S slope of Zone D | |
| Crest (Zone B) | Crest (Zone B) | |
| D/S Slope | D/S Slope | |
| Toe | Toe | |
| Drains | Drains | |

| North Dam | | |
|---------------------|------------|--|
| U/S slope of Zone D | | |
| Crest (Zone B) | | |
| D/S Slope | ⊠ Yes □ No | |
| Toe | | |
| Drains | | |

Were any of the following POTENTIAL PROBLEM INDICATORS found?

| INDICATOR | South Dam | Valley Buttress Berm | Main Dam | North Buttress Berm | North Dam |
|-------------------------|------------|-------------------------|------------|------------------------|------------|
| Seepage | ☐ Yes ☒ No | ☐ Yes ⊠ No | ☐ Yes ☒ No | ☐ Yes ☒ No | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ☒ No | ☐ Yes ☒ No | ☐ Yes ⊠ No | ☐ Yes ☒ No | ☐ Yes ☒ No |
| Cracks | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ☒ No |
| Sloughing/Slides | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Over-steepened Slope | ⊠ Yes □ No | ⊠ Yes □ No | ⊠ Yes □ No | ☐ Yes ☒ No | ⊠ Yes □ No |

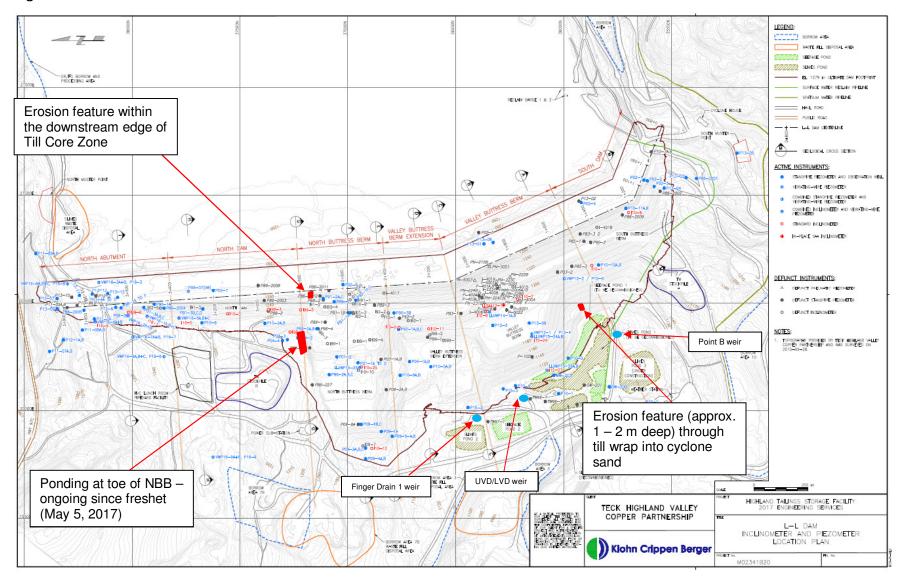
List and describe any deficiencies:

- Over-steepened upper sections of the downstream dam slopes still evident. These locally oversteepened slopes were due to raising of the glacial till core in 2013 and 2015 without concurrently constructing the downstream sand support, and have been noted in 2016 DSI report. EoR has reviewed stability and found acceptable factor of safety and no interim dam safety concern. No signs of instability during inspection.
- 2. These locally over-steepened slopes will be flattened during the upcoming 2017/2018 construction

Notes and Comments:

- 1. Water associated with snowmelt freshet continues to flow from the North Dam and accumulate at the northern toe of the North Buttress Berm (location shown in Figure 1). It appears that the upstream end of an existing culvert which used to feed water from the North Dam drain into the major finger drain beneath the NBB (Finger Drain 1) is buried/blocked. This is likely contributing to the ongoing ponding. This is not considered a dam safety issue, at this time. (Photo 1)
- 2. An erosion feature was noted below the VBB at Sta. 1+600 (approx. El. 1125), (location shown in Figure 1). Runoff has eroded through the till wrap into the cyclone sand and is approximately 1 2 m deep. Erosion feature should be backfilled to prevent further erosion of cyclone sand. (Photo 2)
- 3. Erosion feature (approx. 0.5 m deep) at ~ Sta. 2+750 has eroded the downstream edge of the Till Core Zone (location shown in Figure 1). Will require repair during the next construction raise. Erosion feature should be surveyed for record purposes. (Photo 3)
- 4. Higher than normal water level in Slimes Pond 1 due to recent freshet. Freeboard > 5m. (Photo 4)
- 5. Till Core Zone is covered by wind-blown sand making inspection of its condition impossible. (Photo 5)

Figure 1 - Site Plan



Relevant Photos:



Photo 1- Ponded Water At North Toe of North Buttress Berm (facing west)

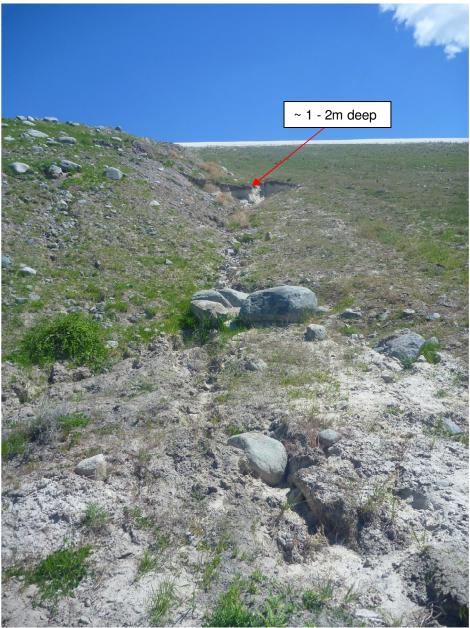


Photo 2 - Erosion Feature at Sta. 1+600



Photo 3 – Erosion feature in downstream edge of Till Core Zone at ~ Sta. 2+750



Photo 4 – Slimes Pond 1



Photo 5 – Windblown sand cover on Till Core Zone

APPENDIX I-E-3

Quarter 4 H-H Dam Safety Inspection Checklist

Q4 QUARTERLY DAM INSPECTION CHECKLIST



| Facility: | H-H Dam | | Inspection Date: | 2017-12-21 |
|---|----------------------|---|------------------|------------|
| Weather: | Cloudy to snow, -7°C | | Inspector(s): | D.Pongracz |
| Minimum observed but top of buffer zone): | ffer (delta level to | 1.8 m at Sta 3.2 m at Sta 2.3 m at Sta (THVCP de | a. 1+400 | ·12-19) |

Are the following components of your dam in <u>SATISFACTORY CONDITION</u>? (check one if applicable)

| EMBANKMENT | Yes/No |
|------------|--------|
| U/S slope | |
| Crest | |
| D/S Slope | |
| D/S Toe | |

Were any of the following <u>POTENTIAL PROBLEM INDICATORS</u> found?

| INDICATOR | EMBANKMENT | INDICATOR | EMBANKMENT |
|------------------|------------|------------------|------------|
| Piping | ☐ Yes ⊠ No | Settlement | ☐ Yes ⊠ No |
| Sinkholes | ☐ Yes ⊠ No | Sloughing/Slides | ☐ Yes ⊠ No |
| Seepage | ☐ Yes ⊠ No | Animal Activity | ☐ Yes ⊠ No |
| External Erosion | ☐ Yes ⊠ No | Excessive Growth | ☐ Yes ⊠ No |
| Cracks | ☐ Yes ⊠ No | Excessive Debris | ☐ Yes ⊠ No |

List and describe any deficiencies (all deficiencies require assessment and/or repair):

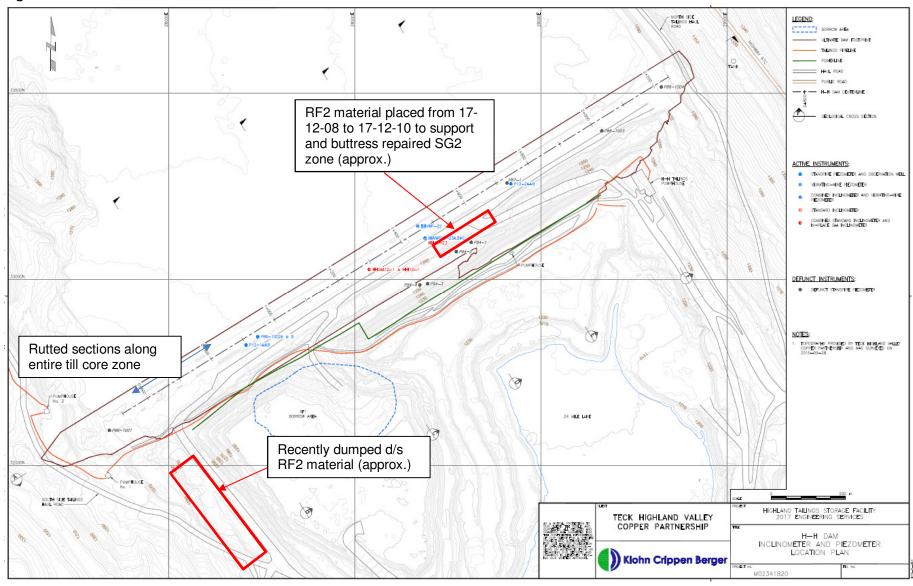
New and Outstanding deficiencies:

None

Comments / Notes:

- 1. Most of the H-H Dam was covered in snow and observations were made to the best of their ability during the current conditions (Photo 1). Evidence of previous discharge from SS1/SS2 along/near the u/s RF2 face approximately between Sta. 0+600 to 0+750 (Photo 2 and 3).
- 2. Recently dumped d/s RF2 material observed between west side of I-9 waste dump and the west access road onto the dam (Photo 4).
- 3. Sections of the till core zone were heavily rutted across the dam (Photo 5).
- 4. Observed area where HVC Mine Ops hauled, dumped and wheel packed d/s RF2 material from December 8 to 10, 2017. The material was placed between ~Sta. 1+475 to ~Sta. 1+625 to ~El. 1268 m to support and buttress the SG2 where cracking had previously occurred (refer to MRM-2017-004) (Photo 6).
- 5. Current tailings discharge northwest of the east abutment (Photo 7).

Figure 1 - SITE PLAN



Relevant Photos:



Photo 1 – Most of the H-H Dam was snow covered at the time of the inspection (looking northeast).



Photo 2 - Evidence of recent discharge from SS1/SS2 along/near the u/s RF2 face approximately between Sta. 0+600 to 0+750 (looking northeast).



Photo 3 - Evidence of recent discharge from SS1/SS2 along/near the u/s RF2 face approximately between Sta. 0+600 to 0+750 (looking east).



Photo 4 – Recently dumped d/s RF2 material observed between west side of I-9 waste dump and the west access road on the dam (looking southeast).



Photo 5 – Sections of the till core zone were heavily rutted (looking northeast).



Photo 6 – Area where THVCP Mine Ops hauled, dumped and wheel packed d/s RF2 material from December 8 to 10, 2017. The material was placed between ~Sta. 1+475 to ~Sta. 1+625 to ~El. 1268 m to support and buttress the SG2 where cracking had previously occurred (refer to MRM-2017-004) (looking north).



Photo 7 – Tailings discharging northwest of the east abutment (looking northeast).

APPENDIX I-E-4

Quarter 4 L-L Dam Safety Inspection Checklist

Q4 QUARTERLY DAM INSPECTION CHECKLIST



| Facility: | L-L Dam | Inspection Date: | 2017-12-20 / 2017-12-21 |
|-----------|---|------------------|-------------------------|
| Weather: | 17-12-20: Clear, -10°C 17-12-21: Snow, -11°C | Inspector(s): | E.Hill / D. Pongracz |

| Tailings Pond | | Seepage Flows | L / min | Comments |
|---------------------------------------|---------|------------------------------|----------------|--|
| Pond El. (m) | 1247.19 | Finger Drain 1 (LL—FS-01) | 491 (16 cm) | Clear (17-12-20) |
| Min. Dam Crest El. (m) | 1260.5 | UVD/LVD (LL—FS-02) | 966 (21 cm) | Mainly clear |
| Freeboard (m) | 13.3 | Point B weir (LL—FS-03) | 965 (21 cm) | Accumulated sediment in weir box and outfall – measurement likely not accurate |
| Required 2017 Crest El (PMF + 2m) (m) | 1260.5 | | | |

Are the following components in <u>SATISFACTORY CONDITION?</u>

| South Dam | Valley Buttress Berm | |
|--------------------------|--------------------------|--|
| U/S slope of U/S S2 zone | U/S slope of U/S S2 zone | |
| Crest (Till Core zone) | Crest (Till Core zone) | |
| D/S Slope | D/S Slope | |
| Toe | Toe | |
| | Drains | |
| | | |

| Main Dam | | North Buttress Berm | |
|--------------------------|------------|--------------------------|------------|
| U/S slope of U/S S2 zone | | U/S slope of U/S S2 zone | |
| Crest (Till Core zone) | ⊠ Yes □ No | Crest (Till Core zone) | |
| D/S Slope | | D/S Slope | |
| Toe | ⊠ Yes □ No | Toe | ⊠ Yes □ No |
| Drains | | Drains | |

| North Dam | | |
|--------------------------|------------|--|
| U/S slope of U/S S2 zone | | |
| Crest (Till Core zone) | | |
| D/S Slope | ⊠ Yes □ No | |
| Toe | | |
| Drains | ⊠ Yes □ No | |

Were any of the following POTENTIAL PROBLEM INDICATORS found?

| INDICATOR | South Dam | South Dam Valley Buttress Berm | | North Buttress Berm | North Dam |
|-------------------------|------------|--------------------------------|------------|------------------------|------------|
| Seepage | ☐ Yes ☒ No | ☐ Yes ⊠ No | ☐ Yes ☒ No | ☐ Yes ☒ No | ☐ Yes ☐ No |
| External Erosion | ☐ Yes ☒ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ☒ No | ☐ Yes ☒ No |
| Cracks | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Settlement | ☐ Yes ☒ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ☒ No | ☐ Yes ☒ No |
| Sloughing/Slides | ☐ Yes ☒ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No | ☐ Yes ⊠ No |
| Over-steepened Slope | ⊠ Yes □ No | ⊠ Yes □ No | ⊠ Yes □ No | ☐ Yes ☒ No | ⊠ Yes □ No |

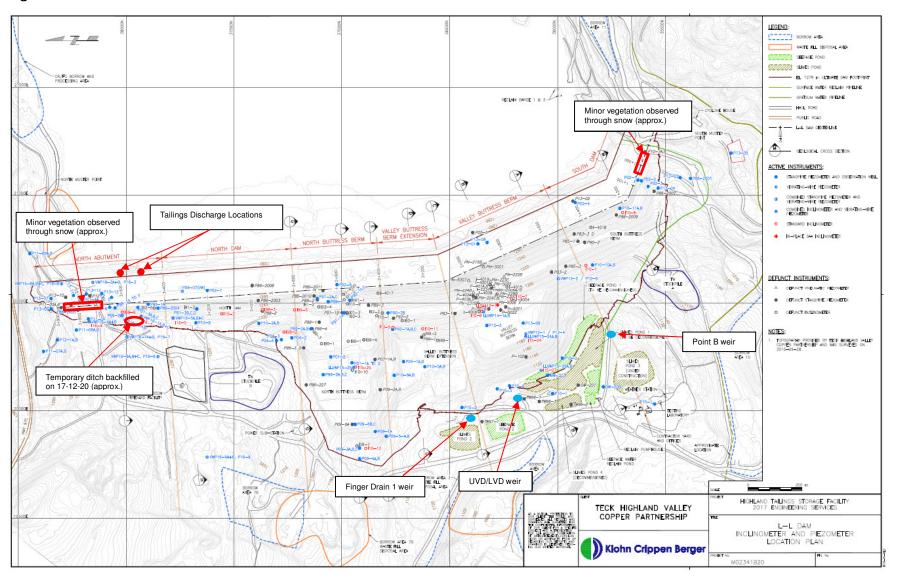
List and describe any deficiencies:

- Over-steepened upper sections of the downstream dam slopes still evident. These locally oversteepened slopes were due to raising of the glacial till core zone in 2013 and 2015 without concurrently constructing the downstream sand support, and have been noted in previous DSI reports and quarterly dam inspections. EoR has reviewed stability and found acceptable factor of safety and no interim dam safety concern. No signs of instability during inspection.
- 2. These locally over-steepened slopes will be addressed during the upcoming 2018/2019 construction.

Notes and Comments:

- 1. The entire L-L Dam was covered in snow and observations were made to the best of their ability during the current conditions (Photo 1).
- 2. Minor vegetation observed through the snow on the till core zone at the North Abutment and South Dam (Photo 2).
- 3. A temporary ditch dug during foundation preparation works at the North Abutment (from Sta. 3+500 to ~3+570) was backfilled on 17-12-20 (Photo 3).
 - o d/s flow was clear.
 - o u/s flow was partially backed up but approximately similar to the 17-12-20 observations.
- 4. No flow from Jim Black Lake reporting to Point B weir (Photo 4).
- 5. No flow from Woods Creek Diversion into Reclaim Pond (Photo 5).
- 6. Tailings discharge locations at Sta. 3+500 and Sta. 3+600 (Photo 6).

Figure 1 - Site Plan



Relevant Photos:



Photo 1- Entire L-L Dam was snow covered at the time of the inspection; two drill rigs seen on the North Buttress Berm (looking North).



Photo 2 – Minor vegetation observed through the snow on the till core zone at the South Dam (looking northwest).



Photo 3 – Temporary ditch dug during foundation preparation works at the North Abutment (from Sta. 3+500 to ~3+570) backfilled on 17-12-20 (looking south).



Photo 4 – No flow from Jim Black Lake reporting to Point B weir (Photo 4).



Photo 5 – No flow from Woods Creek Diversion into the Reclaim Pond (looking north).



Photo 6 – Tailings discharge locations at Sta. 3+500 and 3+600 (photo taken on 17-12-20).



Photo 7 – Point B weir with accumulated sediment in weir box and outfall.

APPENDIX II

Inspection Photographs

APPENDIX II-A

L-L Dam Inspection Photographs

Appendix II-A Inspection Photographs - L-L Dam

LEGEND:

- LL = L-L Dam.
- LL-2017-## refers to 2017 DSI waypoint shown on Figure 2.
- All photographs taken during inspection on September 26 and 27, 2017.

Photo II-A-1 North Abutment Overview (LL-2017-14)

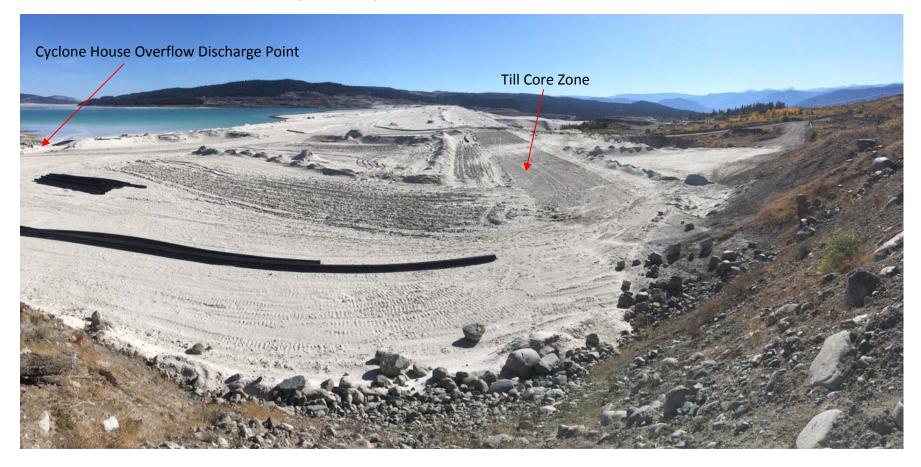


Photo II-A-2 South Abutment Overview (LL-2017-01)



Photo II-A-3 Upstream Dam Face – Facing South (overall 3.5H to 1V) (LL-2017-12)

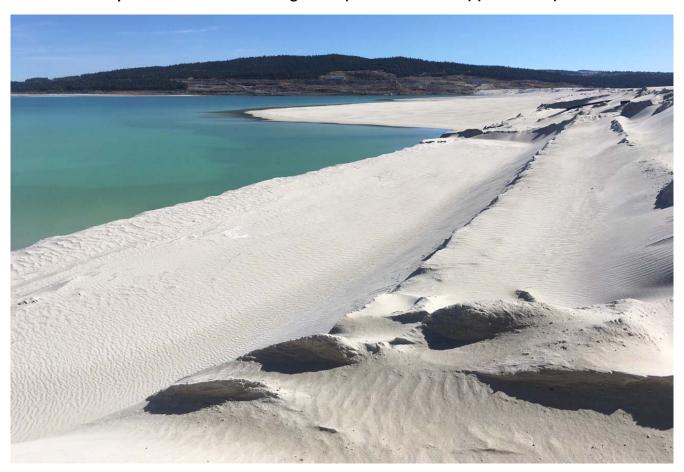


Photo II-A-4 Upstream Dam Face with Undercutting and Overhanging at Old Overflow Discharge Location – Facing East (LL-2017-13)



Photo II-A-5 Upstream Dam Face – Beach was Constructed using Hydraulic Sand Panels in 2015 – Facing North (LL-2017-11)



Page II-A-5

March 2018

Photo II-A-6 Upstream Dam Face – Undercutting at Beaching Location – Facing North (LL-2017-10)



Photo II-A-7 Pond at the Toe of the Upstream Dam Slope – Facing South (LL-2017-09)



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March 2018

Photo II-A-8 Downstream face at South Abutment - Facing North (LL-2017-02)



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March 2018

Photo II-A-9 South Dam – Erosion from Cyclone House Spill (LL-2017-06)



Photo II-A-10 South Dam – Major Drain Outlet with <1 L/min Flow Rate (LL-2017-05)



Photo II-A-11 South Dam Till Core Zone – Facing Northwest (LL-2017-08)



Photo II-A-12 North Dam Till Core Zone – Facing South (LL-2017-15)



Photo II-A-13 North Dam Downstream Toe – Facing South (LL-2017-16)



Photo II-A-14 North Dam Drain - Facing North (LL-2017-16)



Photo II-A-15 North Dam Downstream Slope – Facing North (LL-2017-17)



Photo II-A-16 North Buttress Berm North Slope – Facing West (LL-2017-18)

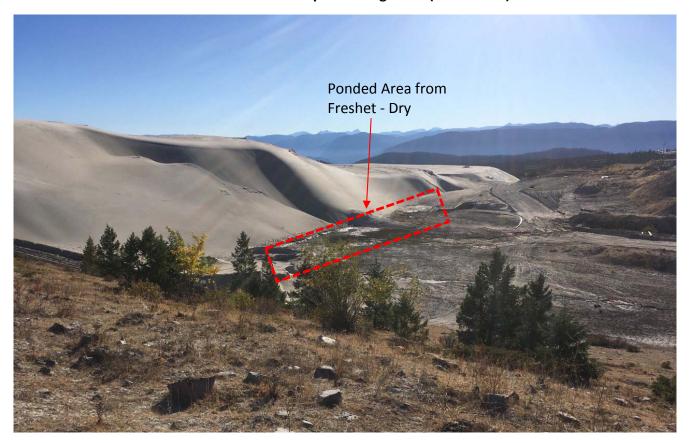


Photo II-A-17 North Buttress Berm South Slope – Facing Northwest (LL-2017-19)



Photo II-A-18 Toe and Downstream Face of South Dam - Facing Northwest (LL-2017-24)



Photo II-A-19 Downstream Slope of South Dam - Facing North (LL-2017-04)



Photo II-A-20 Downstream Toe at South Abutment - Facing East (LL-2017-07)



Photo II-A-21 Downstream Slope at South Abutment - Facing Northwest (LL-2017-03)



Photo II-A-22 Valley Buttress and Valley Buttress Berm Extension - Facing West (LL-2017-19)



Photo II-A-23 Finger Drain No.1 Measuring Weir (LL—FS-01) (LL-2017-20)



Photo II-A-24 UVD/LVD Drain Measuring Weir (LL—FS-02) (LL-2017-22)



Photo II-A-25 Seepage Pond 2 Weir (LL—FS-04) (LL-2017-21)



Photo II-A-26 Point A Weir – No Flow, Overgrown by Vegetation and No Longer Functional (KCB3) (LL-2017-23)



APPENDIX II-B

H-H Dam Inspection Photographs

Appendix II-B Inspection Photographs - H-H Dam

LEGEND:

- HH = H-H Dam.
- LL-2017-## refers to 2017 DSI waypoint shown on Figure 4.
- All photographs taken during inspection on September 26 and 27, 2017.

Photo II-B-1 Overview of Dam from West Abutment; RF2 fill overbuilt onto SG2 fill zone, requires cutting back during next crest raise (HH-2017-01)



Photo II-B-2 Overview of Dam from East Abutment and Exposed Bedrock from 2015 Abutment Foundation Preparation Works (HH-2017-09)



Photo II-B-3 Till Core Zone and RF1 Zone - Facing Southwest; sporadic vegetation growth (HH-2017-05)



Photo II-B-4 Till Core Zone - Facing Northeast; upstream buffer at HHVWP15-22; RF2 material has been placed on Till Core Zone (HH-2017-06)



Photo II-B-5 Longitudinal cracking in Till Core Zone from Sta. 1+200 to Sta. 1+400 (not continuous) along centerline of Dam, up to 20 cm deep (HH-2017-04)



Photo II-B-6 Existing longitudinal crack on downstream edge of SG2 fill zone, with no signs of fresh movement or vertical displacement across crack; crack is >30 cm deep and up to 20 cm wide; hairline crack approx. 1 m upstream of the main crack (HH-2017-07)



Page II-B-7

Photo II-B-7 Existing longitudinal crack on downstream edge of SG2 fill zone, with no signs of fresh movement or vertical displacement across crack; crack is >30 cm deep and up to 20 cm wide; hairline crack approx. 1 m upstream of the main crack (HH-2017-07)



Photo II-B-8 Upstream Face of Dam - Facing Northeast; RF2/RF1 buffer zone on the upstream edge of dam crest, approx. 1 m high; available buffer is approx. 0.5 m (HH-2017-08)



Photo II-B-9 Downstream Face of Dam - Facing Northeast (HH-2017-02)



Photo II-B-10 Downstream Face of Dam - Facing Northeast (HH-2017-15)

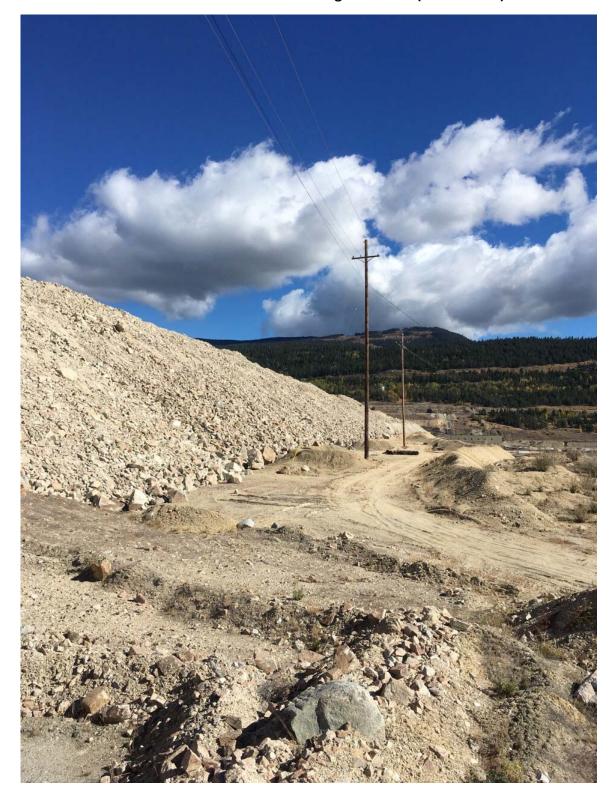


Photo II-B-11 Downstream Toe of Dam - Facing Northeast (HH-2017-14)



Photo II-B-12 Downstream Toe of Dam - Facing West (HH-2017-13)



Photo II-B-13 East Abutment - Facing Northeast; new VWP, inclinometer and settlement system installed in 2017 behind the H-H Pumphouse lock-block retaining wall (HH-2017-12)

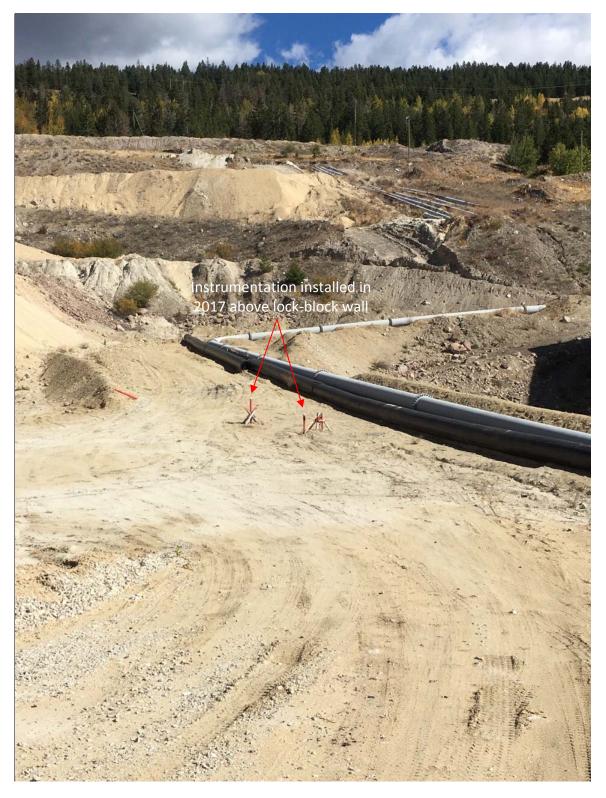


Photo II-B-14 Downstream Face of Dam - Facing West (HH-2017-10)



Photo II-B-15 Lock-block Retaining Wall at Toe of Dam; new survey prisms (HH-2017-11)



APPENDIX II-C

L-L Dam Seepage Slimes Ponds Inspection Photographs

Appendix II-C Inspection Photographs - L-L Dam Seepage and Slimes Pond

LEGEND:

- SS = L-L Dam seepage and slimes ponds.
- SS-2017-## refers to 2017 DSI waypoint shown on Figure 2.
- All photographs taken during inspection on September 26 and 27, 2017.

Photo II-C-1 Overview of Seepage and Slimes Collection Ponds – Facing Northwest (SS-2017-01)

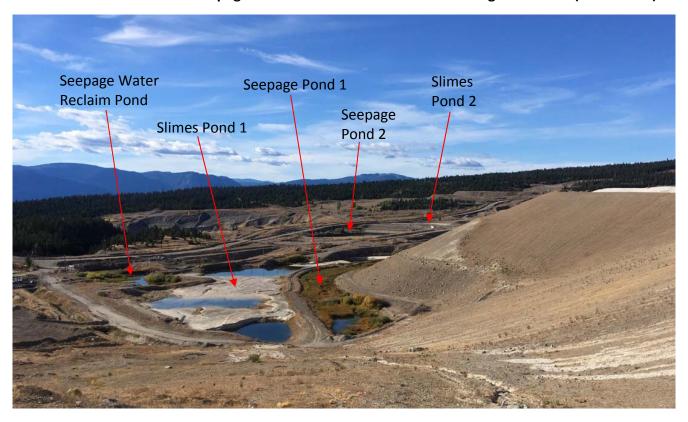


Photo II-C-2 Slimes Pond 1 – Facing South (SS-2017-04)



Photo II-C-3 Slimes Pond 1 – Facing Northwest (SS-2017-02)



Photo II-C-4 South End of Seepage Pond 1 – Facing Northwest (SS-2017-03)



Photo II-C-5 Seepage Pond 1 Outlet (SS-2017-03)



Photo II-C-6 Slimes Pond 2 – Slimes excavate/removed in 2017 – Facing North (SS-2017-05)



Photo II-C-7 Slimes Pond 2 Crest – Facing East (SS-2017-06)

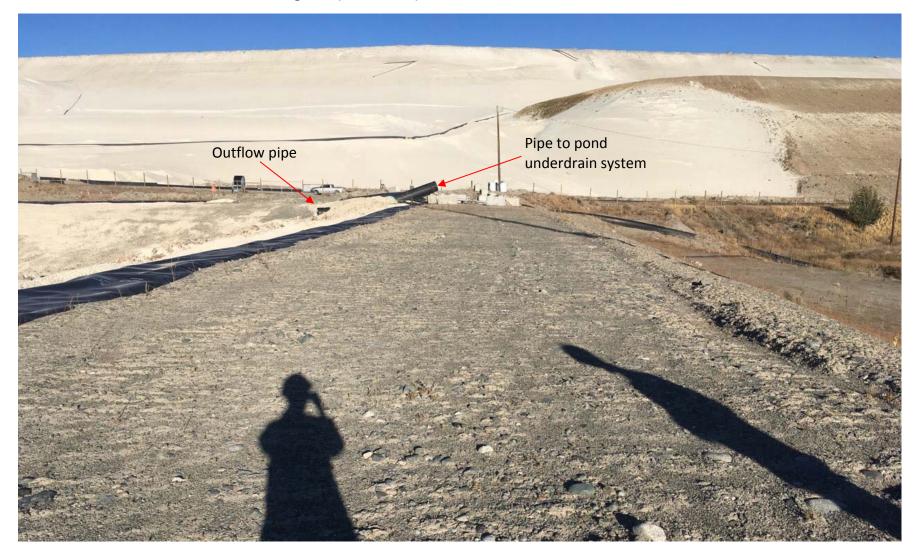


Photo II-C-8 Slimes Pond 2 Downstream Face – Facing East (SS-2017-06)



Photo II-C-9 Seepage Pond 2 Overview – Facing Southeast (SS-2017-06)



Photo II-C-10 Seepage Pond 2 Outflow Pipe – Facing West; Outlet not yet repaired (SS-2017-08)



Photo II-C-11 Seepage Pond 2 Tear in Liner Repaired in 2017 – On east side of pond (SS-2017-08)



Photo II-C-12 Seepage Pond 2 Outlet Pipe and Lock-block Wall (SS-2017-07)



Photo II-C-13 Seepage Water Reclaim Pond – Facing North (SS-2017-09)



Photo II-C-14 Seepage Water Reclaim Pond – Outfall from Woods Creek Diversion – Facing Northwest (SS-2017-09)

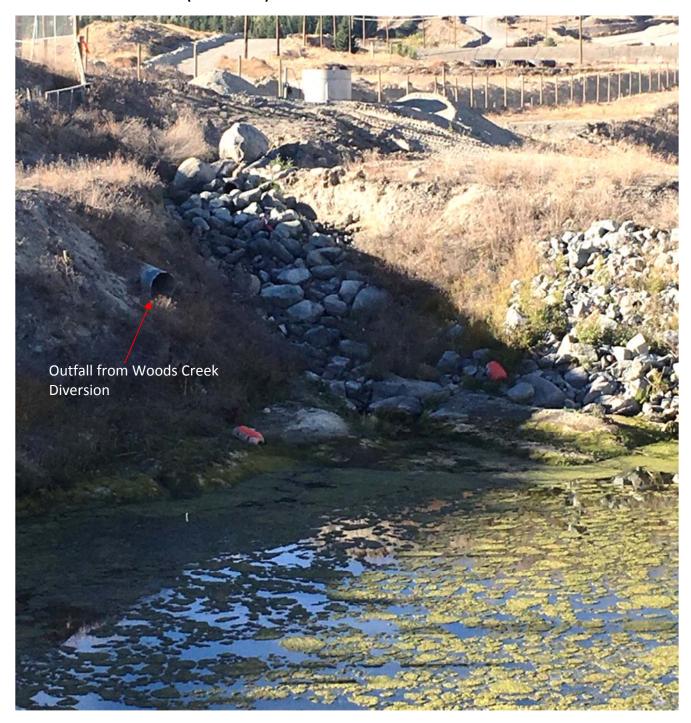


Photo II-C-15 Slimes Pond 3 (partially constructed) – Facing West (SS-2017-01)



Photo II-C-16 Slimes Pond 3 (partially constructed) – Facing South (SS-2017-10)



Photo II-C-17 Slimes Pond 3 (partially constructed) – Ponded Water on Crest from Spring Lines in Slope – Facing East (SS-2017-11)



APPENDIX II-D

24 Mile Lake Inspection Photographs

Appendix II-D Inspection Photographs - 24 Mile Lake

LEGEND:

- ML = 24 Mile Lake.
- ML-2017-## refers to 2017 DSI waypoint shown on Figure 4.
- All photographs taken during inspection on September 26 and 27, 2017.

Photo II-D-1 Overview of 24 Mile Lake, facing H-H Dam (north); >10 m freeboard observed (ML-2017-03)



Photo II-D-2 West side of Lake; multiple erosion gullies observed (ML-2017-01)



Photo II-D-3 Outflow pump location, facing H-H Dam (northwest) (ML-2017-02)



Photo II-D-4 East side of Lake (ML-2017-04)



Photo II-D-5 Southeast Side of Lake (ML-2017-01)



Photo II-D-6 H-H Pumphouse outflow channel, facing south (ML-2017-05)



Photo II-D-7 H-H Pumphouse emergency dump outflow pipes, facing southwest (ML-2017-06)



APPENDIX III

Reference Dam Design Drawings

Appendix III Design Drawings

III-A - L-L Dam

Design drawings are taken from the following report:

 Klohn Crippen Berger. 2010. "Tailings Storage Facility - Ultimate Dam Crest Elevation for 2028 Mine Plan", August 20.

List of drawings -

| Figure 4 | L-L Dam General Arrangement Ultimate Dam |
|-----------|--|
| Figure 9 | L-L Ultimate Dam Station 1+160 |
| Figure 10 | L-L Ultimate Dam Station 1+600 |
| Figure 11 | L-L Ultimate Dam Station 2+000 |
| Figure 12 | L-L Ultimate Dam Station 2+200 |
| Figure 13 | L-L Ultimate Dam Station 2+550 |
| Figure 14 | L-L Ultimate Dam Station 2+900 |
| Figure 15 | L-L Ultimate Dam Station 3+200 |
| Figure 17 | Ultimate Dam Spillway, LLO and Drainage Ditches Preliminary Arrangement – Plan |
| Figure 18 | Ultimate Dam Spillway and LLO Preliminary Profiles & Typ. Sections |
| | Figure 9 Figure 10 Figure 11 Figure 12 Figure 13 Figure 14 Figure 15 Figure 17 |

III-B - H-H Dam

Design drawings are taken from the following report:

 Klohn Crippen Berger. 2010. "Tailings Storage Facility - Ultimate Dam Crest Elevation for 2028 Mine Plan", August 20.

List of drawings -

- Figure 5 H-H Dam General Arrangement Ultimate Dam
- Figure 16 H-H Ultimate Dam Station 1+500

III-C - L-L Dam Seepage and Slimes Ponds

C-1 – Seepage Pond 1

No design drawings available

C-2 - Seepage Pond 2

- THVCP Drawing 1011-021-SlimePond-ASB.dwg "Slimes Pond Plan and Sections", September 2012 (as-built)
- THVCP Drawing "Seepage Pond #2 Ditch Design", February 2014

C-3 - Slimes Pond 1

No design drawings available

C-4 – Slimes Pond 2

- Drawing No. D-74001 "Slimes Pond 2 Earthworks Plan", Oct 2014.
- Drawing No. D-74002 "Slimes Pond 2 Cross Sections and Liner Details", Oct 2014.
- Drawing No. D-74003 "Slimes Pond 2 Outflow Pipe and Underdrain Sump Details", Oct 2014.
- Drawing No. D-74004 "Slimes Pond 2 Fill and Placement Specifications", Oct 2014.

C-5 – Seepage Water Reclaim Pond

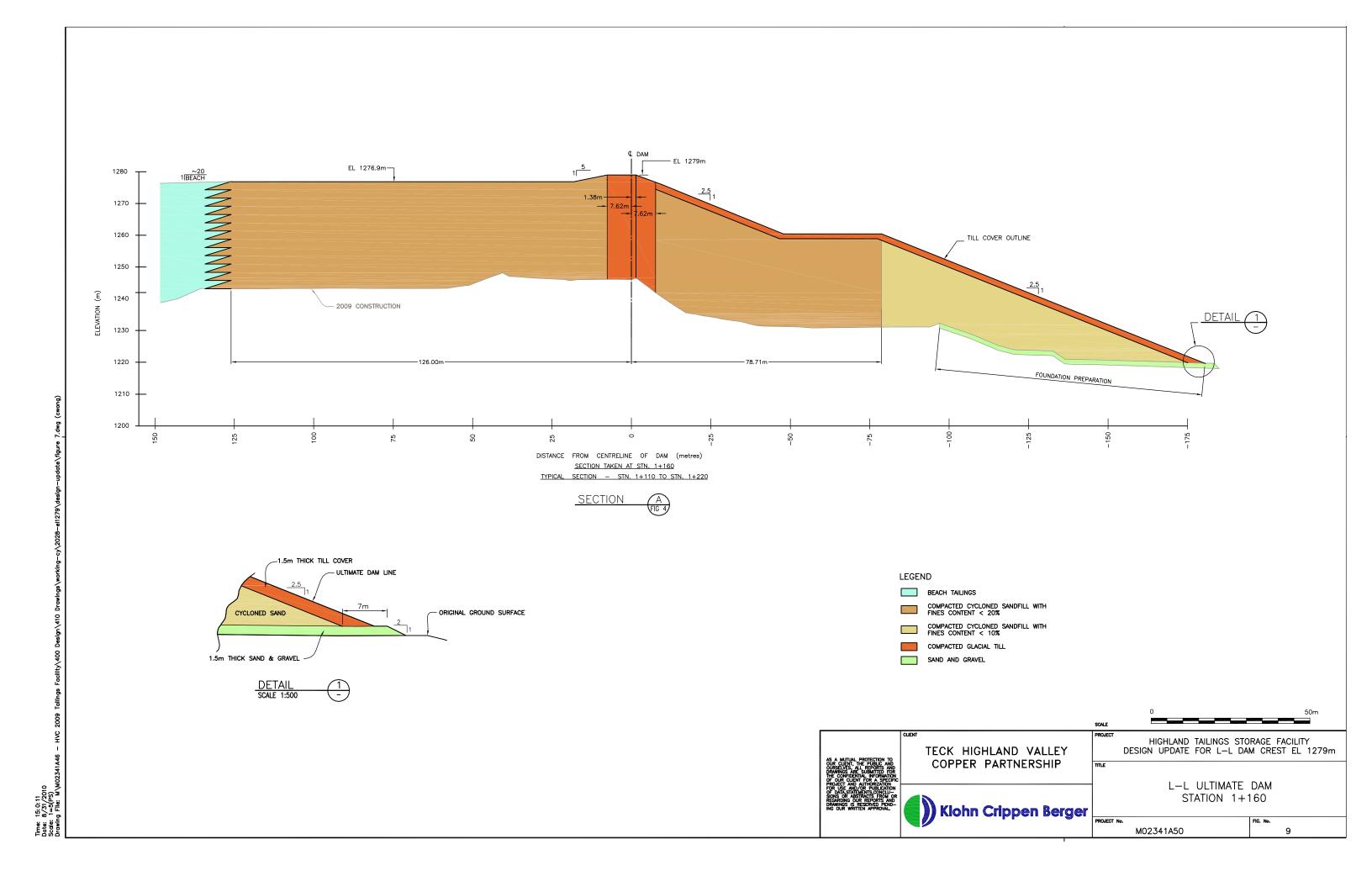
Drawing 111-803-140 Rev 1 "Water Collection Dam L-L Axis Plan and Sections" 1977

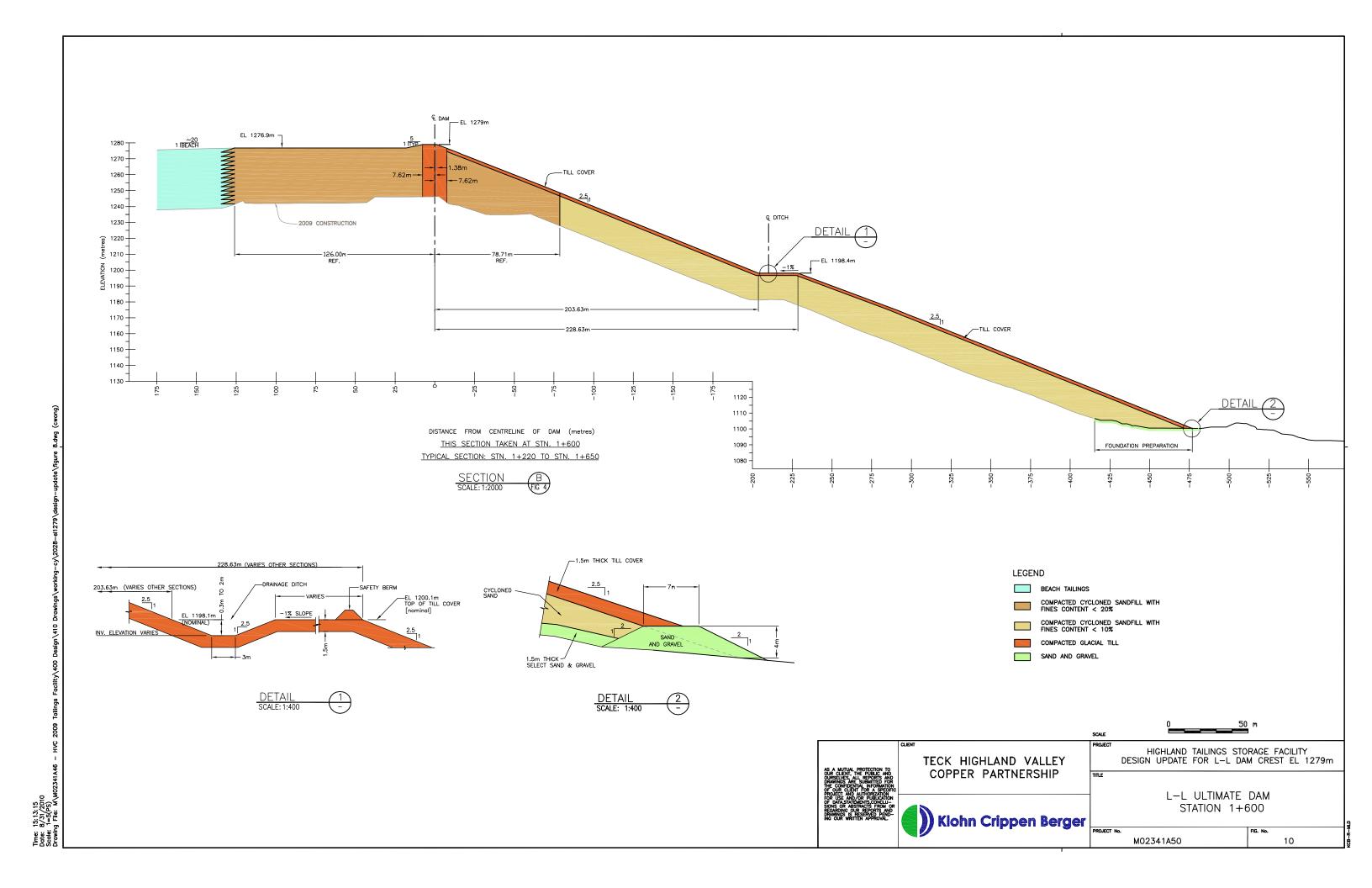
III-C - 24 Mile Lake

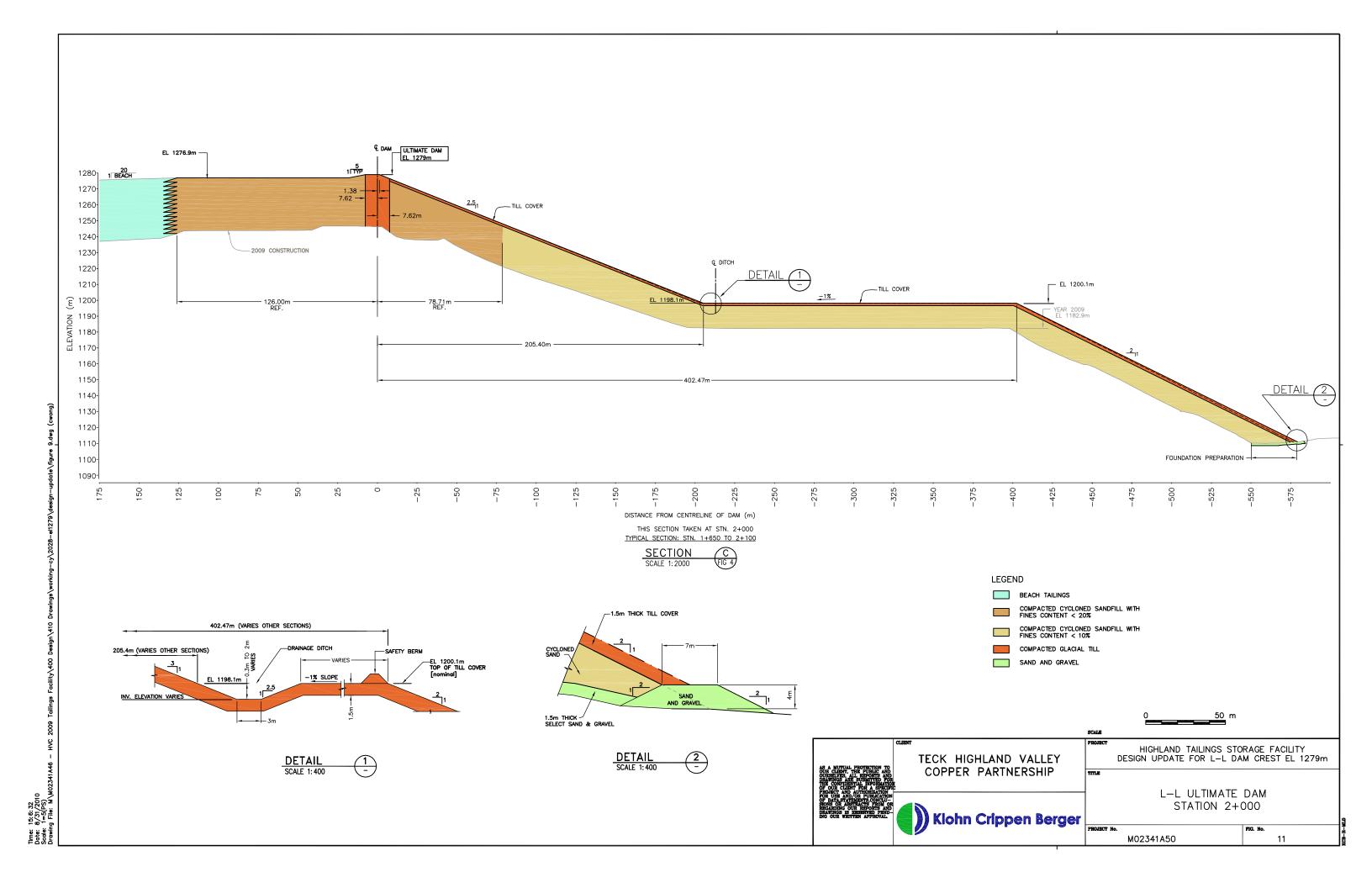
No design drawings available

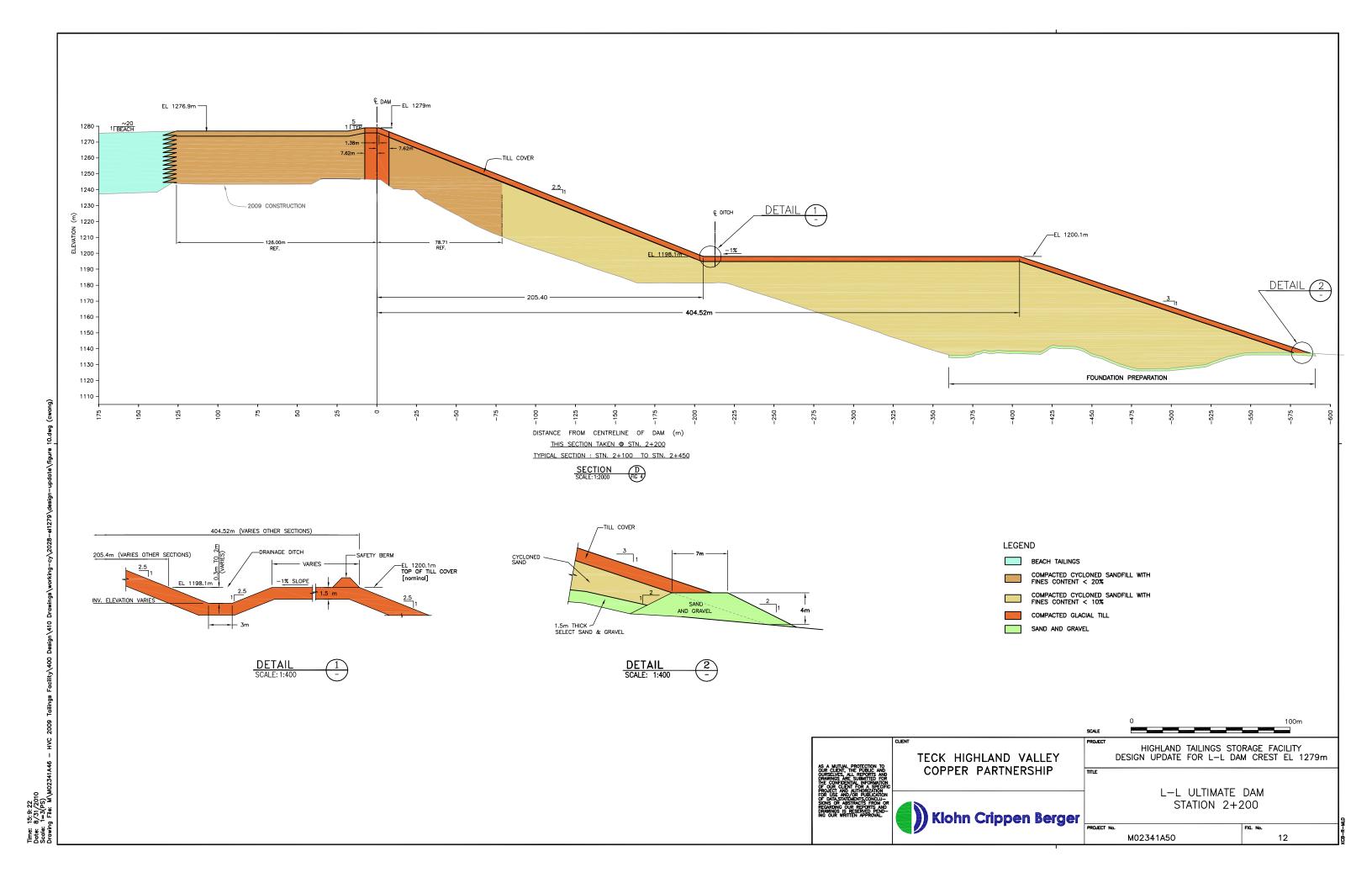
APPENDIX III-A

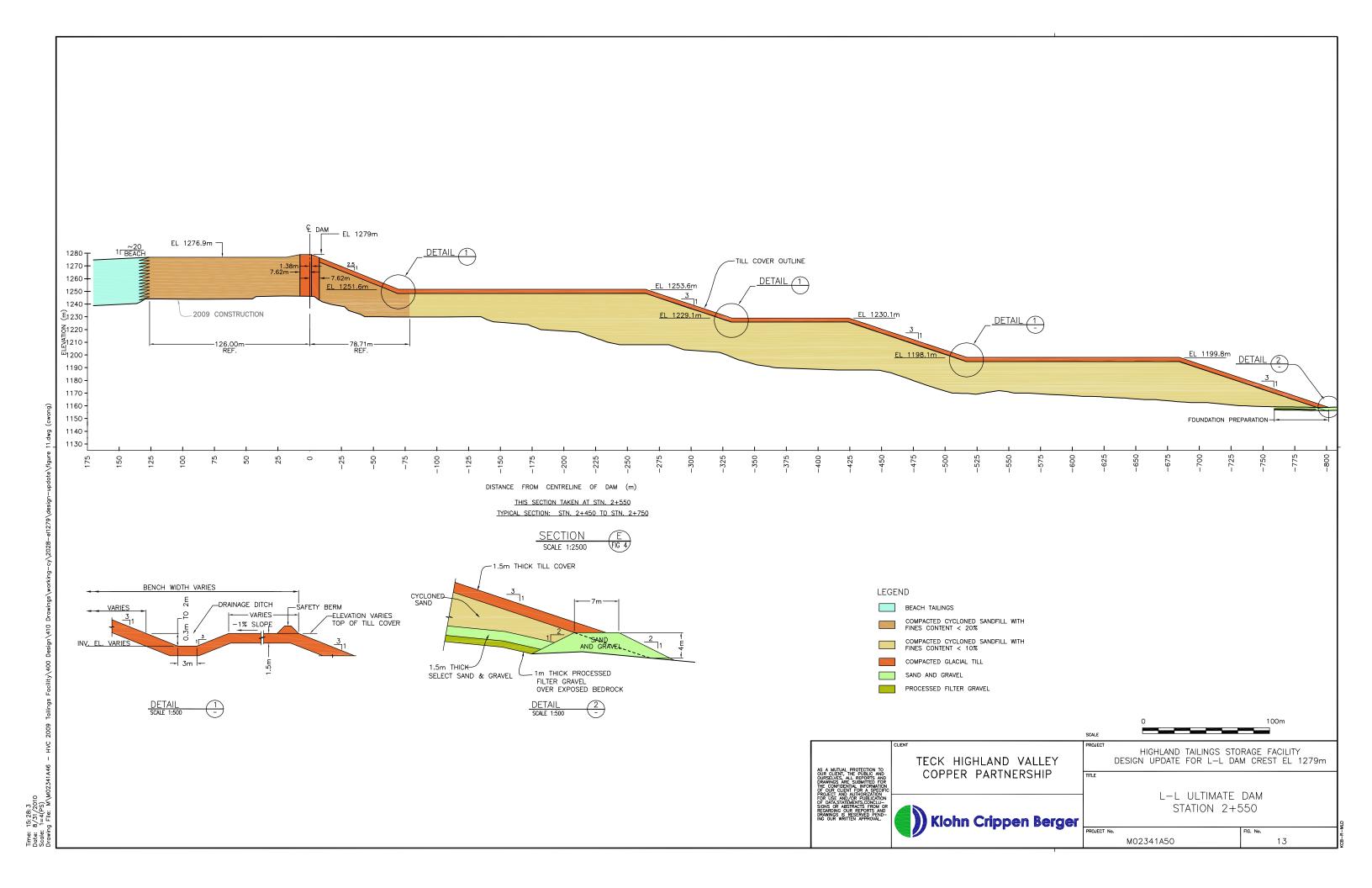
L-L Dam

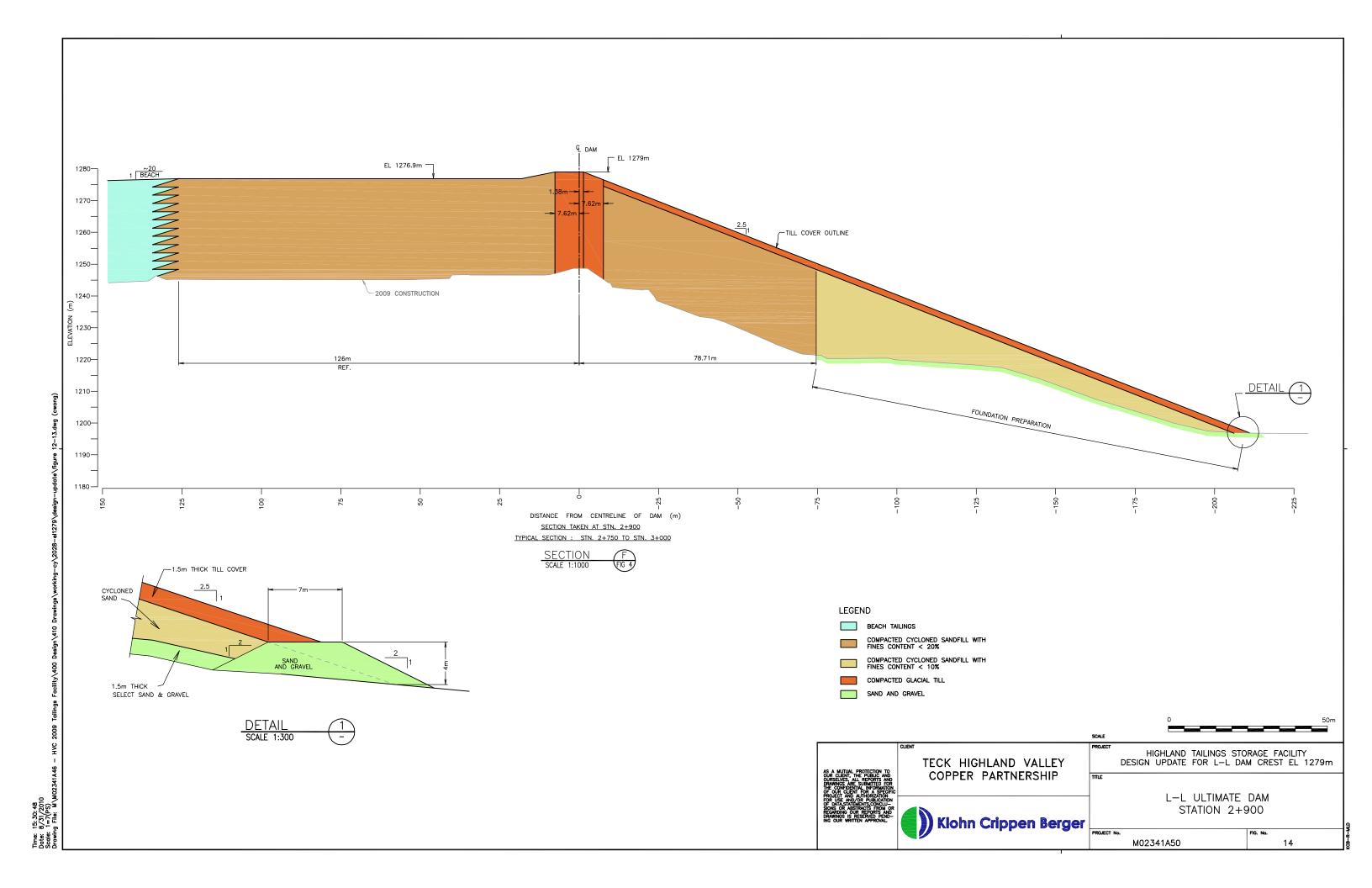


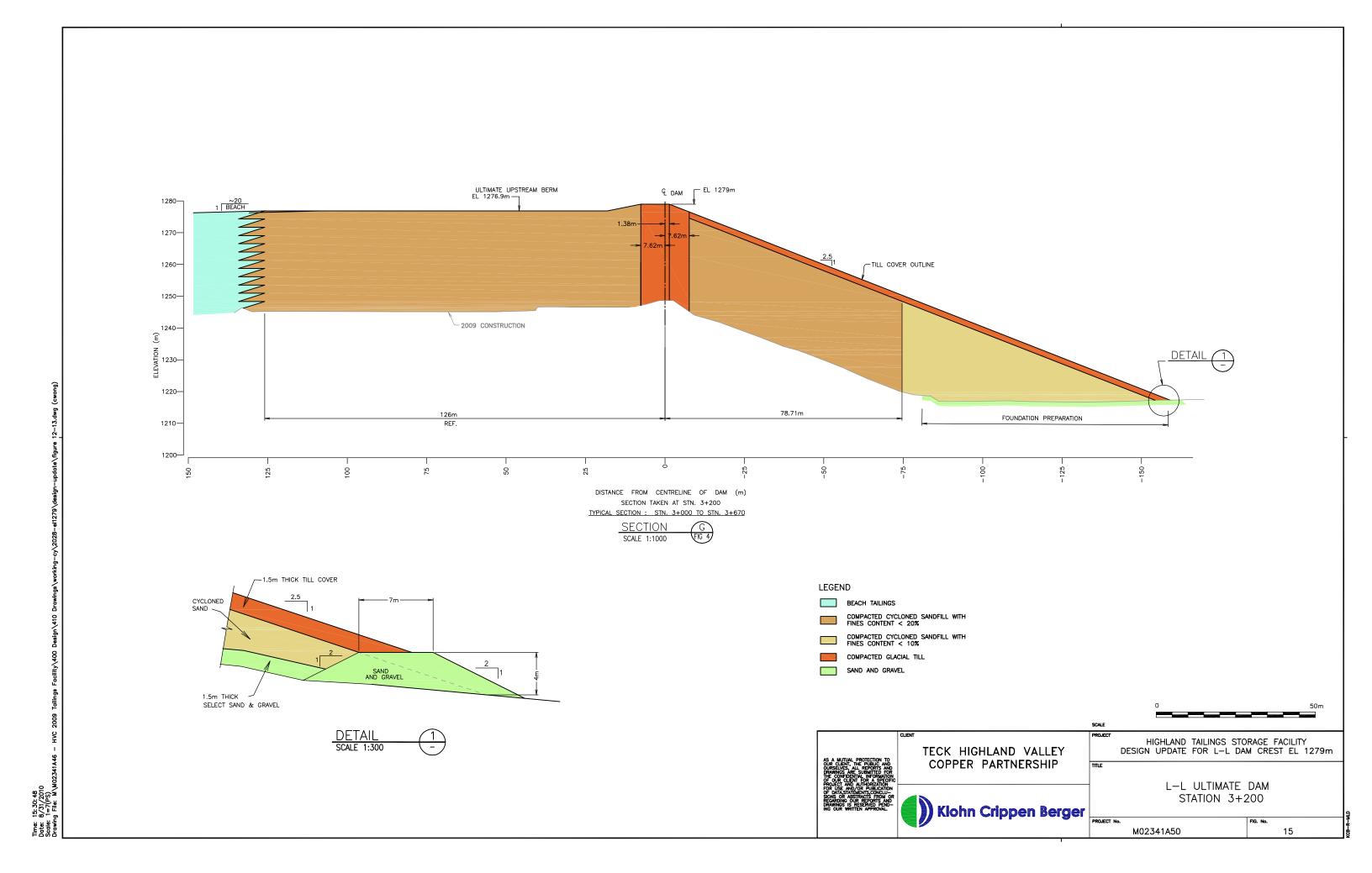


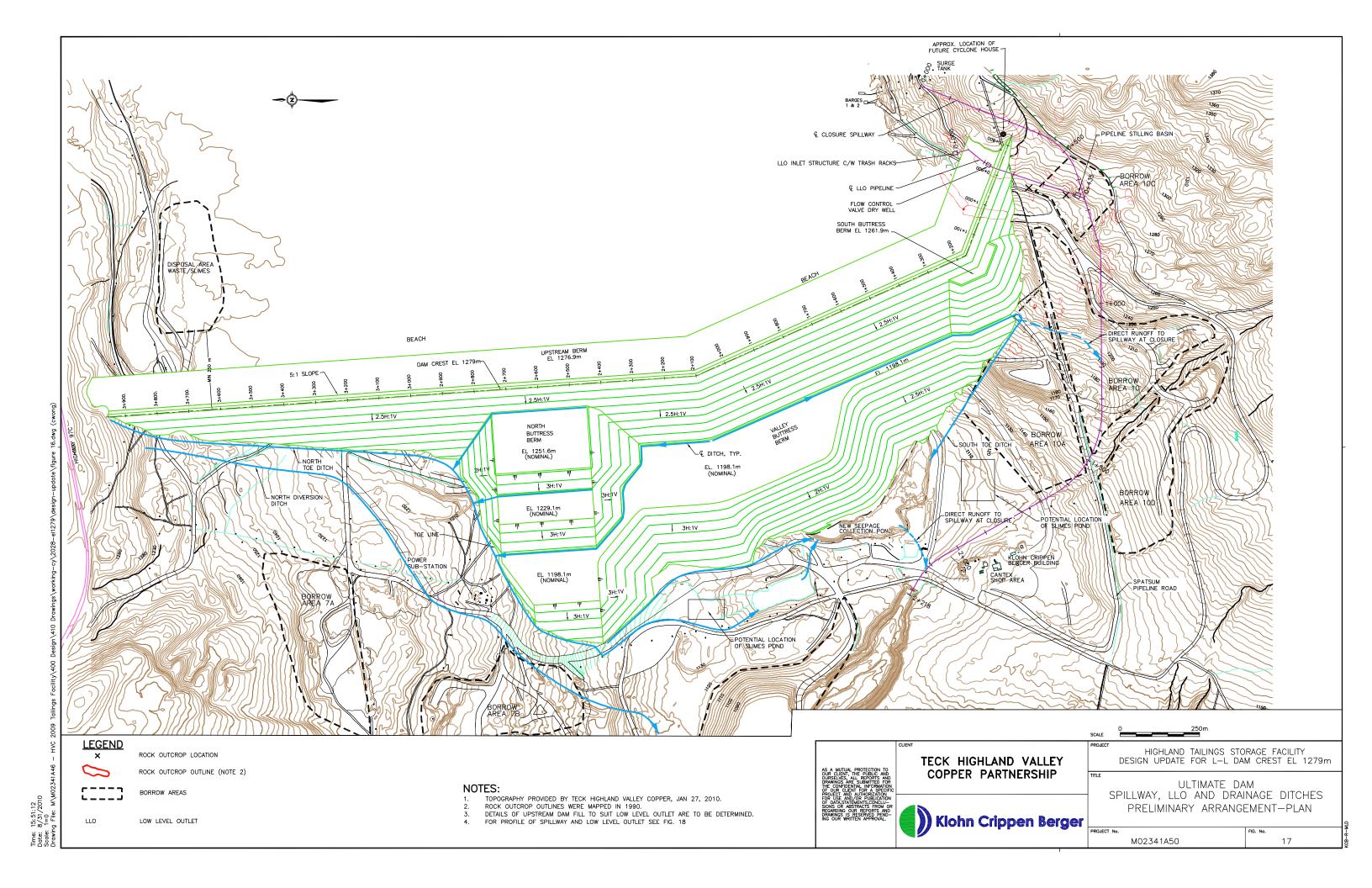


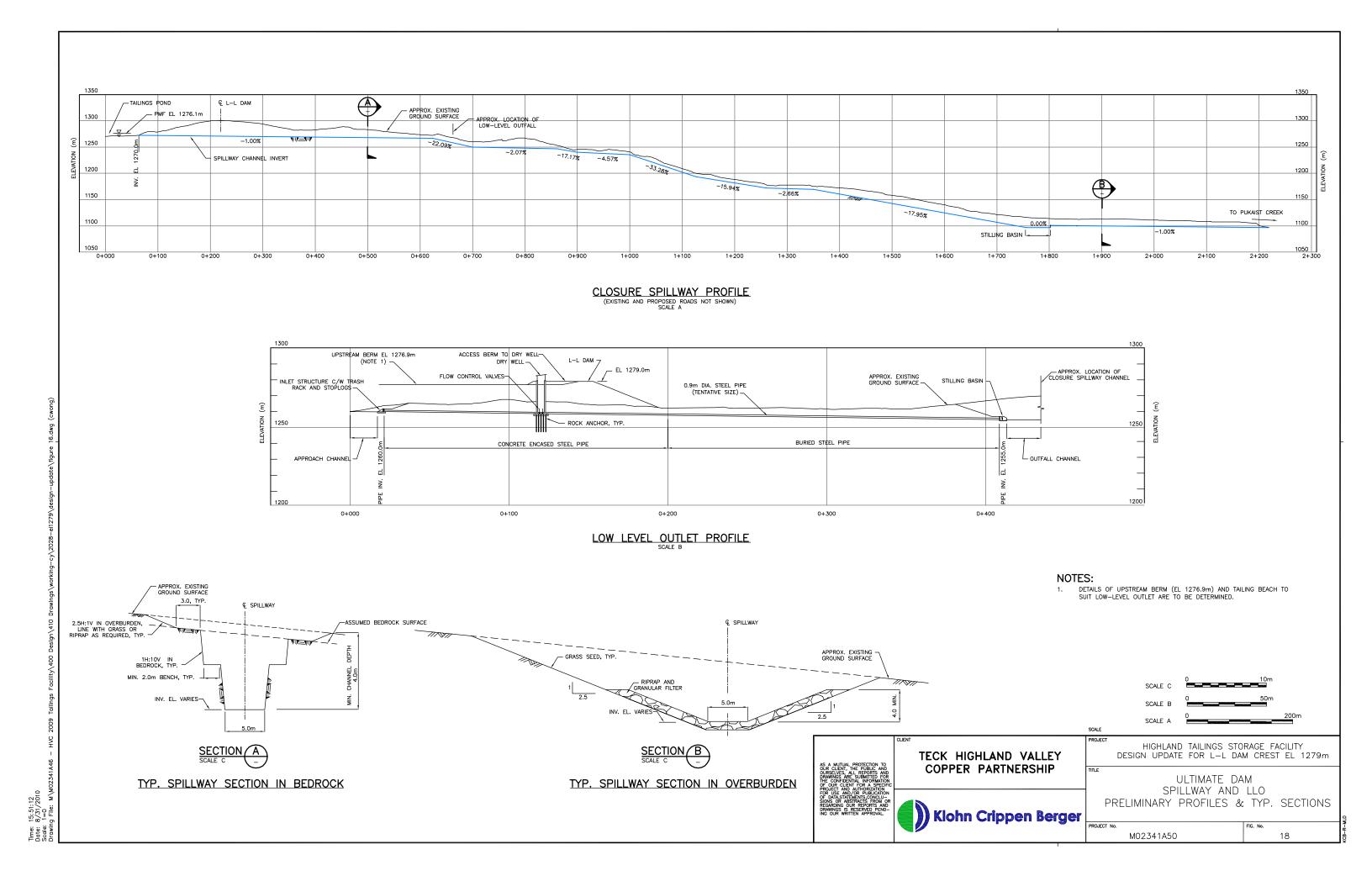






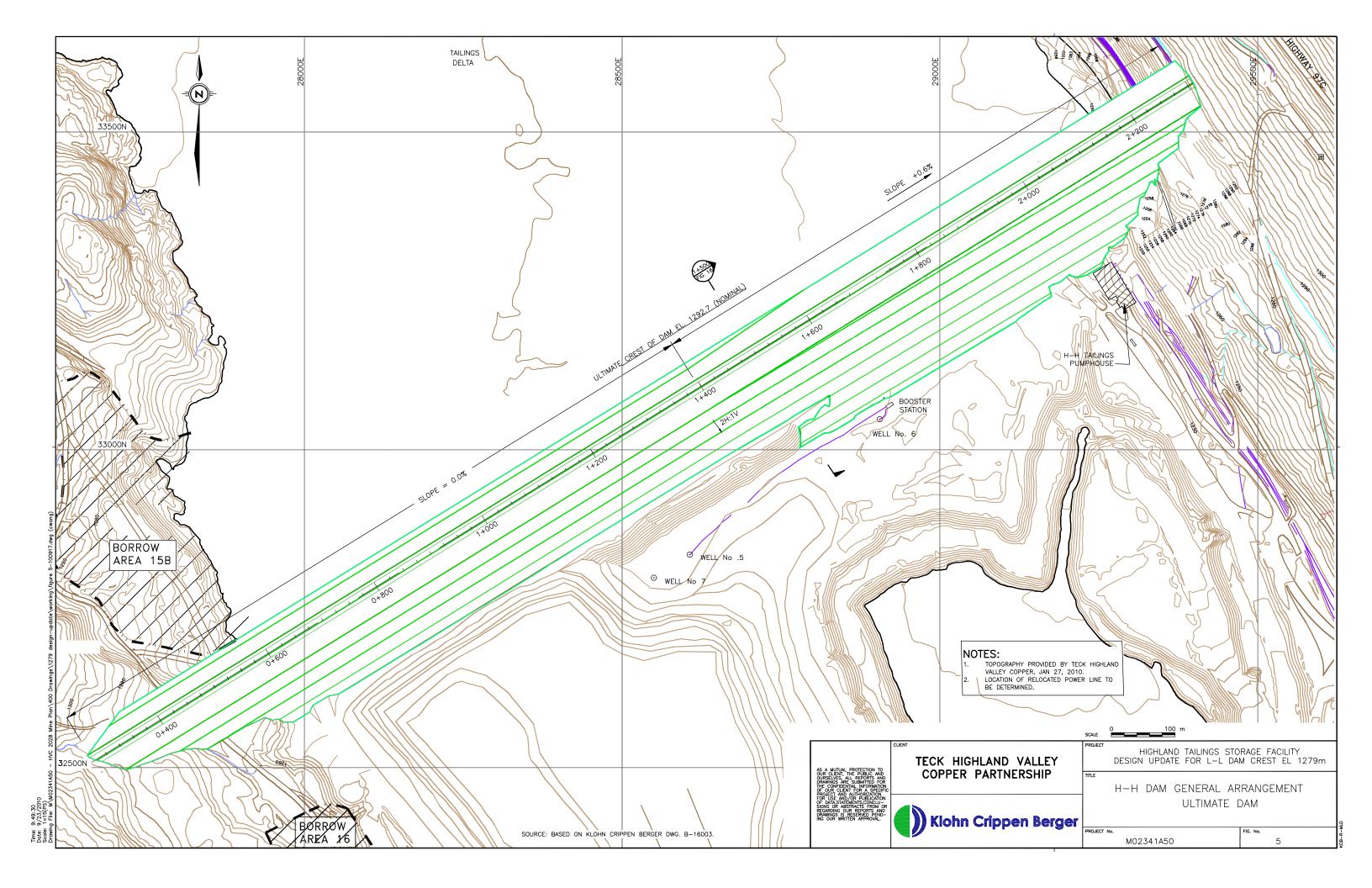


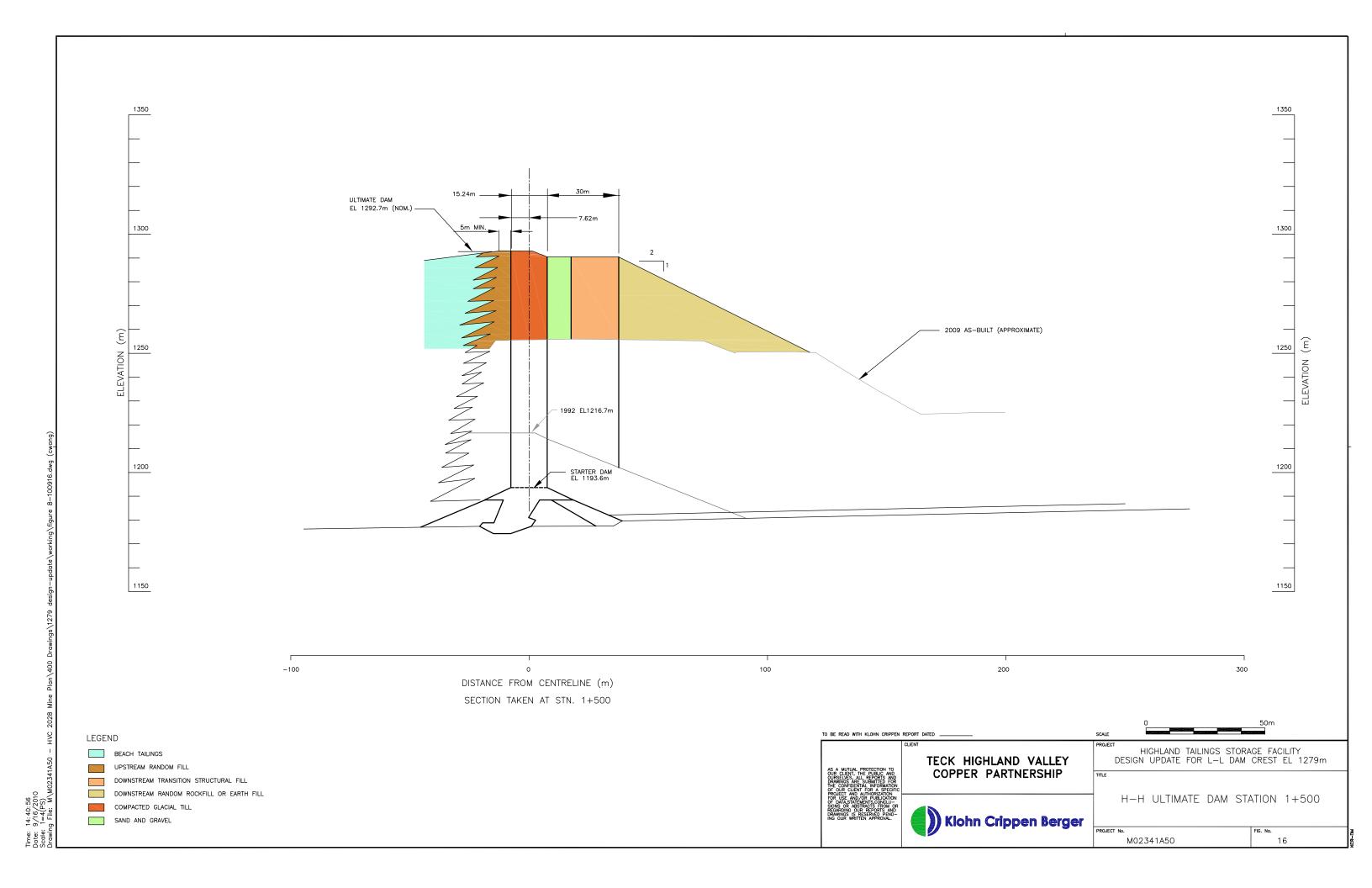




APPENDIX III-B

H-H Dam



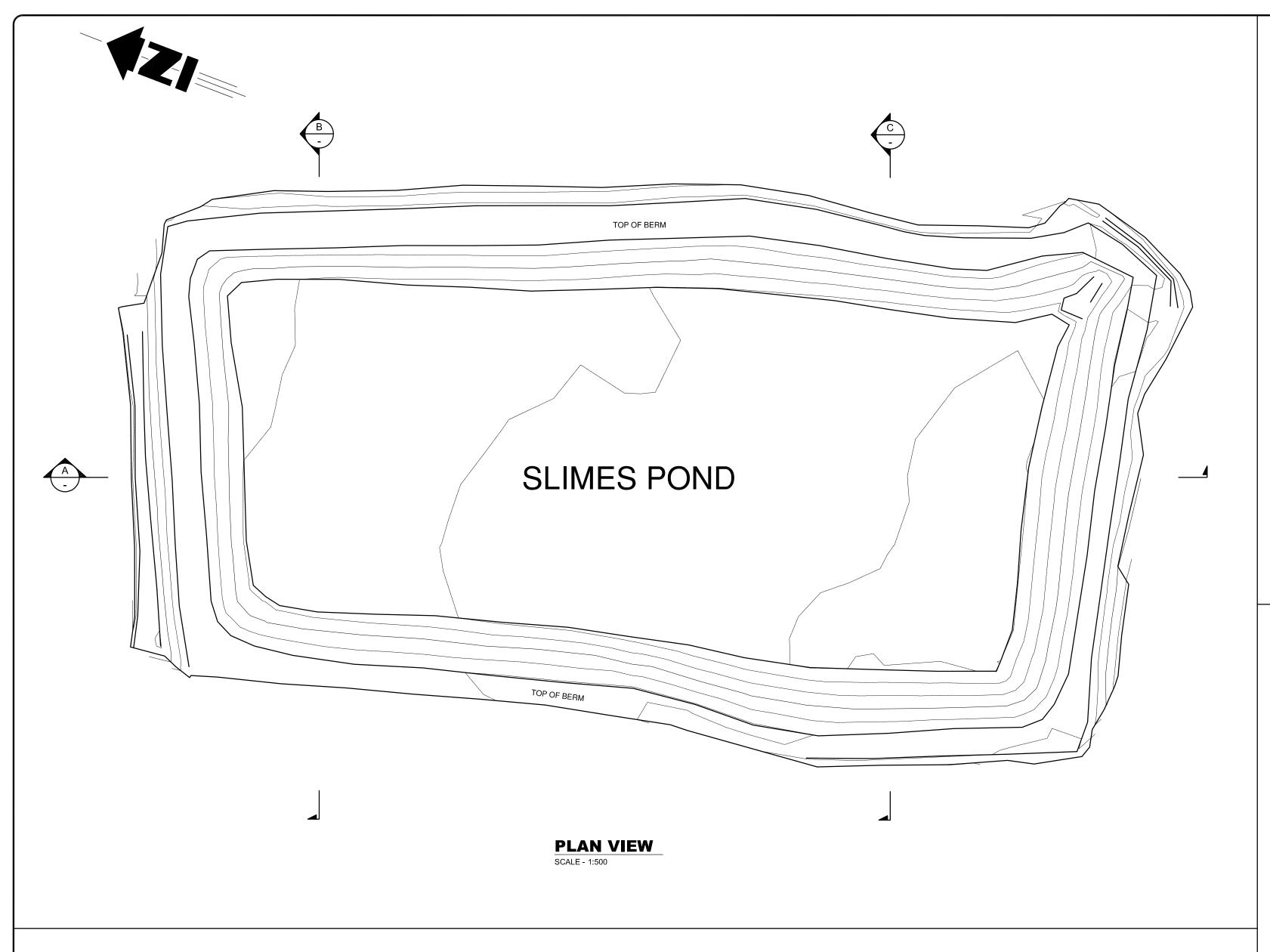


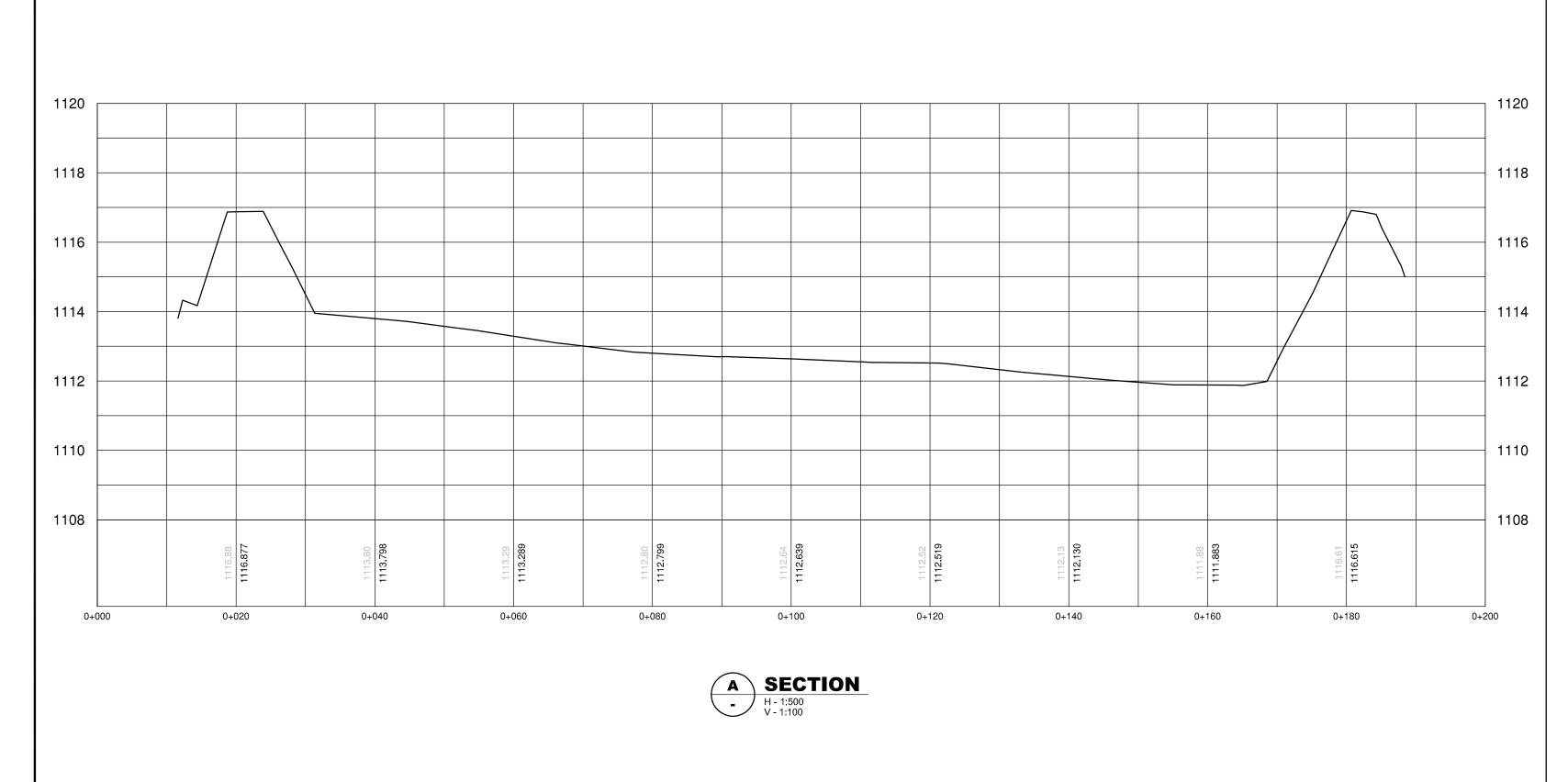
L-L Dam Seepage and Slimes Ponds

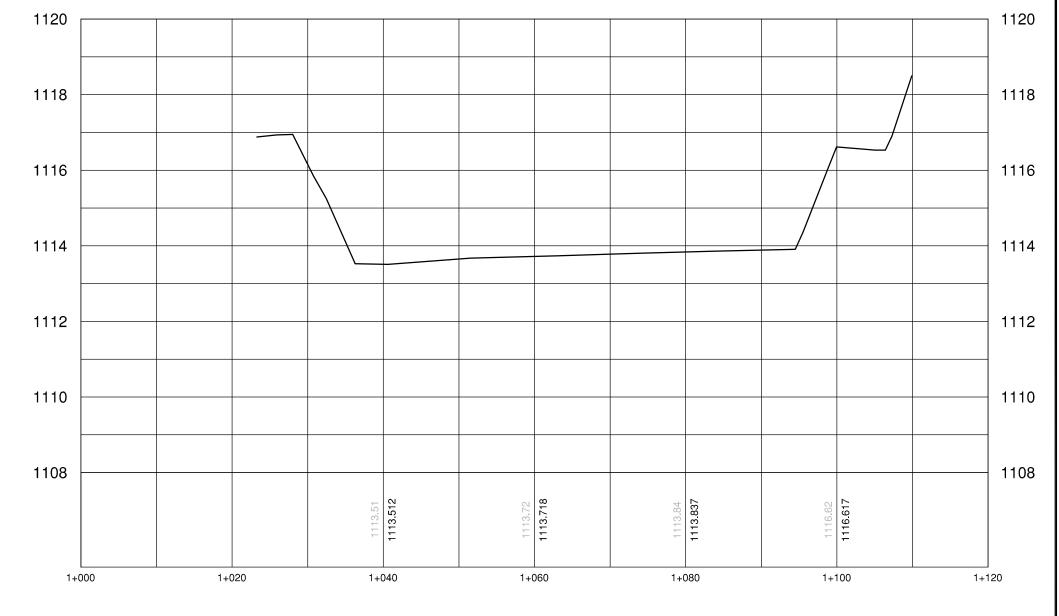
Seepage Pond 1

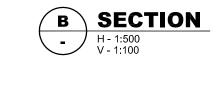
Drawings Not Available

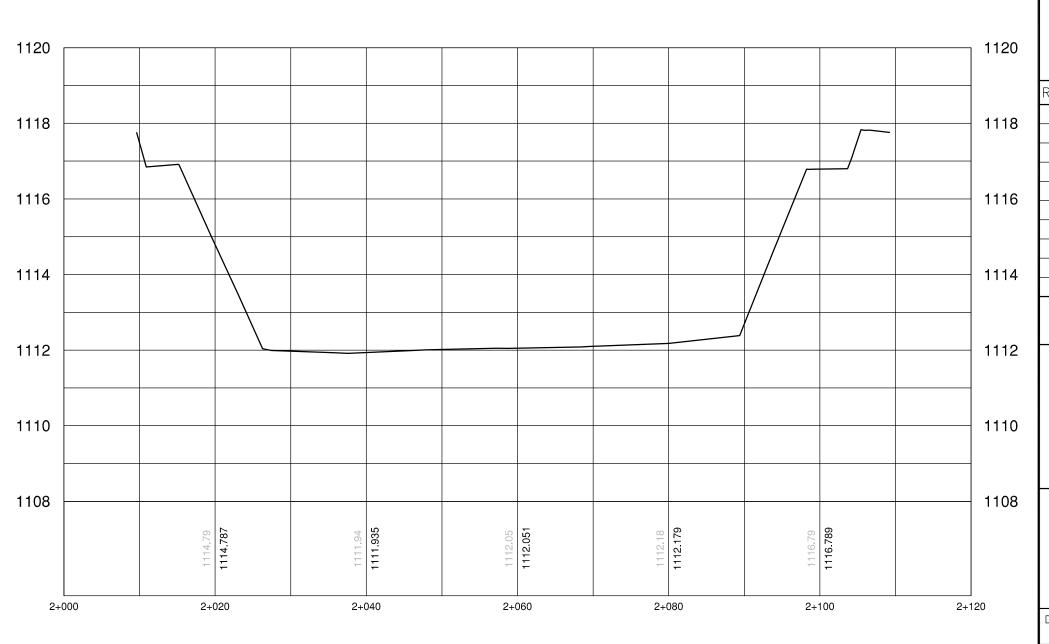
Seepage Pond 2













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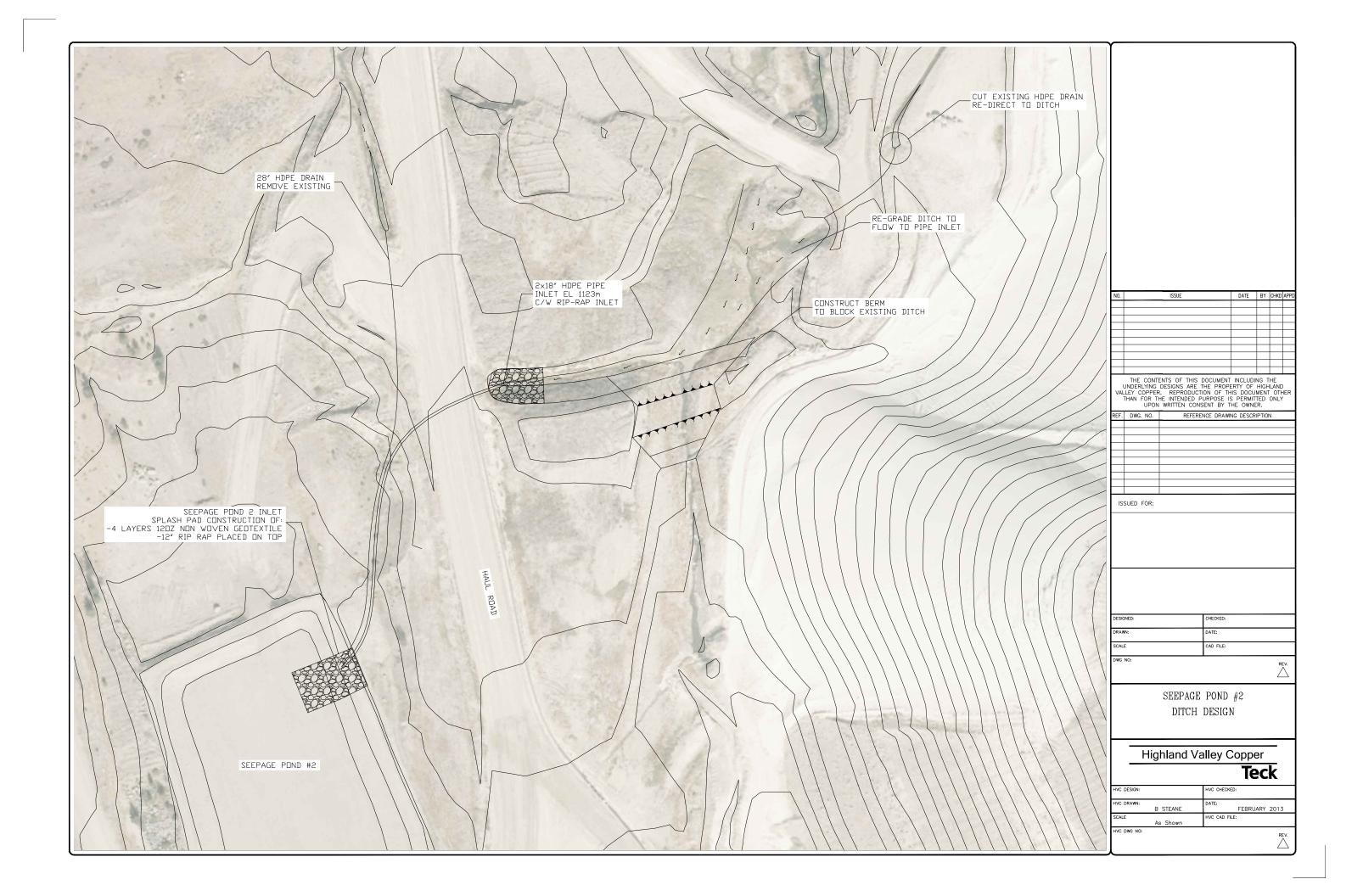
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SLIMES POND PLAN AND SECTIONS

Highland Valley Copper **Teck**

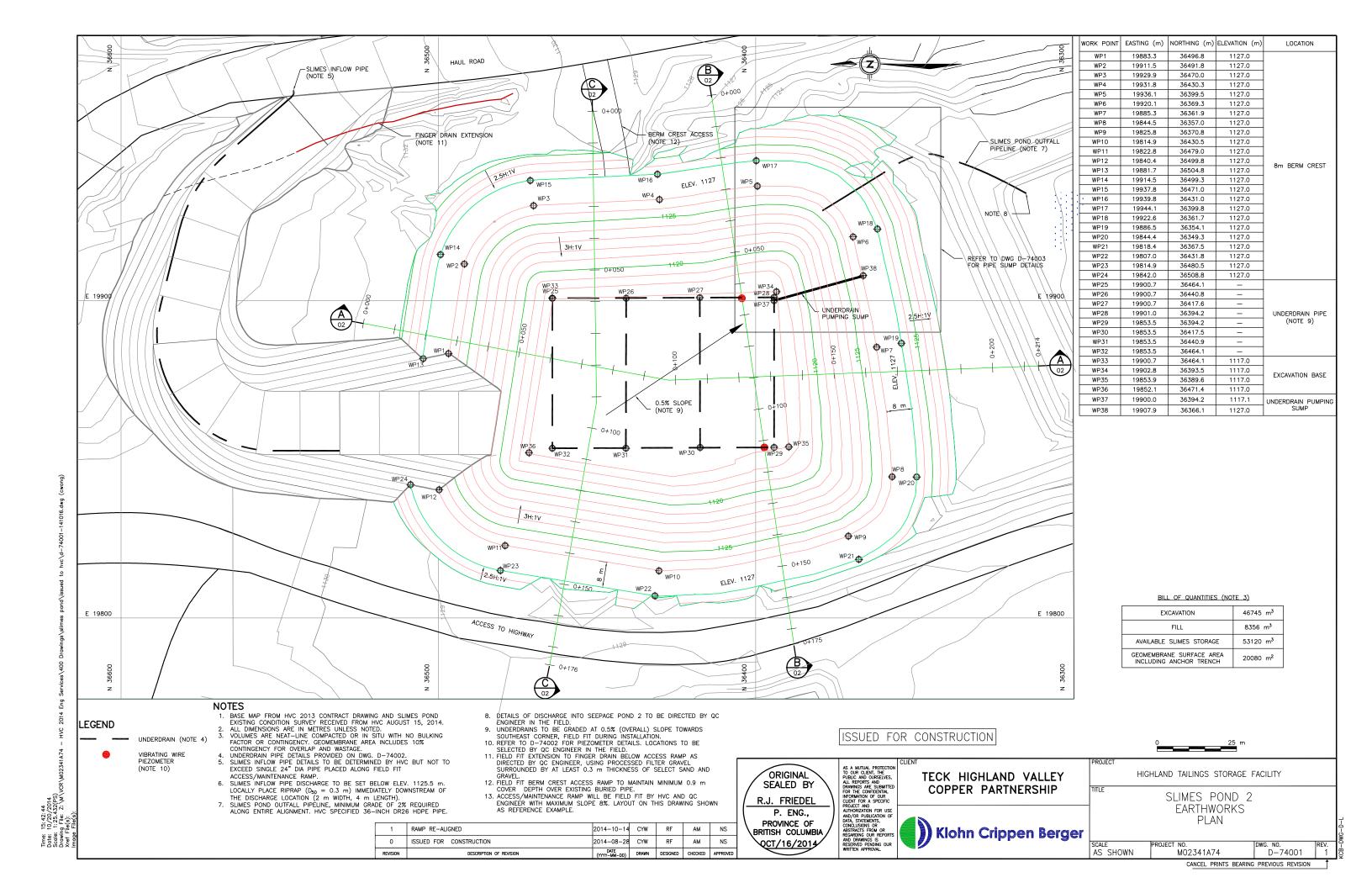
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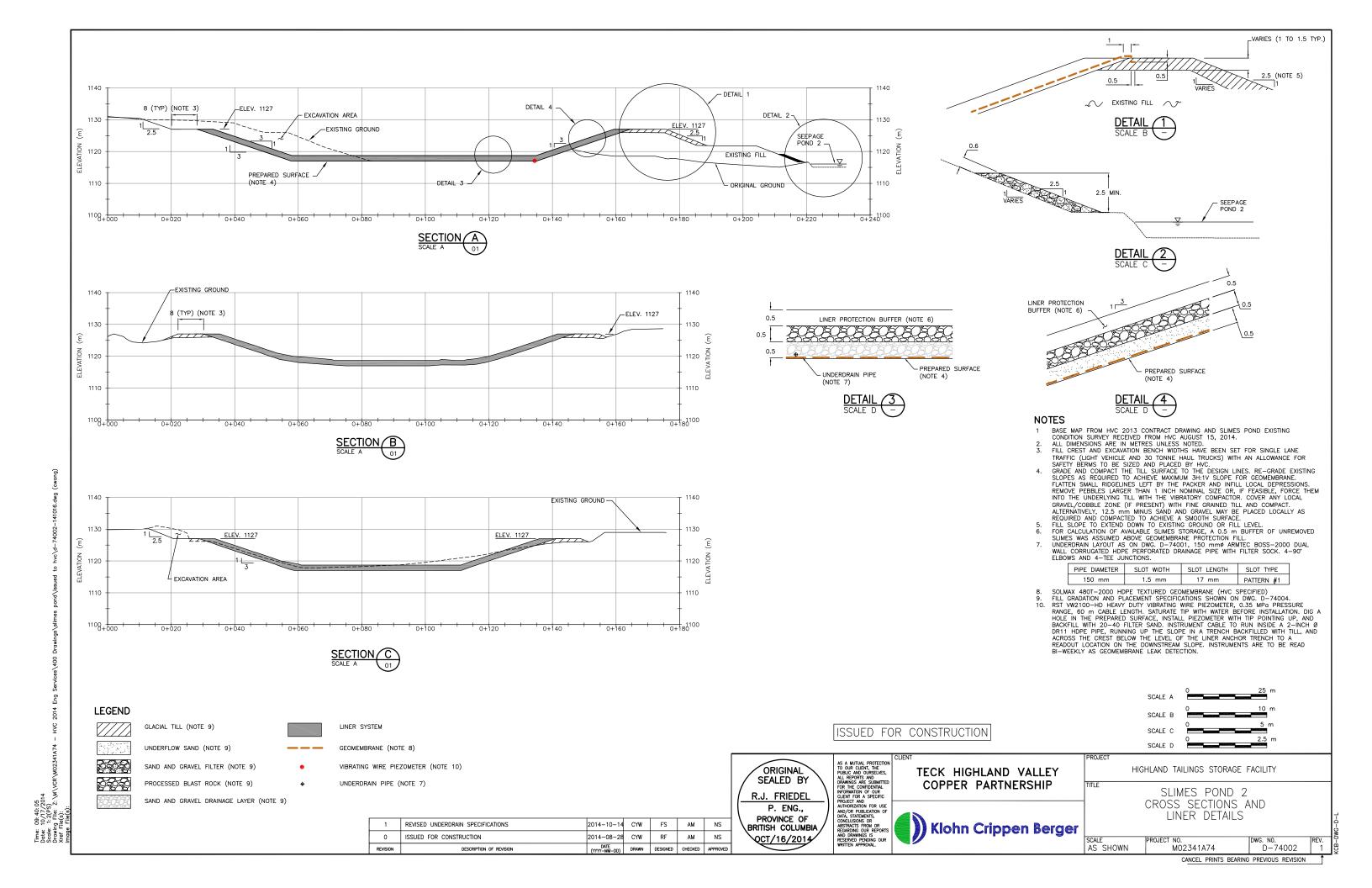


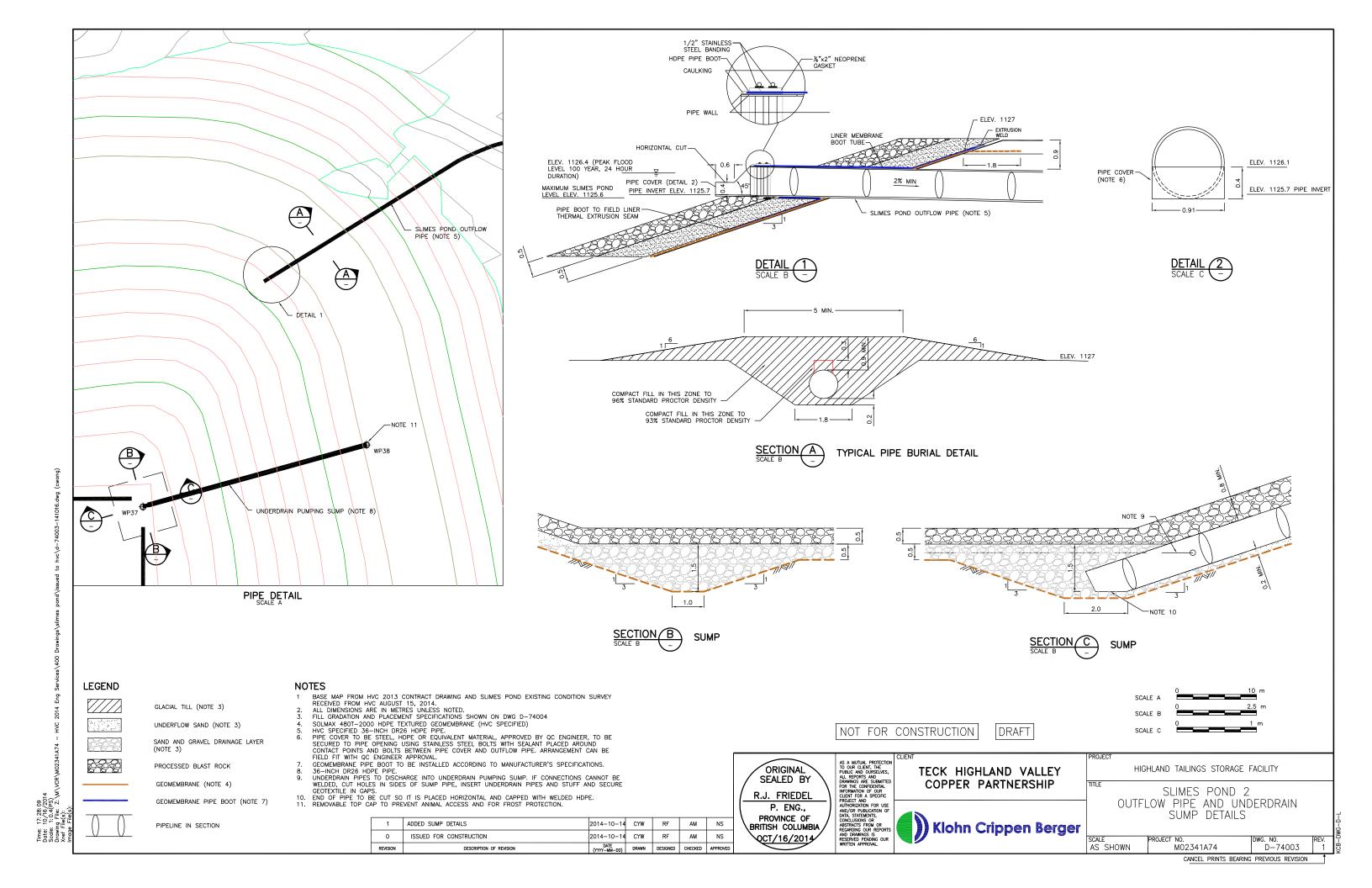
Slimes Pond 1

Drawings Not Available

Slimes Pond 2







| | MINIMUM | MAXIMUM |
|------------------|---------|---------|
| D ₁₀₀ | 450 | 510 |
| D ₈₅ | 360 | 420 |
| D ₅₀ | 300 | 345 |
| D ₁₅ | 120 | 180 |

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Klohn Crippen Berger

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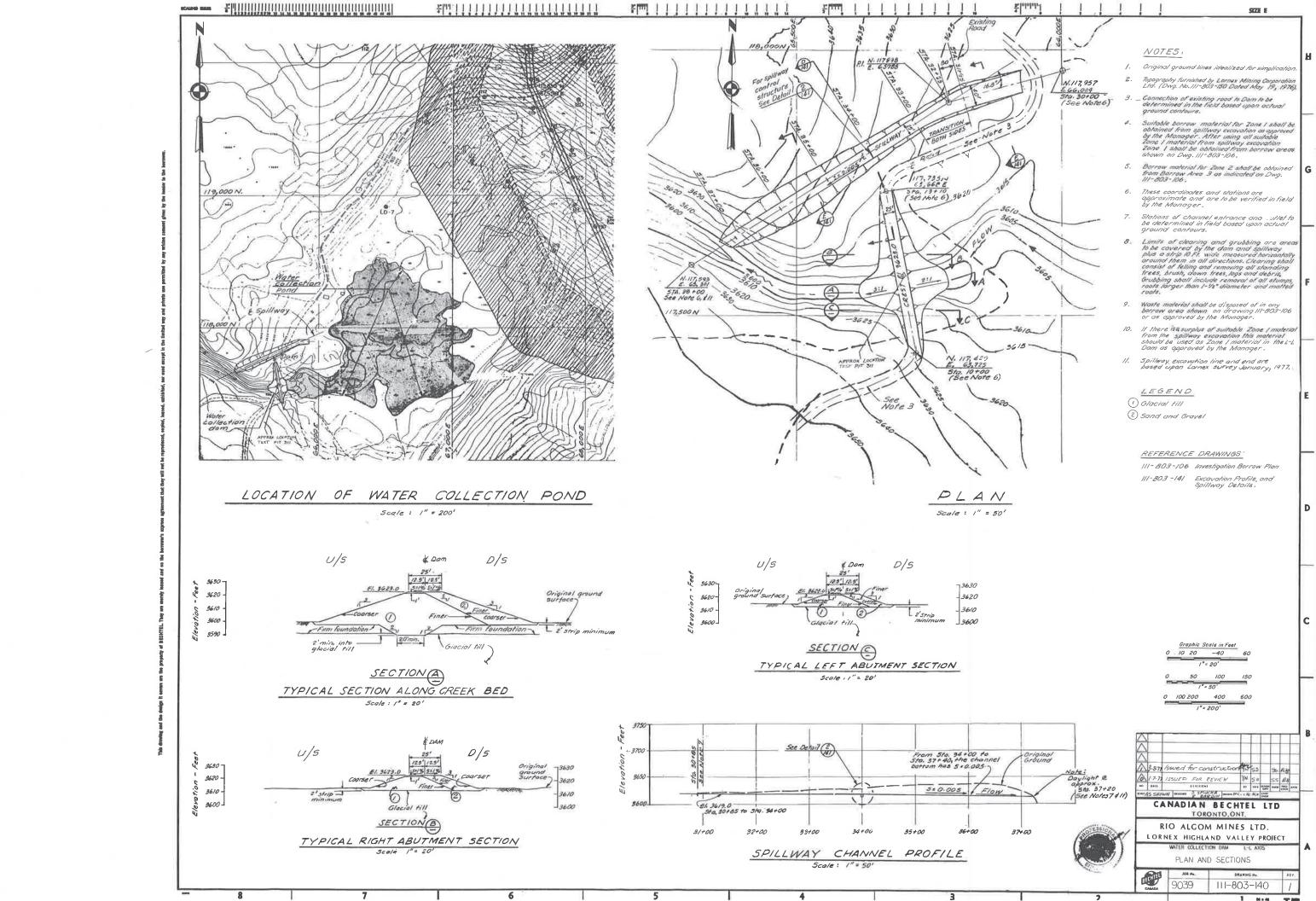
SLIMES POND 2 FILL AND PLACEMENT SPECIFICATIONS

PROJECT NO. MO2341A74 SCALE AS SHOWN D-74004

CANCEL PRINTS BEARING PREVIOUS REVISION

REVISED DESCRIPTION 2014-10-14 CYW RF AM NS RF 2014-08-28 CYW AM NS ISSUED FOR CONSTRUCTION DRAWN DESIGNED CHECKED APPROVED DESCRIPTION OF REVISION

Seepage Water Reclaim Pond



APPENDIX III-D 24 Mile Lake

Drawings Not Available

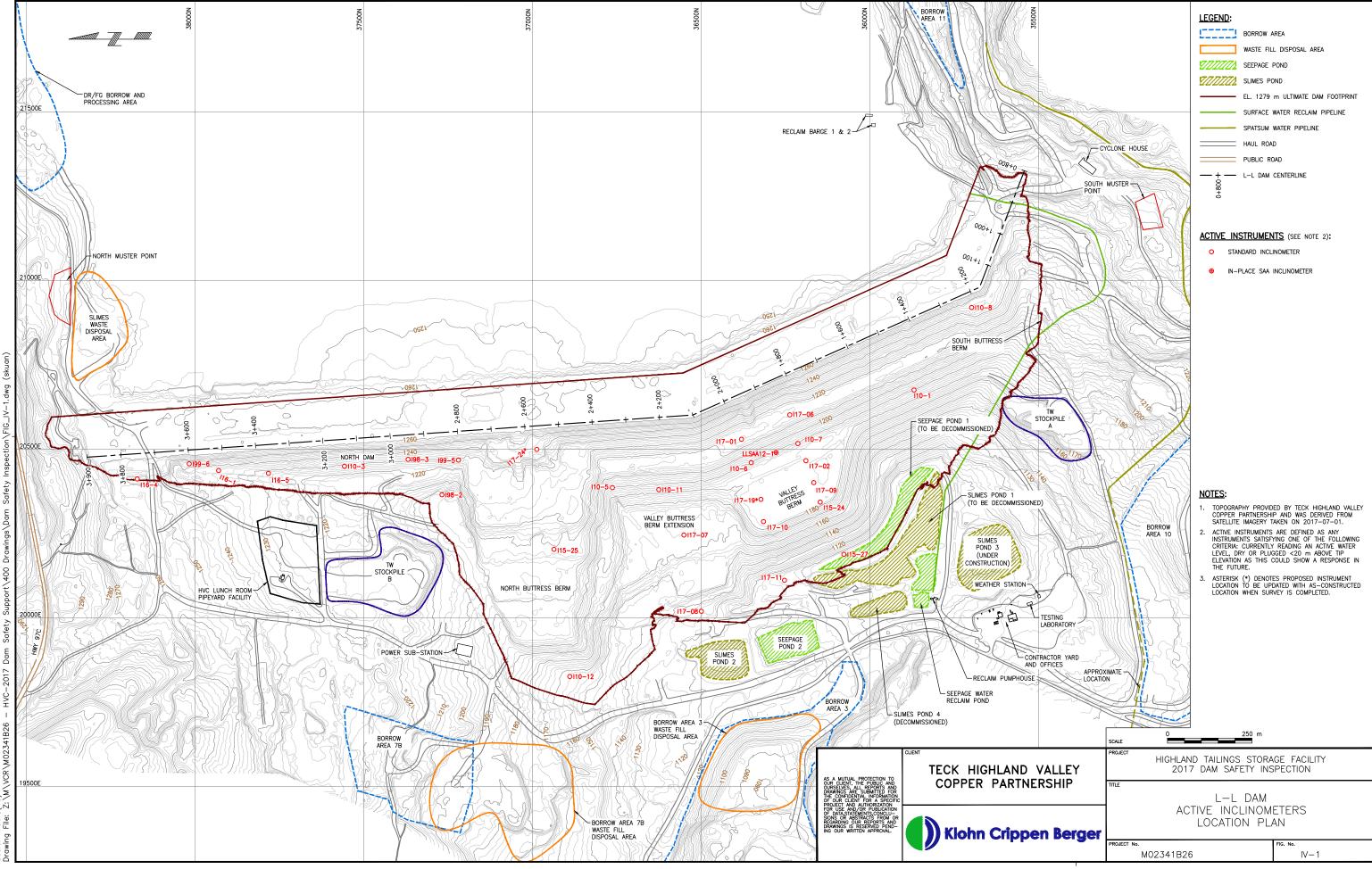
APPENDIX IV

Inclinometer Plots

Appendix IV Inclinometer Plots

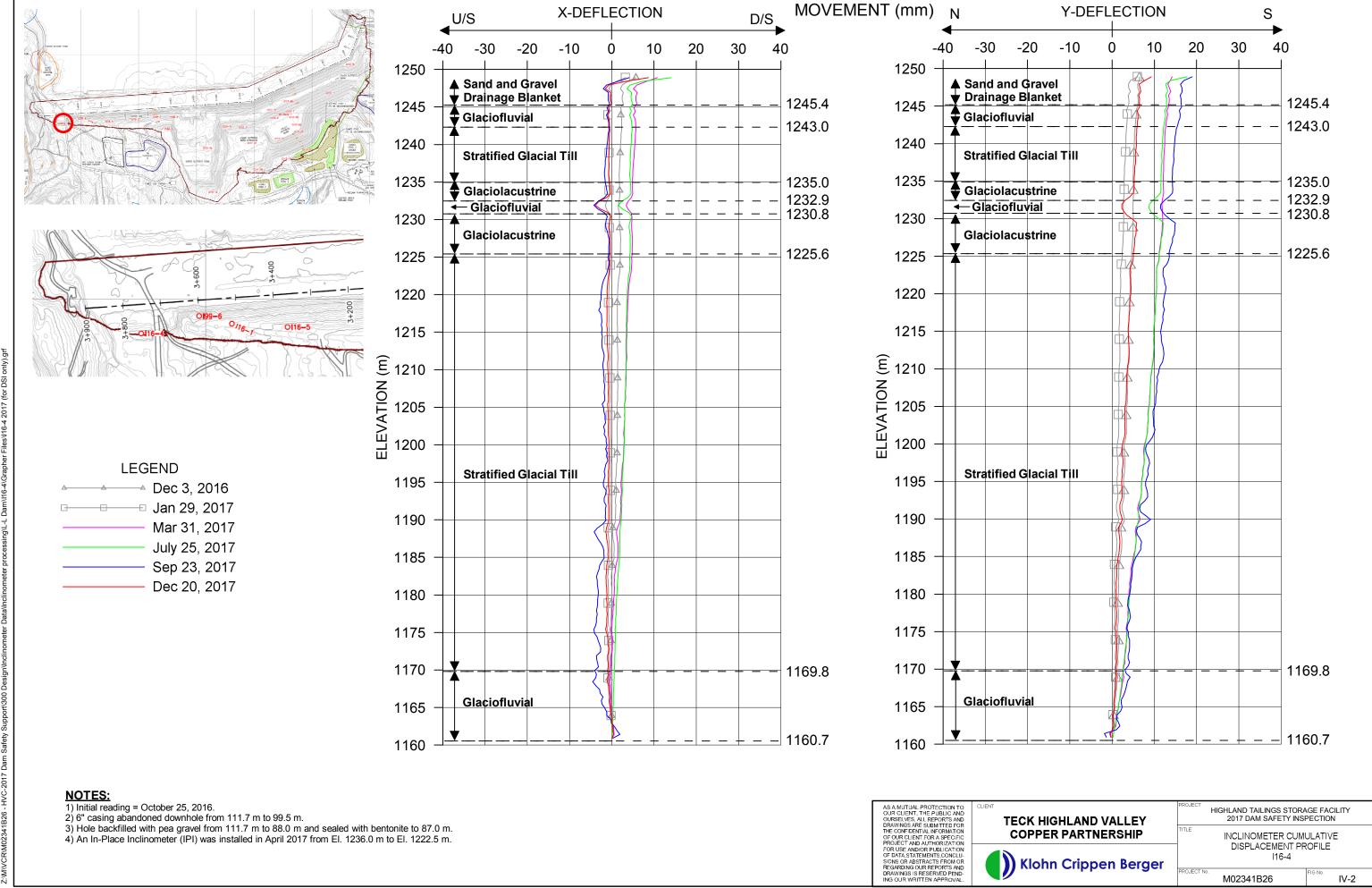
| Figure IV-1 | L-L Dam Active Inclinometers Location Plan |
|--------------|---|
| Figure IV-2 | Inclinometer Cumulative Displacement Profile – I16-4 |
| Figure IV-3 | Inclinometer Cumulative Displacement Profile – 199-6 |
| Figure IV-4 | Inclinometer Cumulative Displacement Profile – I16-1 |
| Figure IV-5 | Inclinometer Cumulative Displacement Profile – I16-5 |
| Figure IV-6 | Inclinometer Cumulative Displacement Profile – I10-3 |
| Figure IV-7 | Inclinometer Cumulative Displacement Profile – 198-3 |
| Figure IV-8 | Inclinometer Cumulative Displacement Profile – 198-2 |
| Figure IV-9 | Inclinometer Cumulative Displacement Profile – 199-5 |
| Figure IV-10 | Inclinometer Cumulative Displacement Profile – I15-25 |
| Figure IV-11 | Inclinometer Cumulative Displacement Profile – I10-12 |
| Figure IV-12 | Inclinometer Cumulative Displacement Profile – I10-5 |
| Figure IV-13 | Inclinometer Cumulative Displacement Profile – I10-11 |
| Figure IV-14 | Inclinometer Cumulative Displacement Profile – I17-7 |
| Figure IV-15 | Inclinometer Cumulative Displacement Profile – I17-8 |
| Figure IV-16 | Inclinometer Cumulative Displacement Profile – I17-1 |
| Figure IV-17 | Inclinometer Cumulative Displacement Profile – I10-6 |
| Figure IV-18 | Inclinometer Cumulative Displacement Profile – I17-10 |
| Figure IV-19 | Inclinometer Cumulative Displacement Profile – I17-11 |
| Figure IV-20 | Inclinometer Cumulative Displacement Profile – LLSAA12-1 |
| Figure IV-21 | Inclinometer Cumulative Displacement Profile – I17-06 |
| Figure IV-22 | Inclinometer Cumulative Displacement Profile – I10-7 |
| Figure IV-23 | Inclinometer Cumulative Displacement Profile – I17-02 |
| Figure IV-24 | Inclinometer Cumulative Displacement Profile – I17-09 |
| Figure IV-25 | Inclinometer Cumulative Displacement Profile – I15-24 |
| Figure IV-26 | Inclinometer Cumulative Displacement Profile – I15-27 |
| Figure IV-27 | Inclinometer Cumulative Displacement Profile – I10-1 |
| Figure IV-28 | Inclinometer Cumulative Displacement Profile – I10-8 |
| Figure IV-29 | L-L Dam Horizontal Displacements - Top of Lacustrine Unit 2014-2017 |
| Figure IV-30 | L-L Dam Horizontal Displacements Across Lacustrine Unit 2014-2017 |
| Figure IV-31 | H-H Dam Active Inclinometers Location Plan |
| Figure IV-32 | Inclinometer Cumulative Displacement Profile – HHSAA12-1 |
| Figure IV-33 | Inclinometer Cumulative Displacement Profile – HHI15-23 |

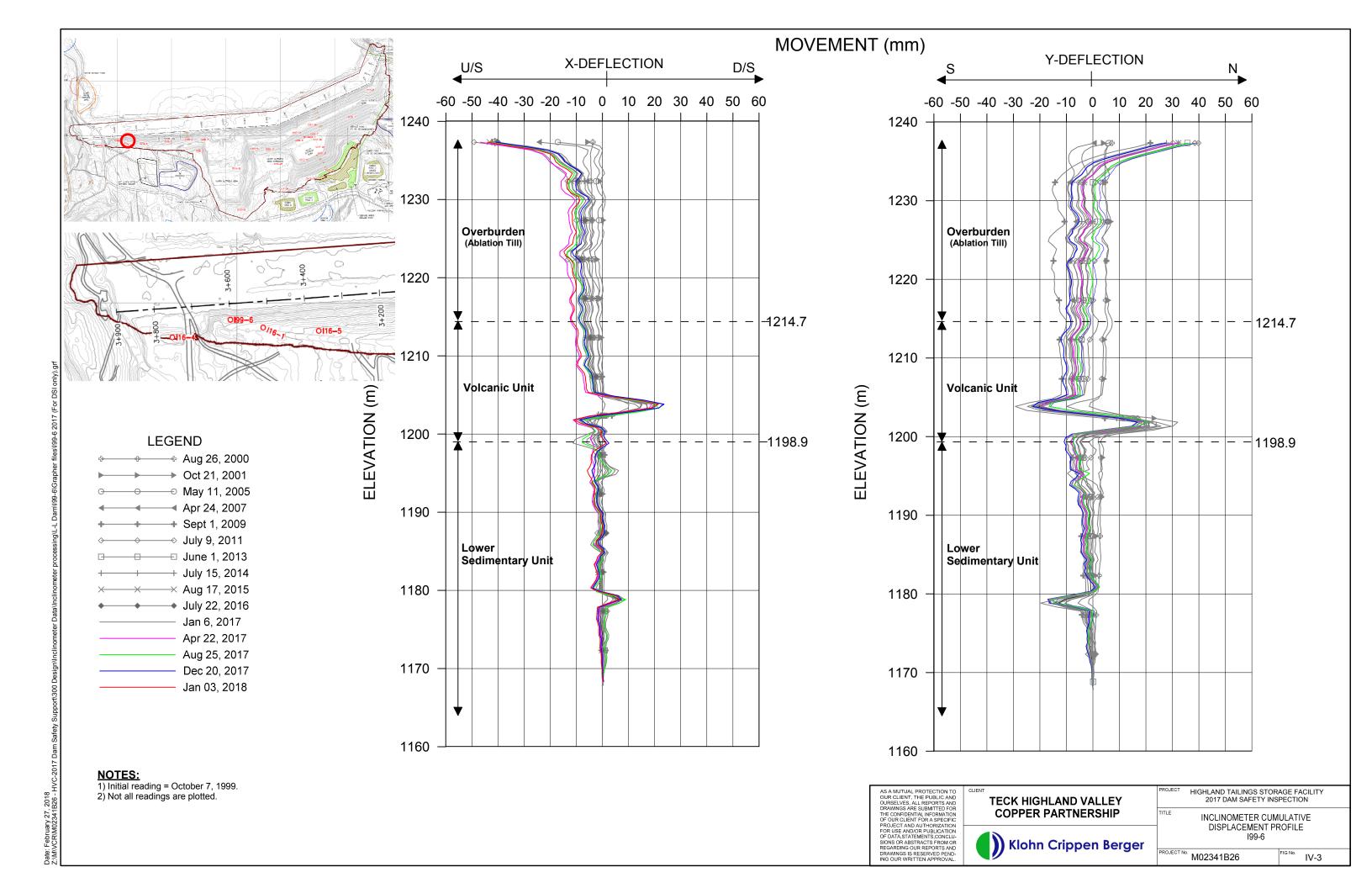
| Figure IV-34 | Inclinometer Cumulative Displacement Profile – HHI17-17 |
|--------------|---|
| Figure IV-35 | Inclinometer Cumulative Displacement Profile – HHI17-16 |
| Figure IV-36 | Inclinometer Cumulative Displacement Profile –HHI17-14 |
| Figure IV-37 | H-H Dam Horizontal Displacements – Dam Fill 2016-2018 |
| Figure IV-38 | H-H Dam Vertical Displacements – HHI17-14A (Sondex) |
| Figure IV-39 | H-H Dam Vertical Displacements – HHI17-16A (Sondex) |
| Figure IV-40 | H-H Dam Vertical Displacements – HHI17-17A (Sondex) |

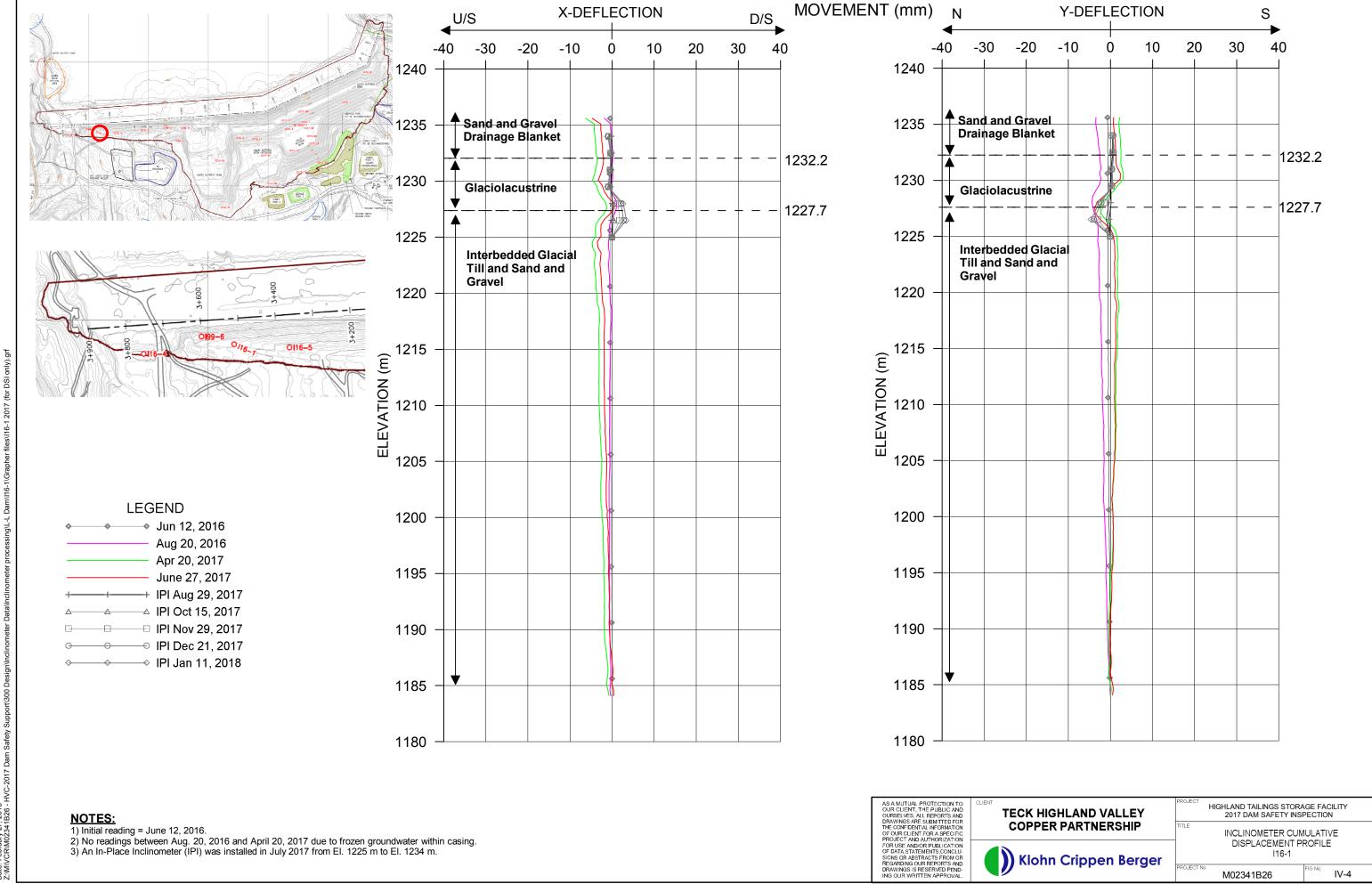


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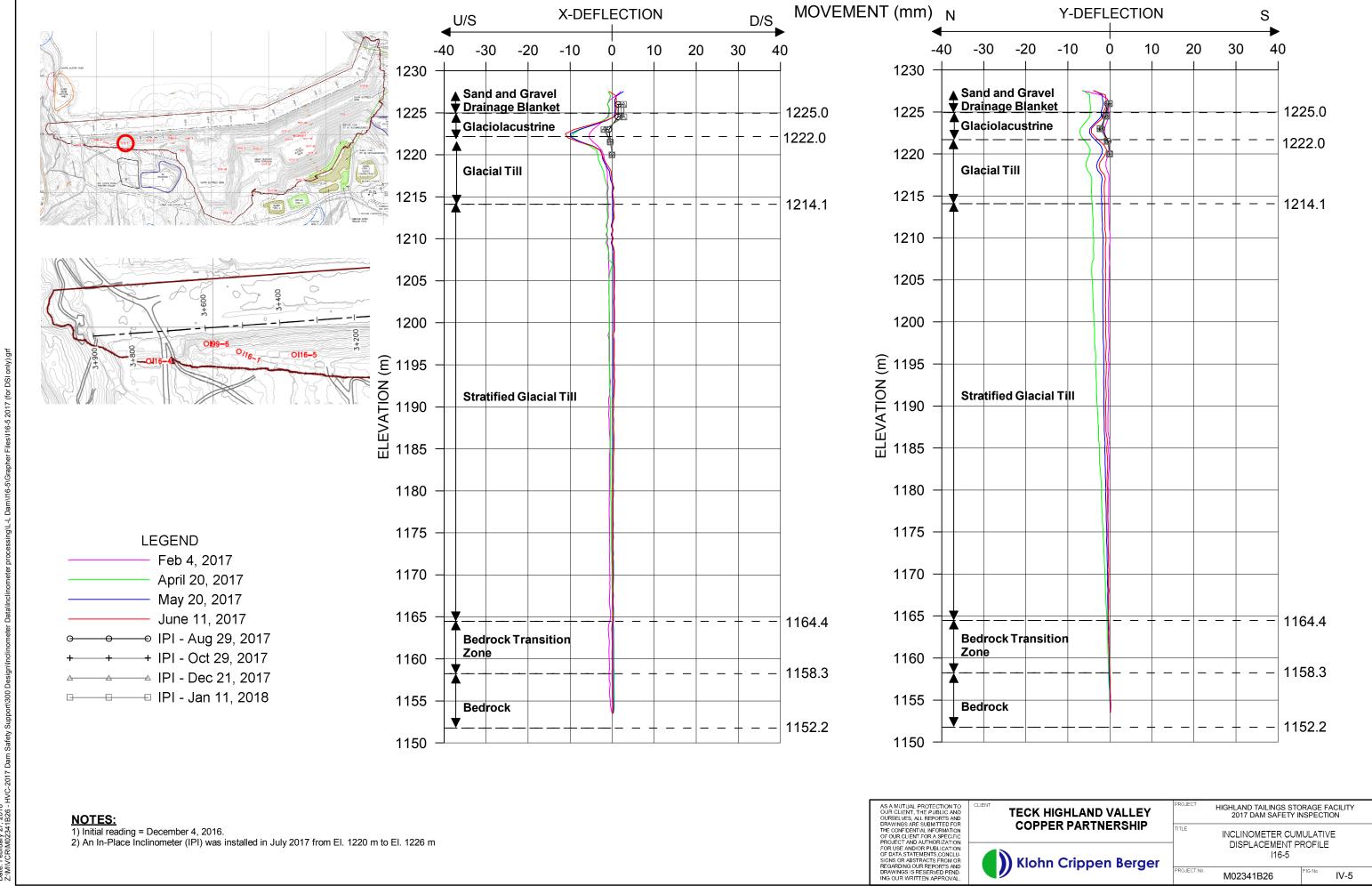




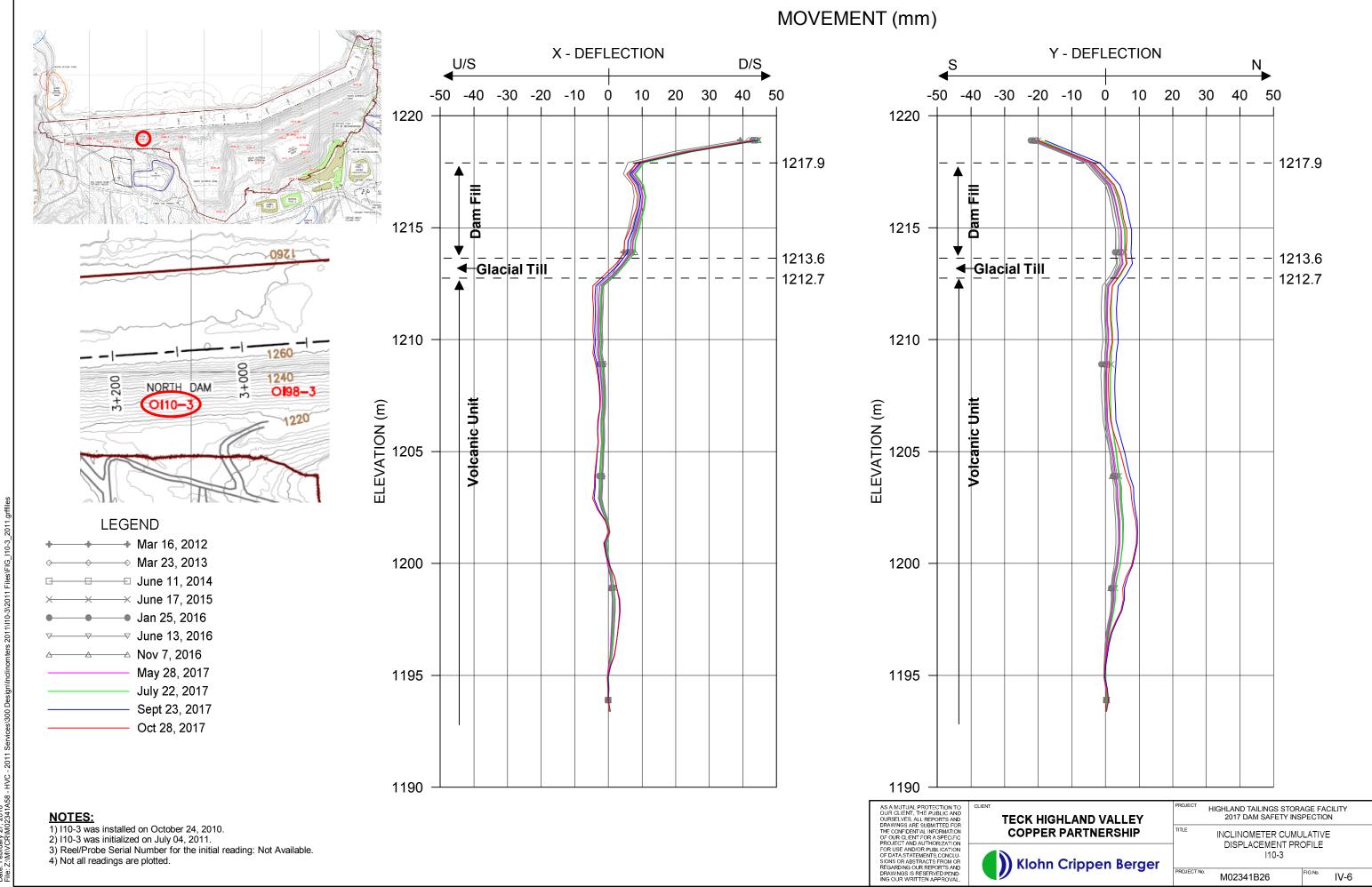


IV-4

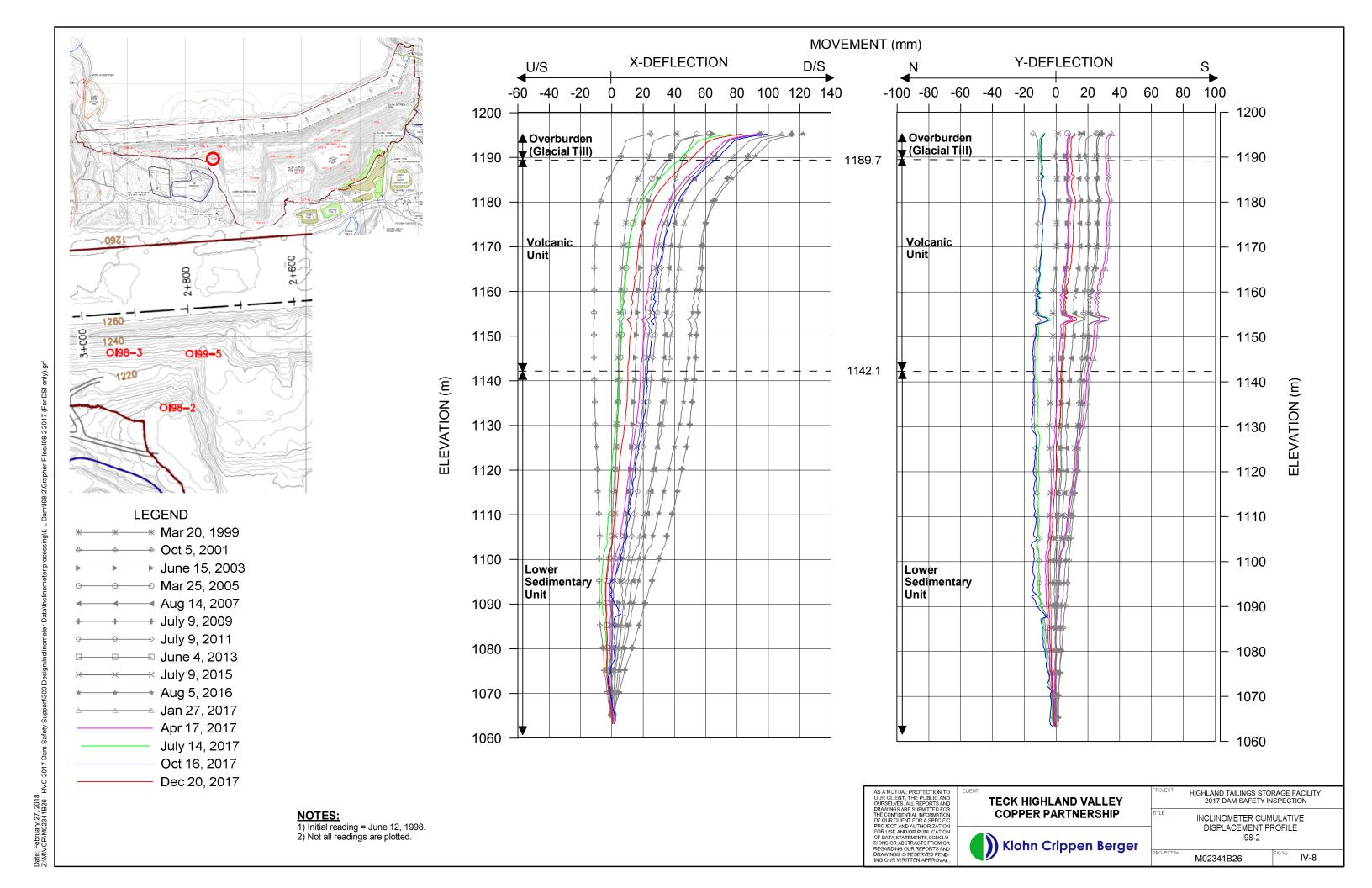
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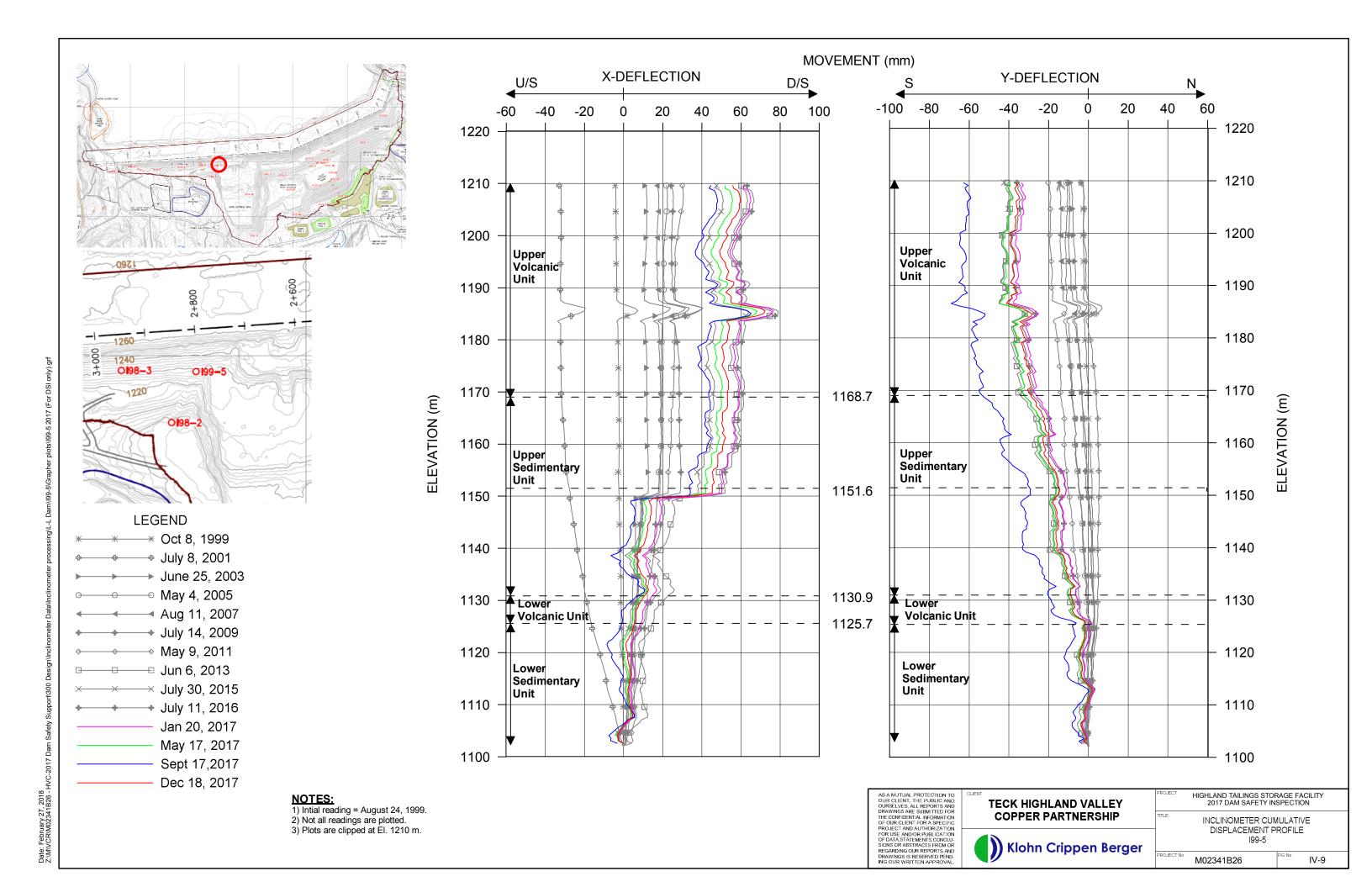


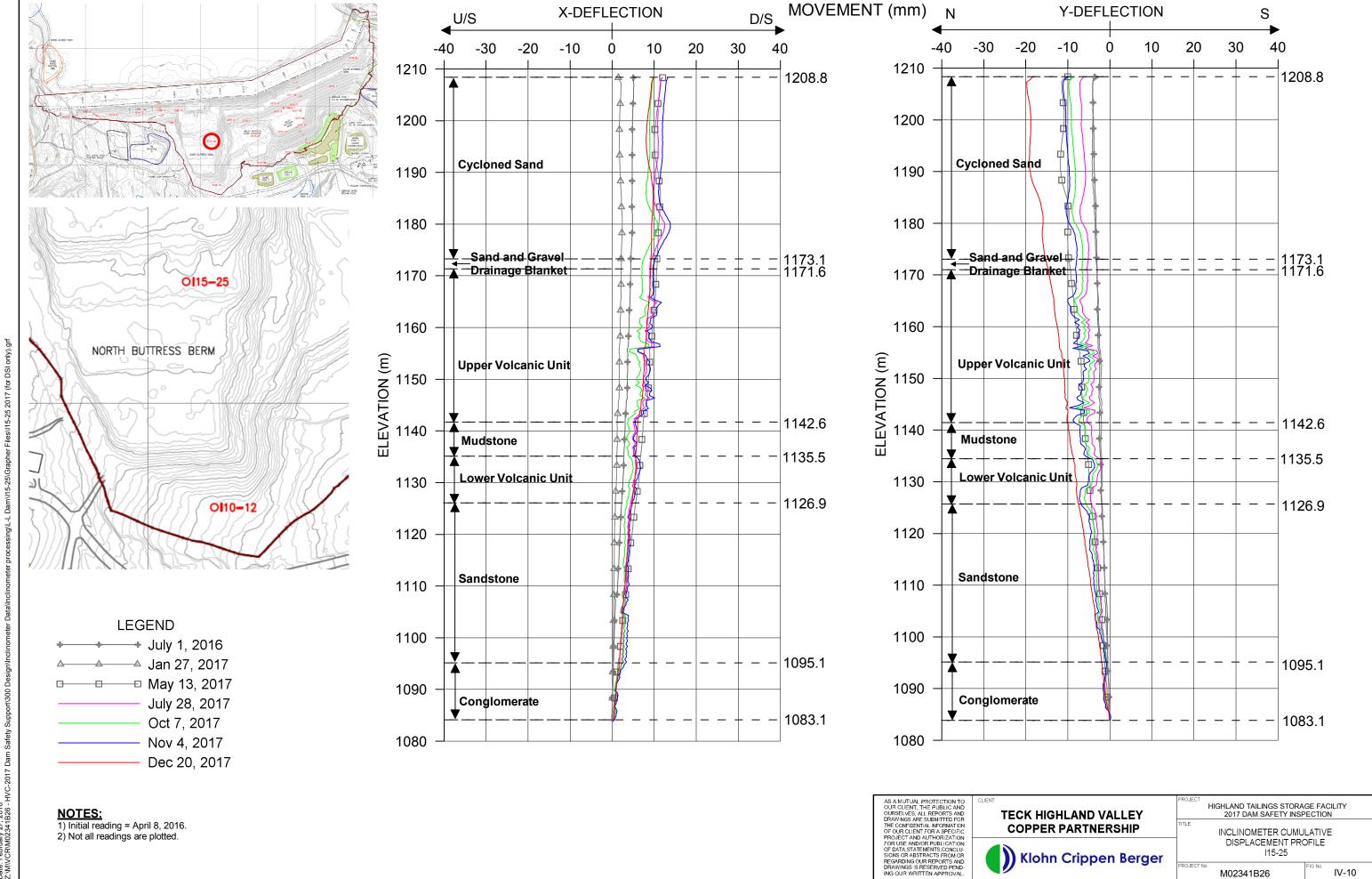
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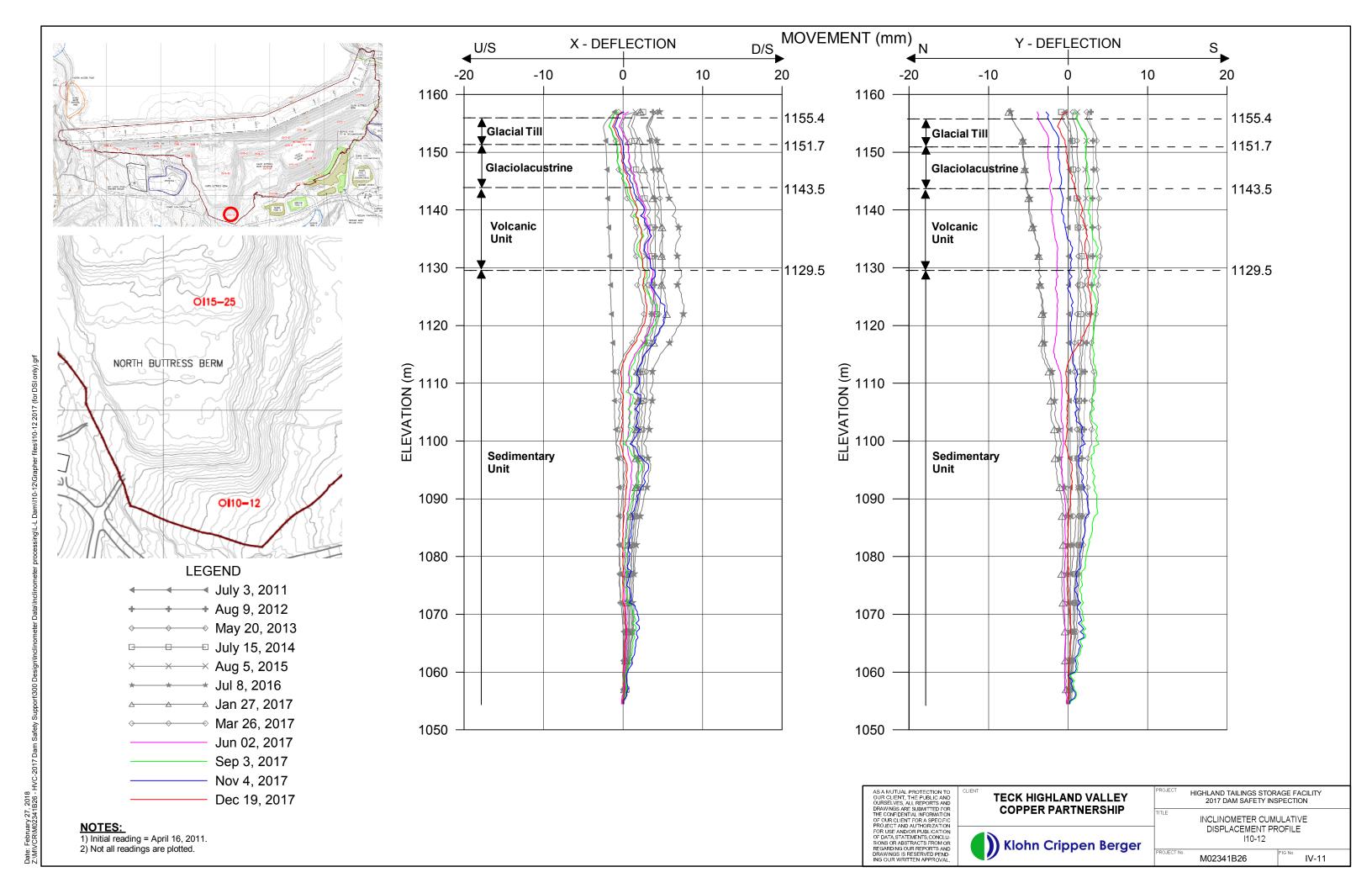
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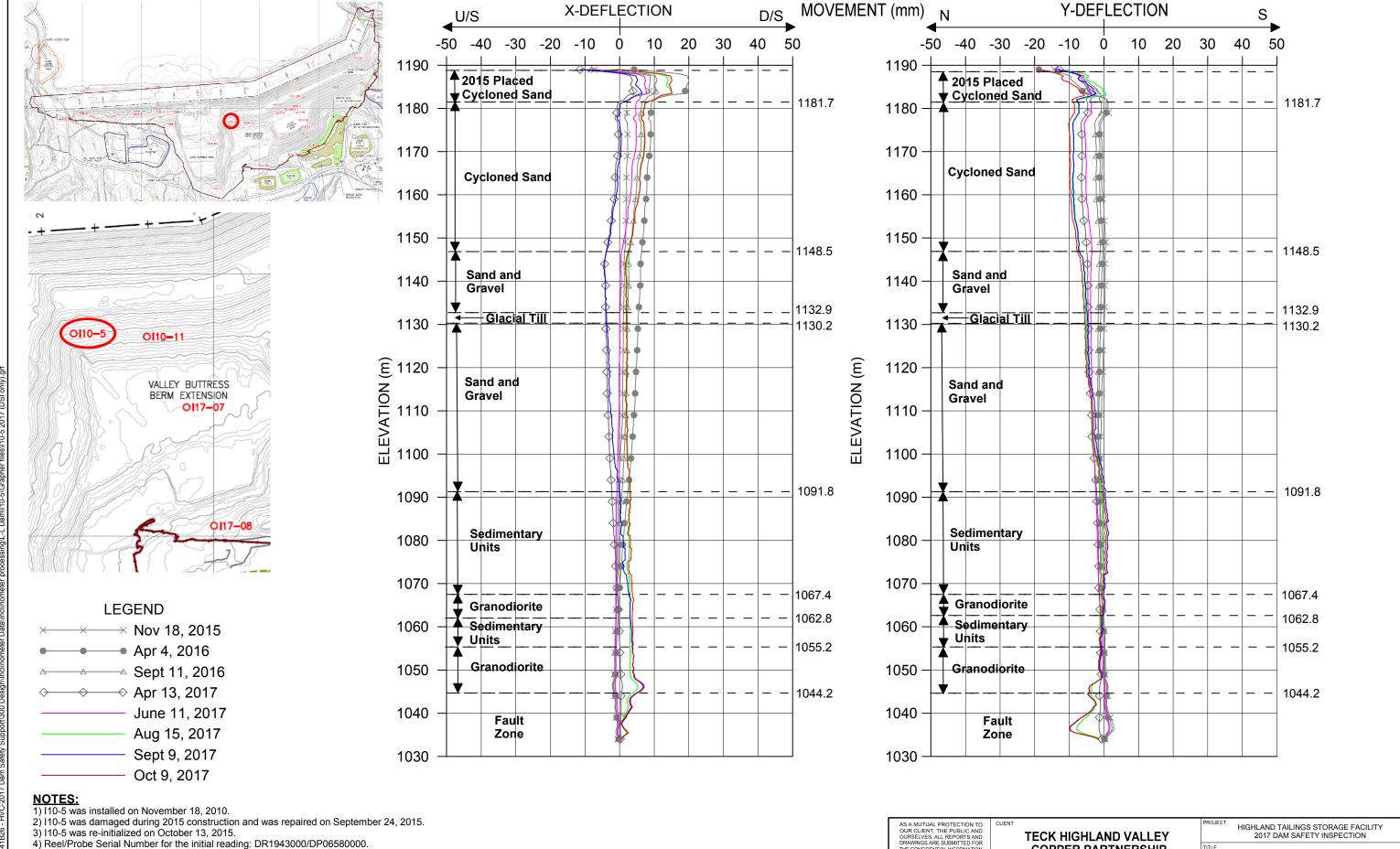






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6) 2017 readings for August, September and October are inconsistent with historical trends, and are under review by KCB/THVCP (likely due to surveillance error). Readings are shown on this figure as the deviation from historical trend are minor and do not indicate an immediate dam safety issue.

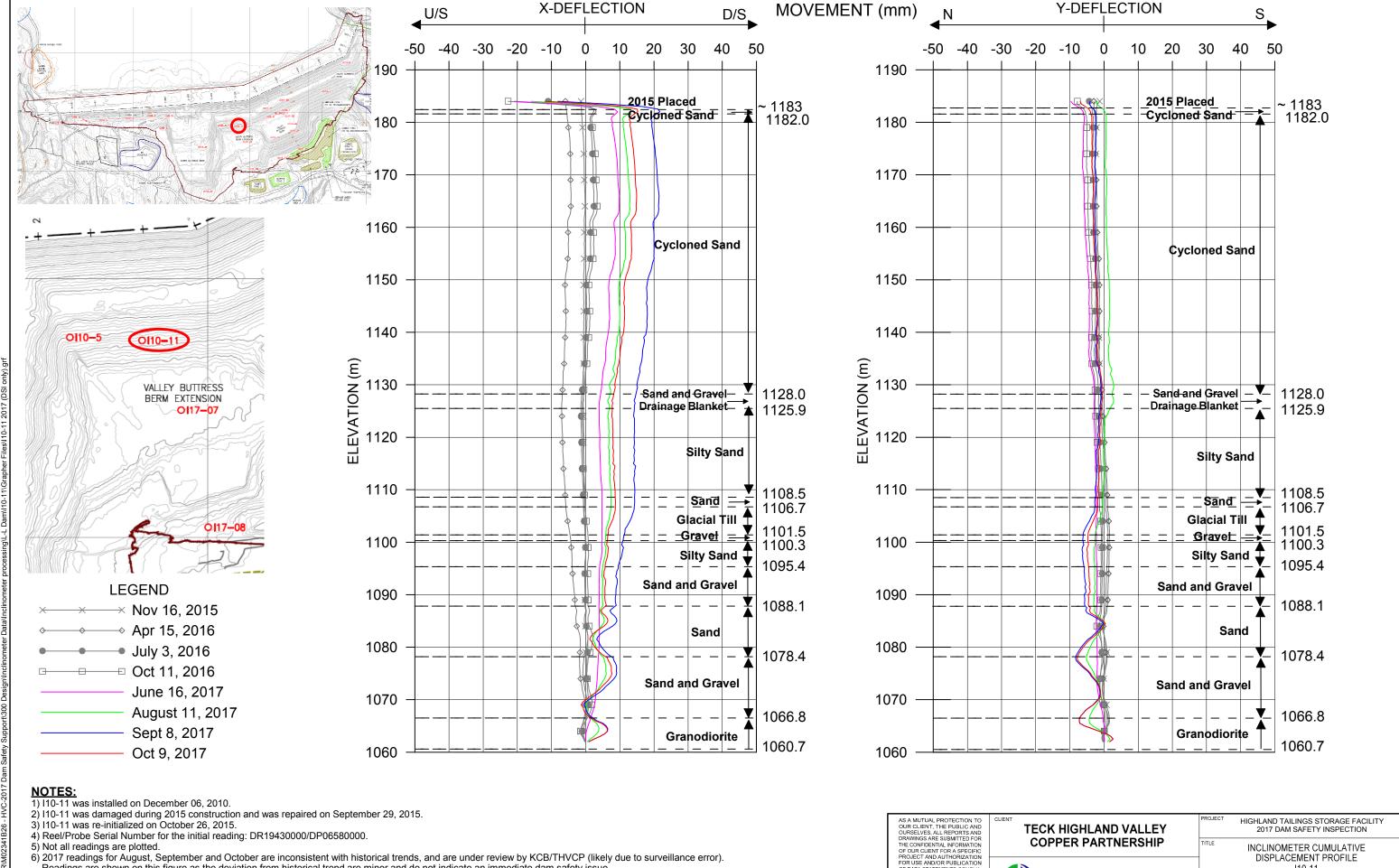
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INCLINOMETER CUMULATIVE DISPLACEMENT PROFILE I10-5

Klohn Crippen Berger

M02341B26 IV-12



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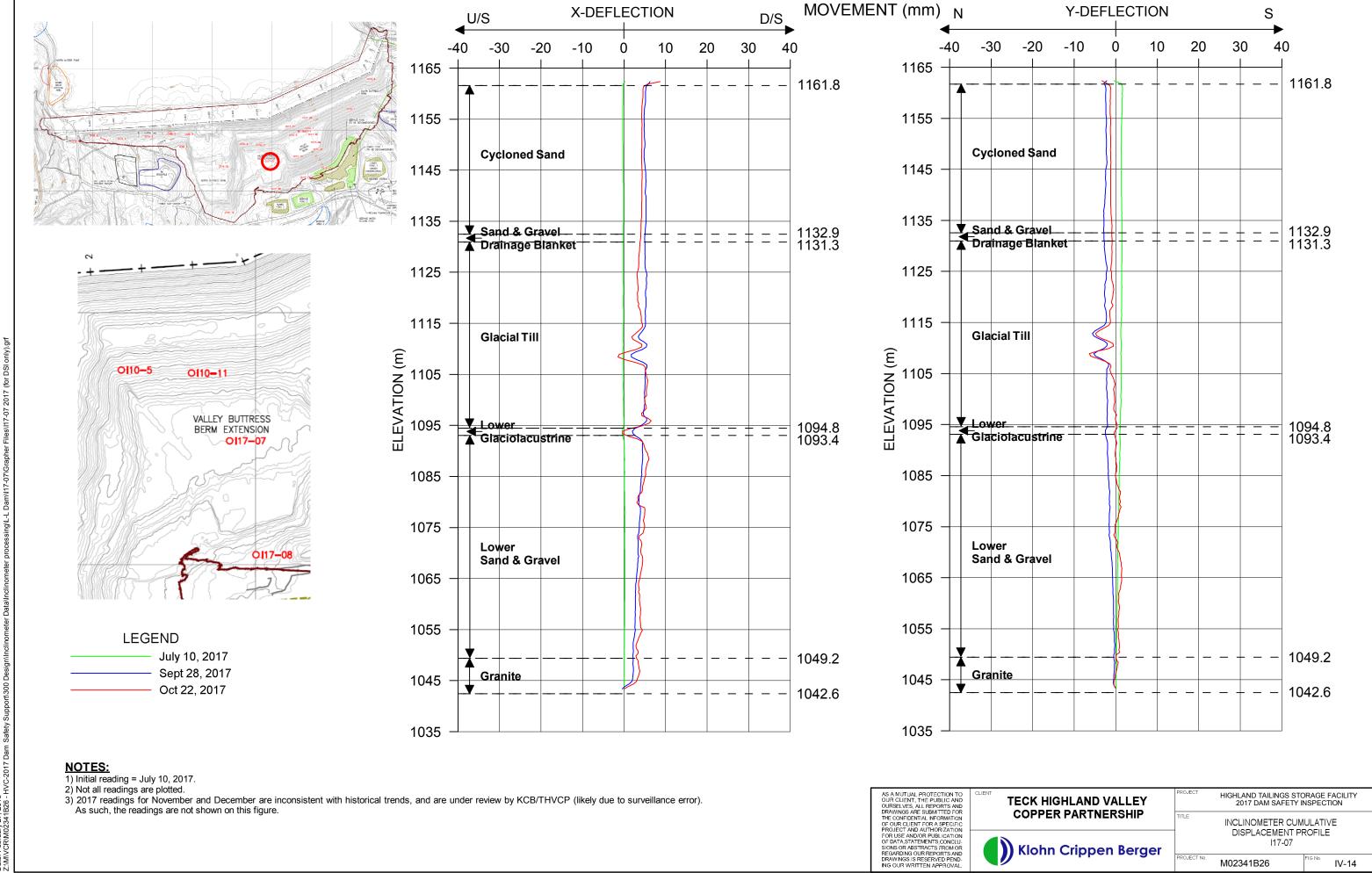
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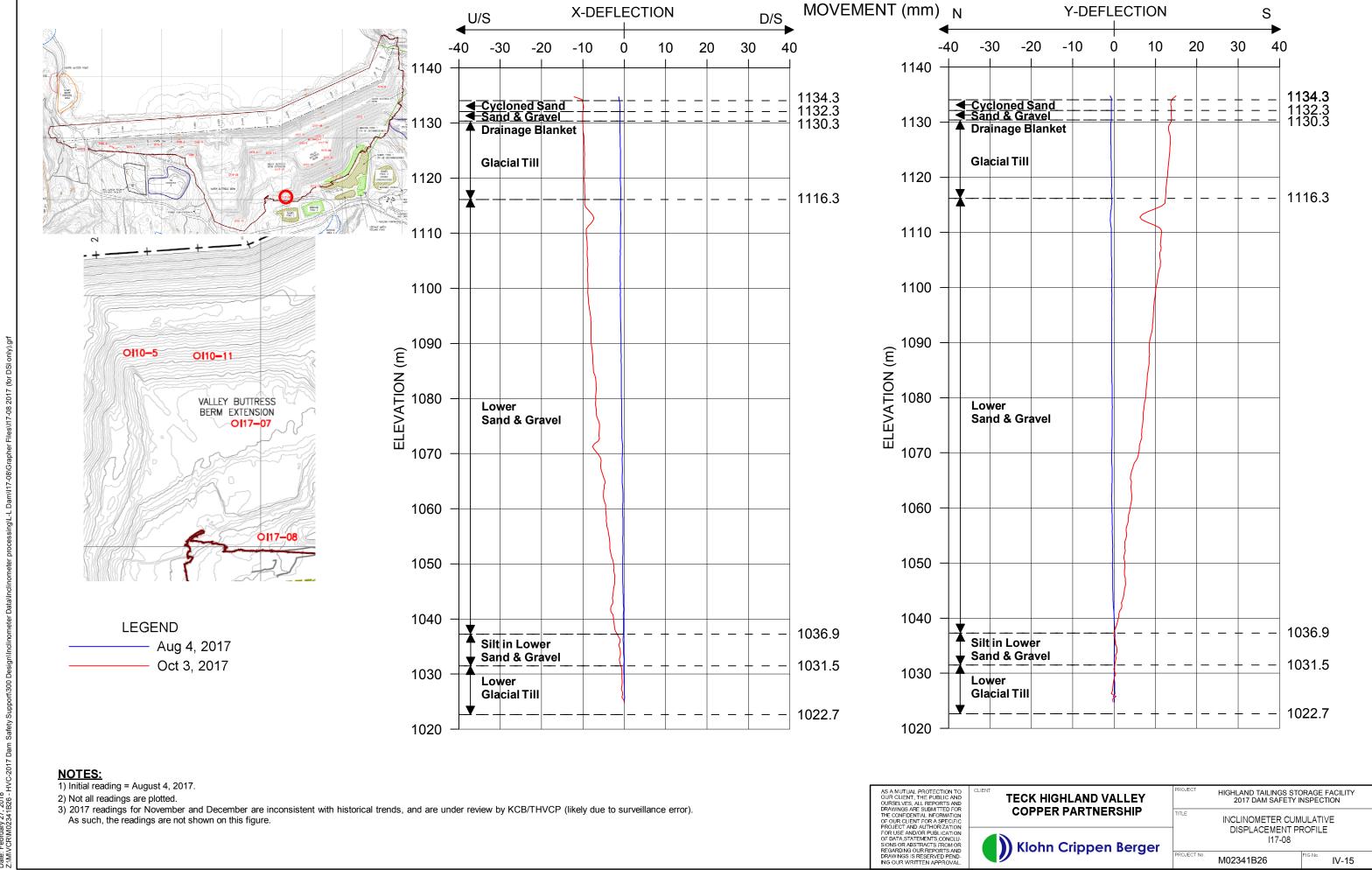
110-11

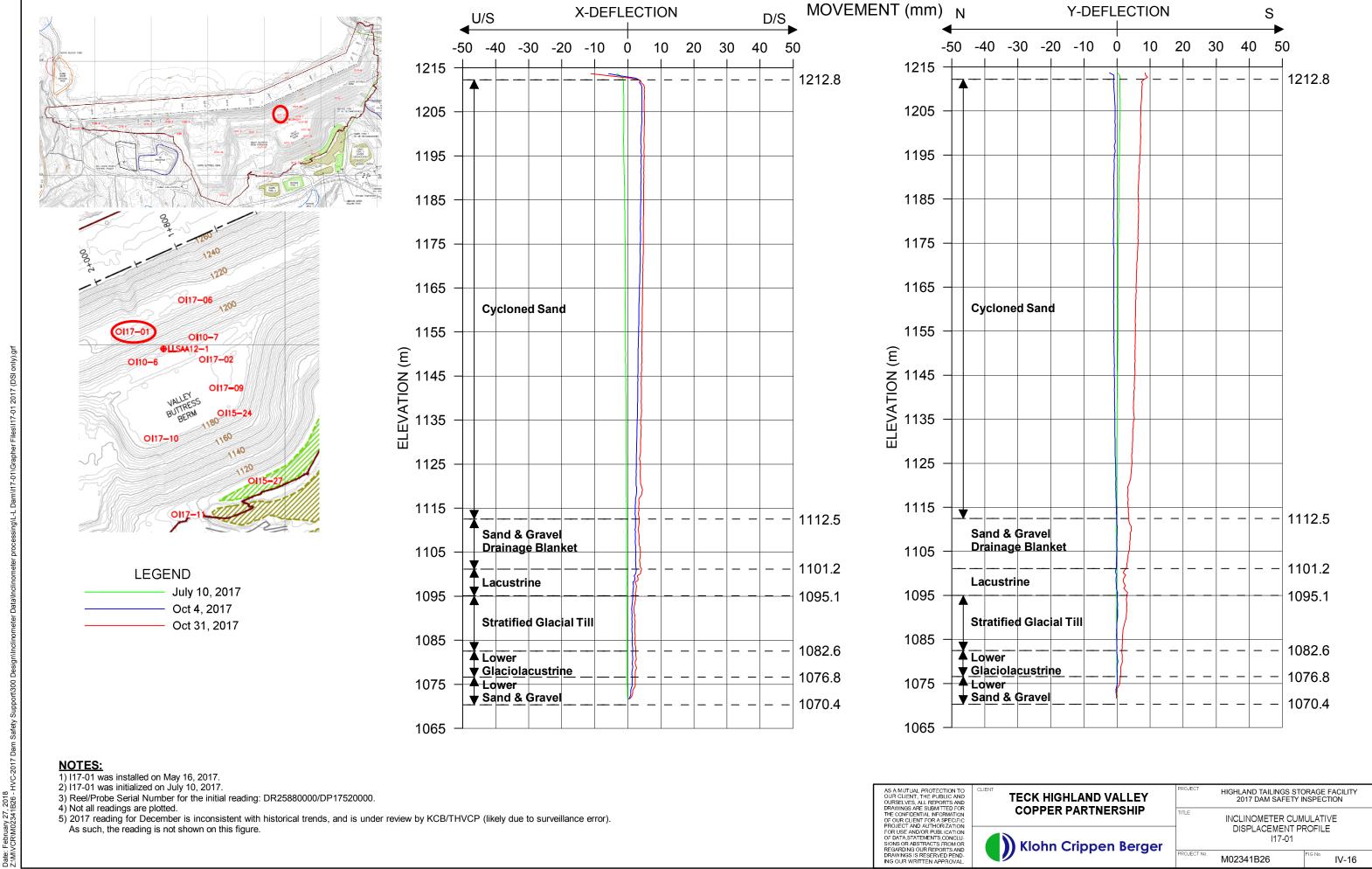
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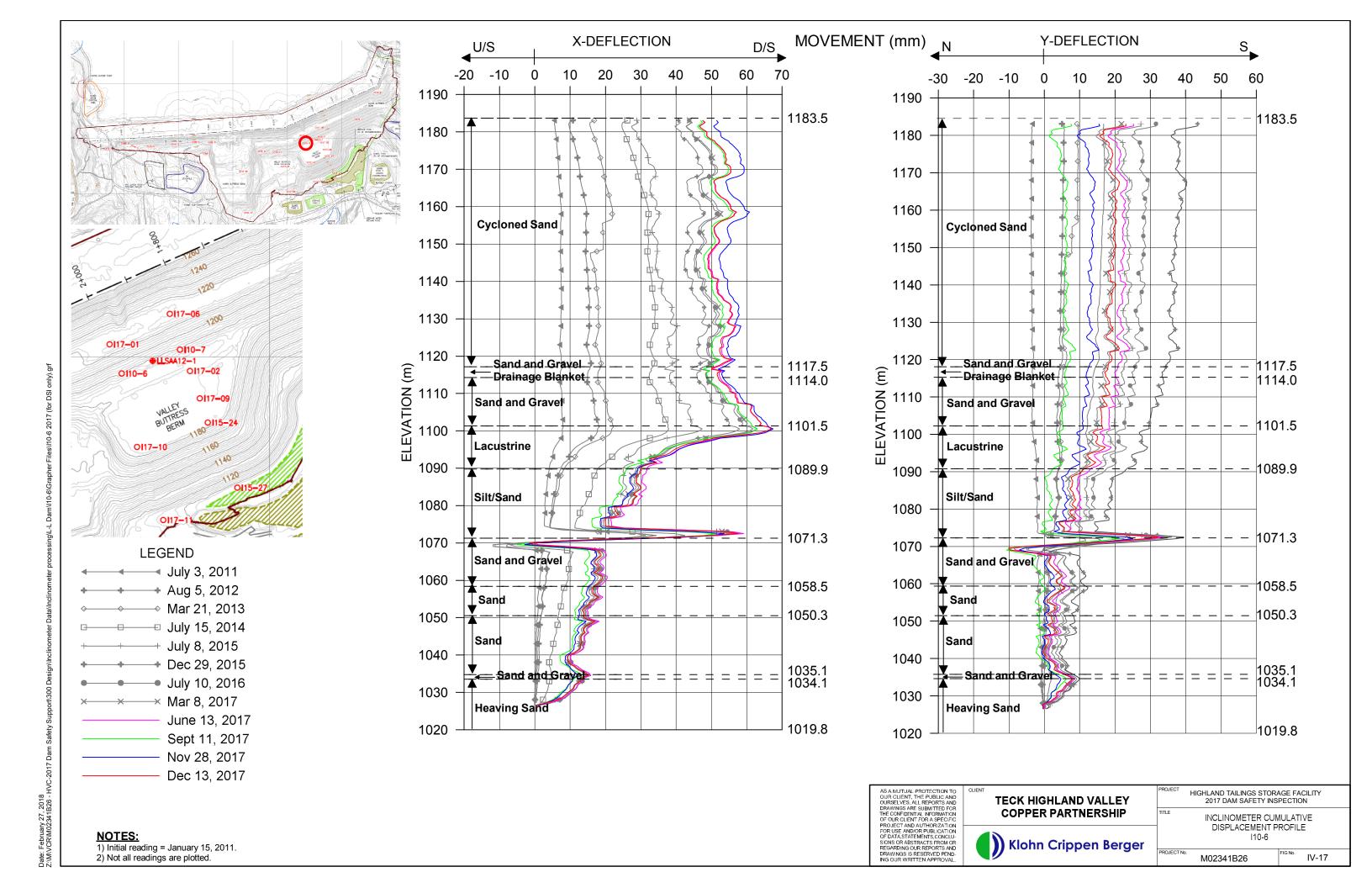
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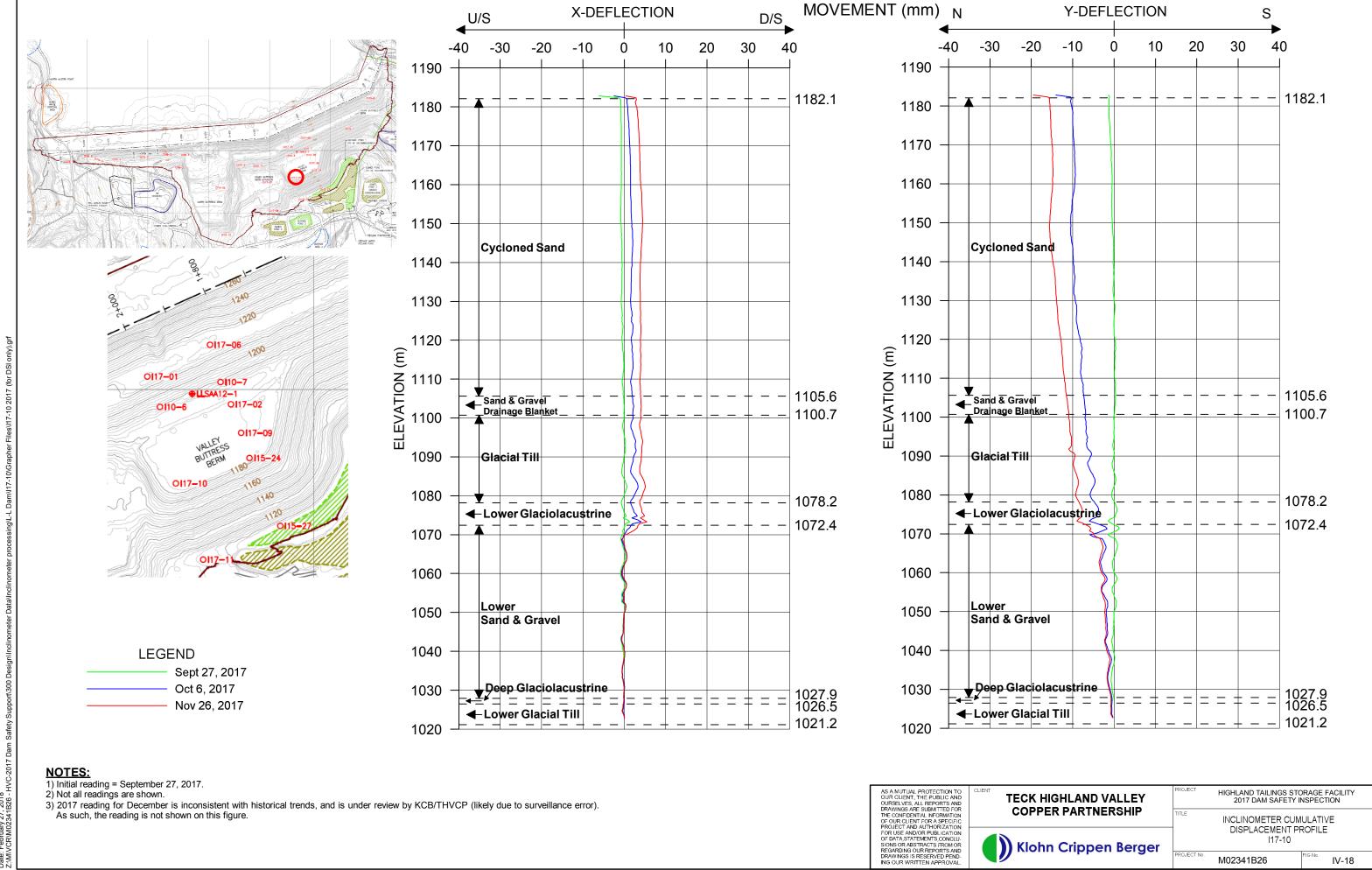
Readings are shown on this figure as the deviation from historical trend are minor and do not indicate an immediate dam safety issue.





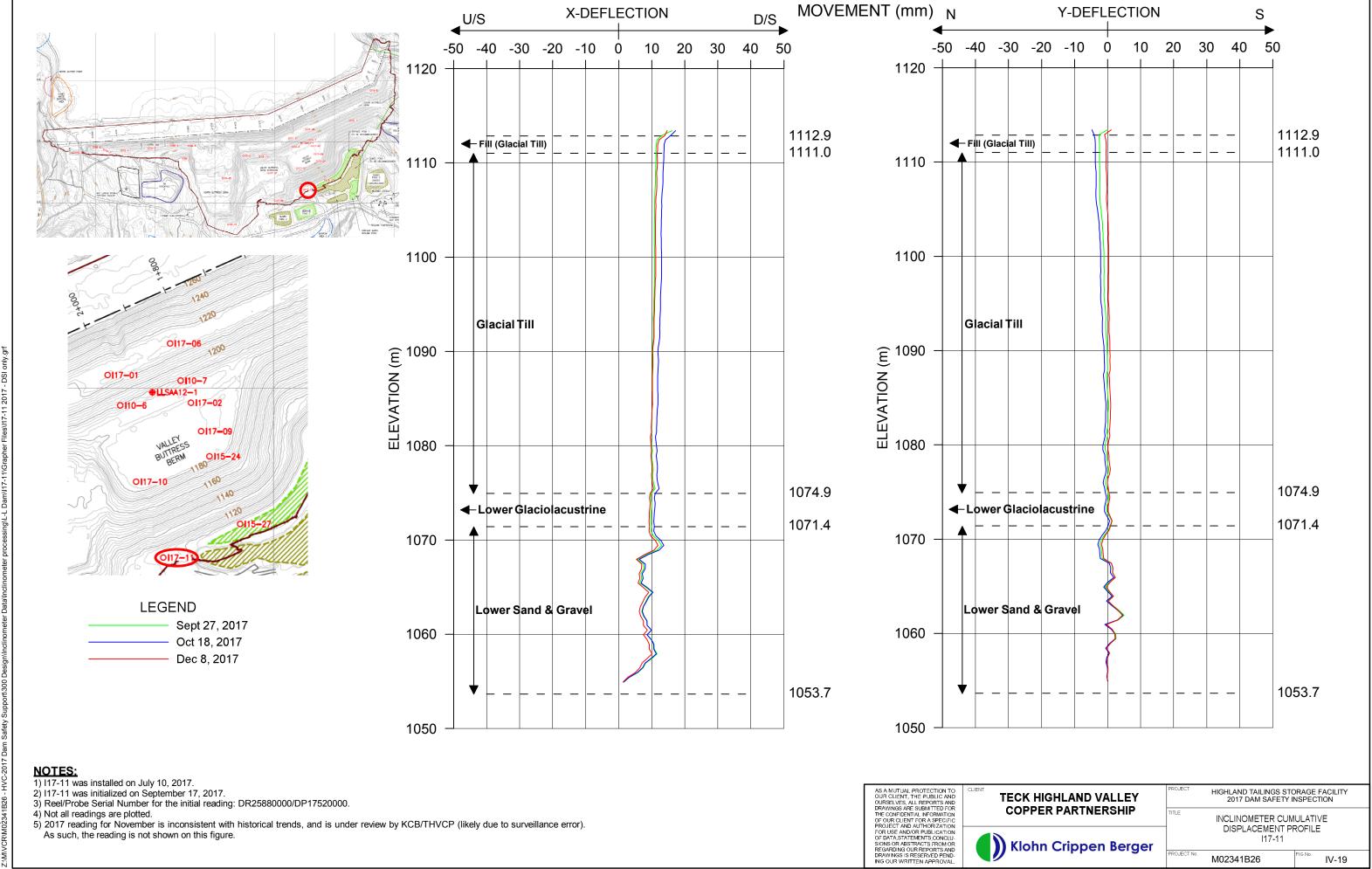


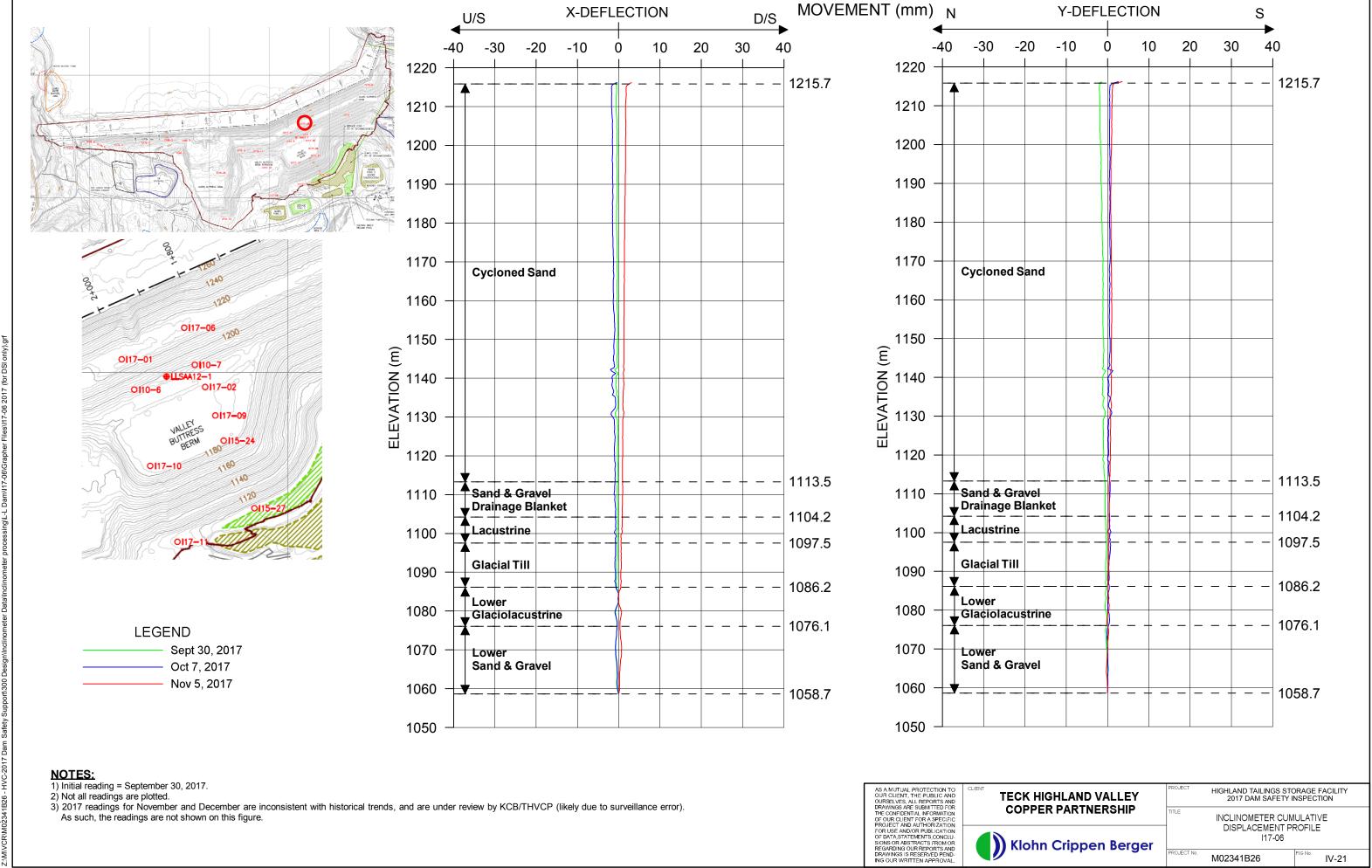


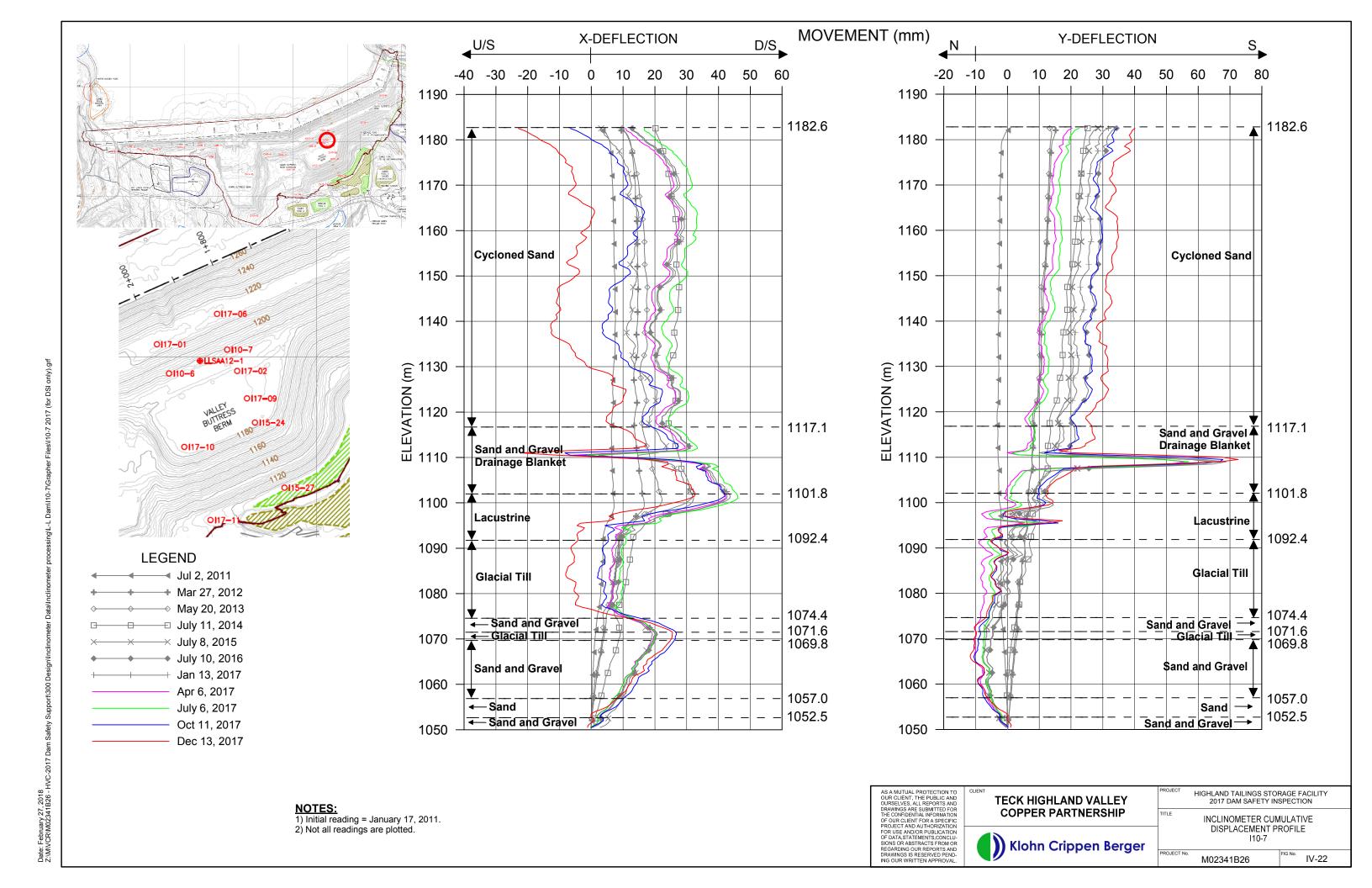


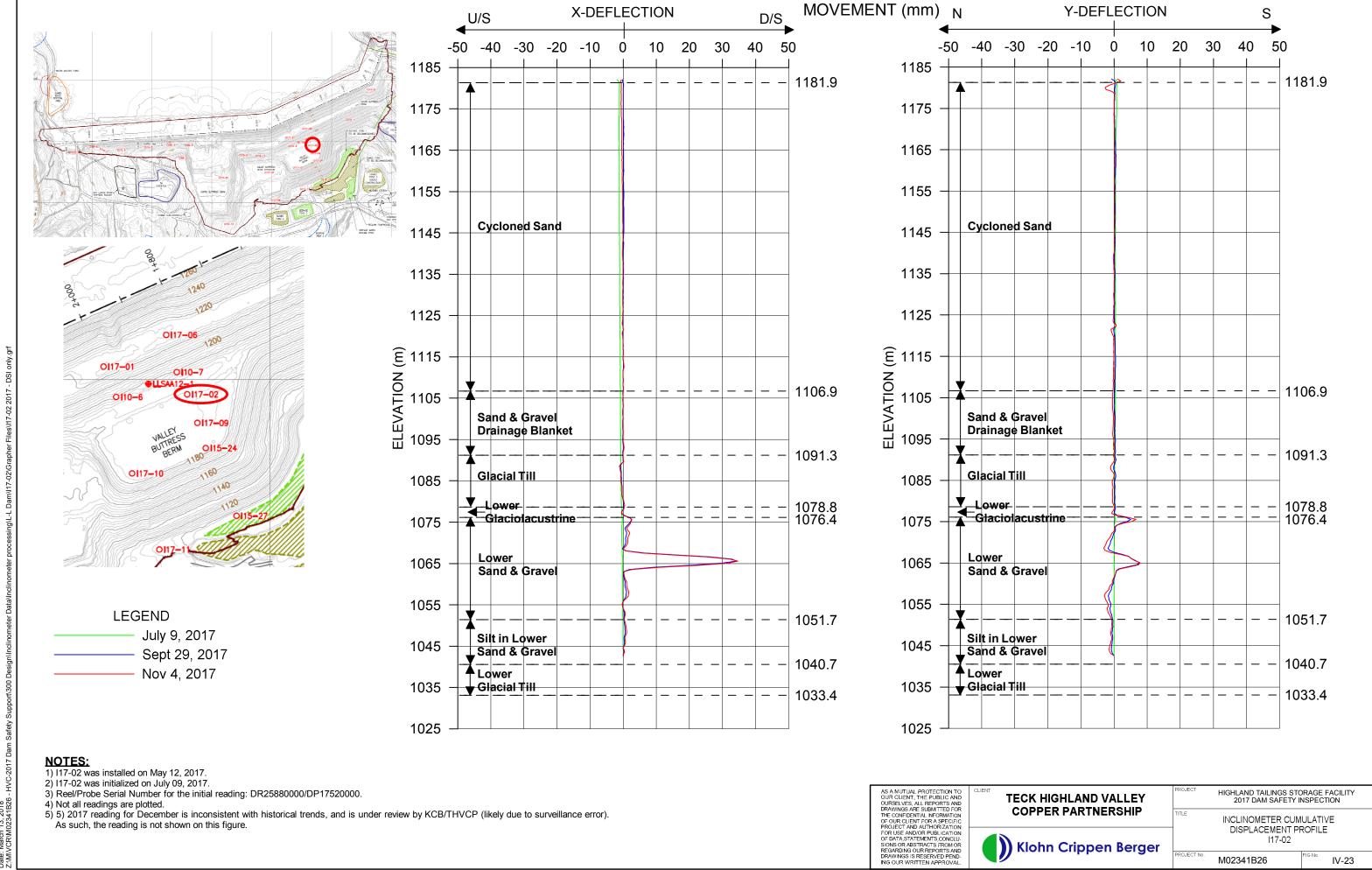
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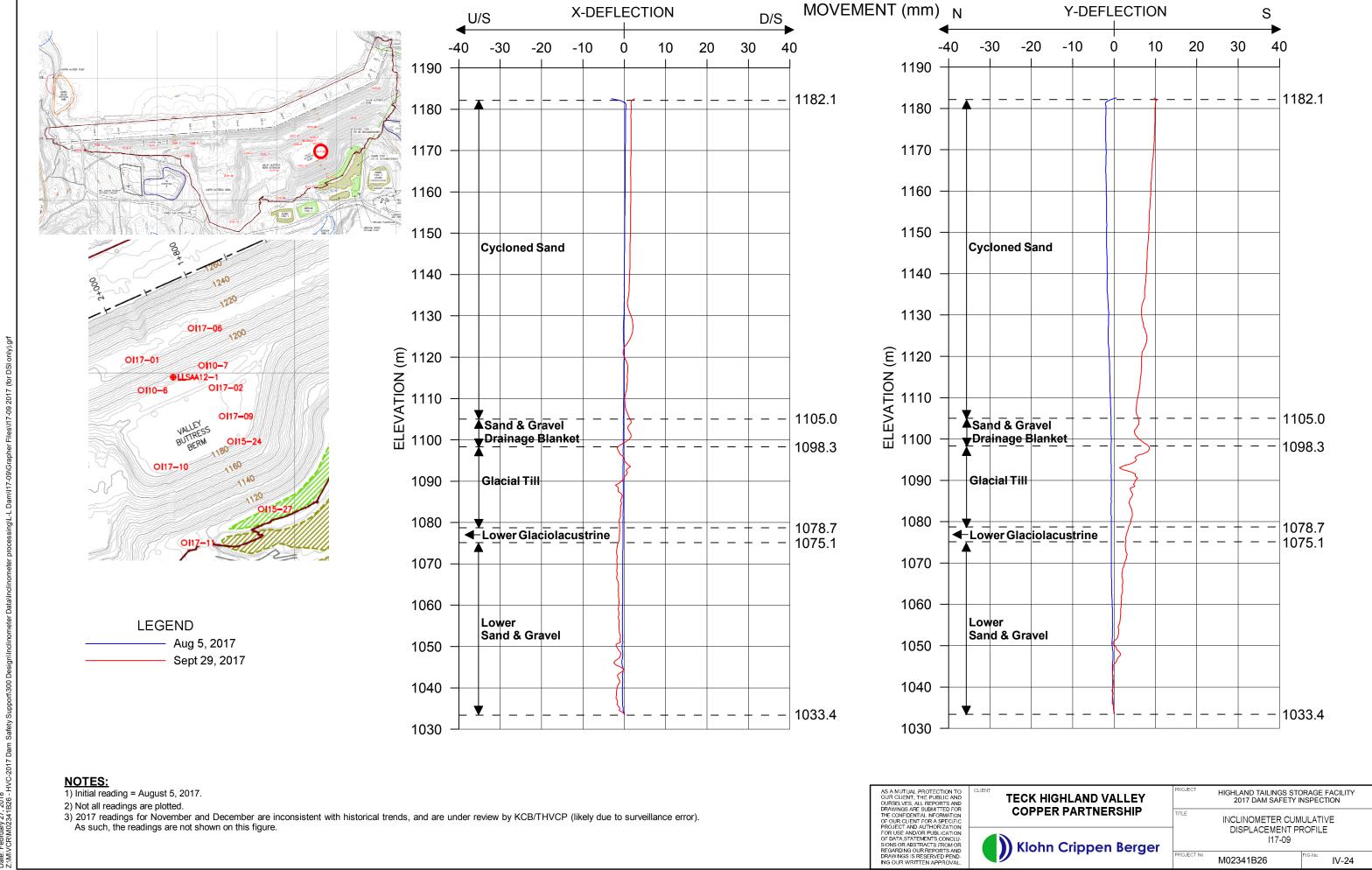
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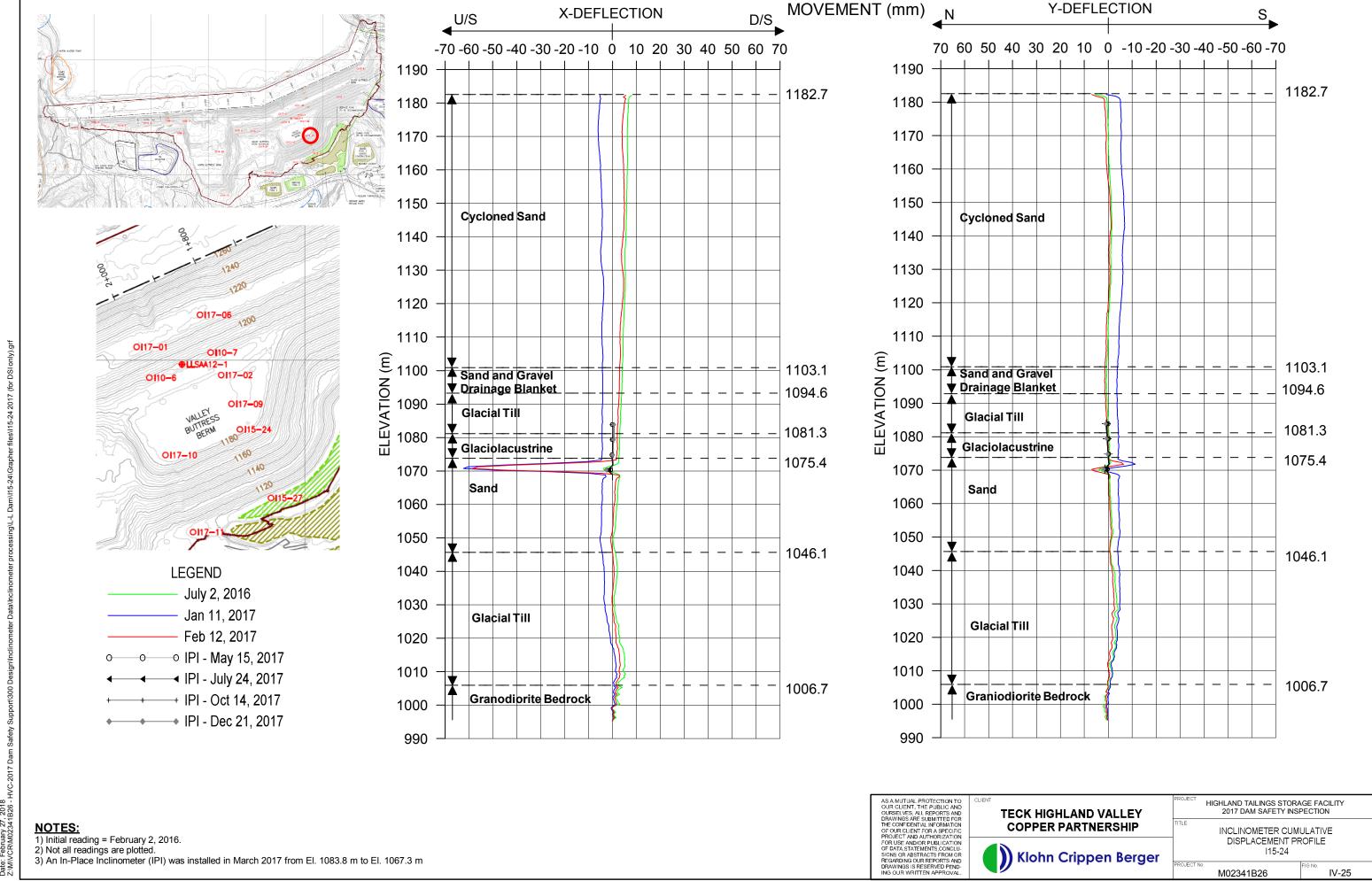


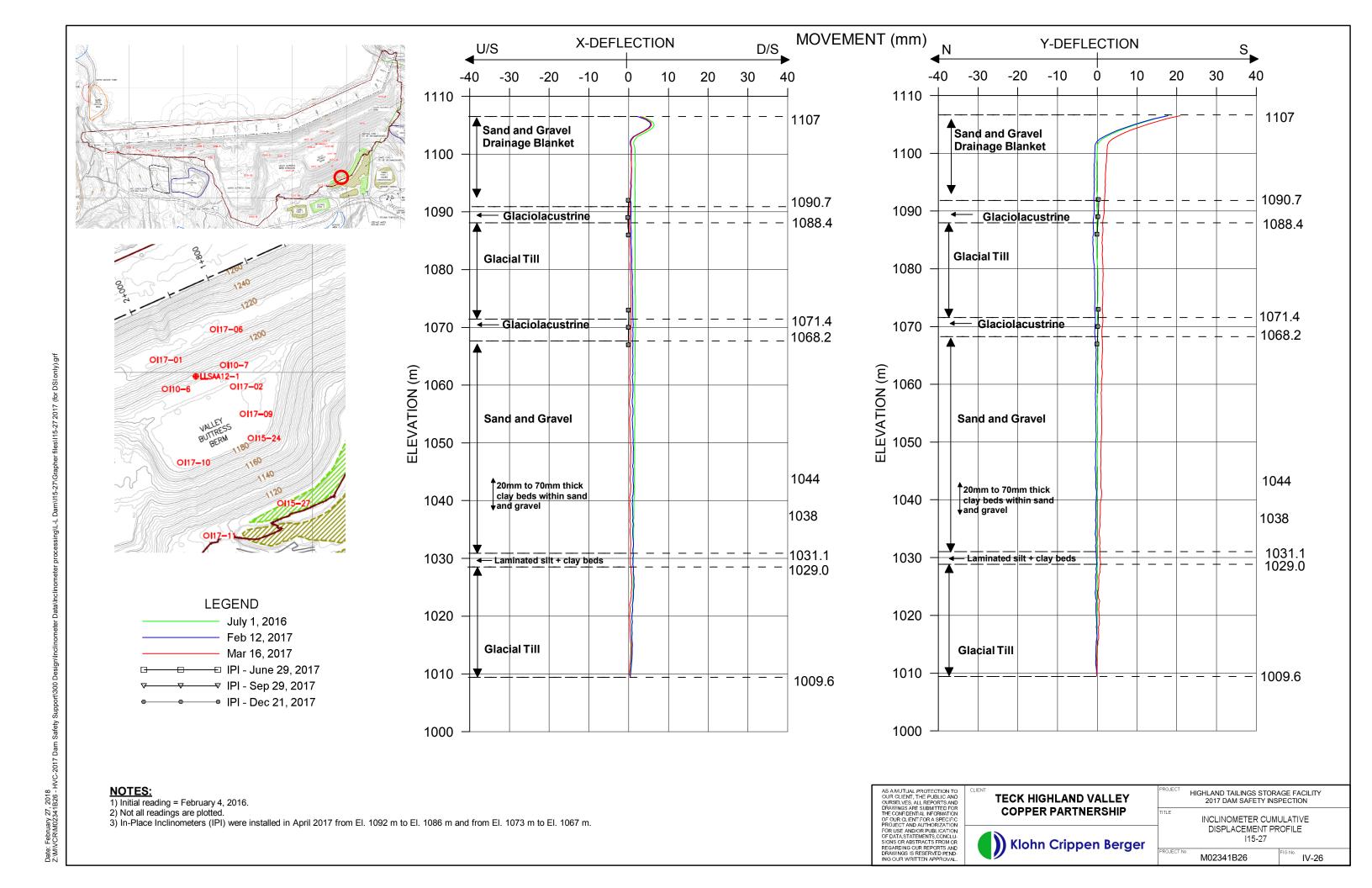


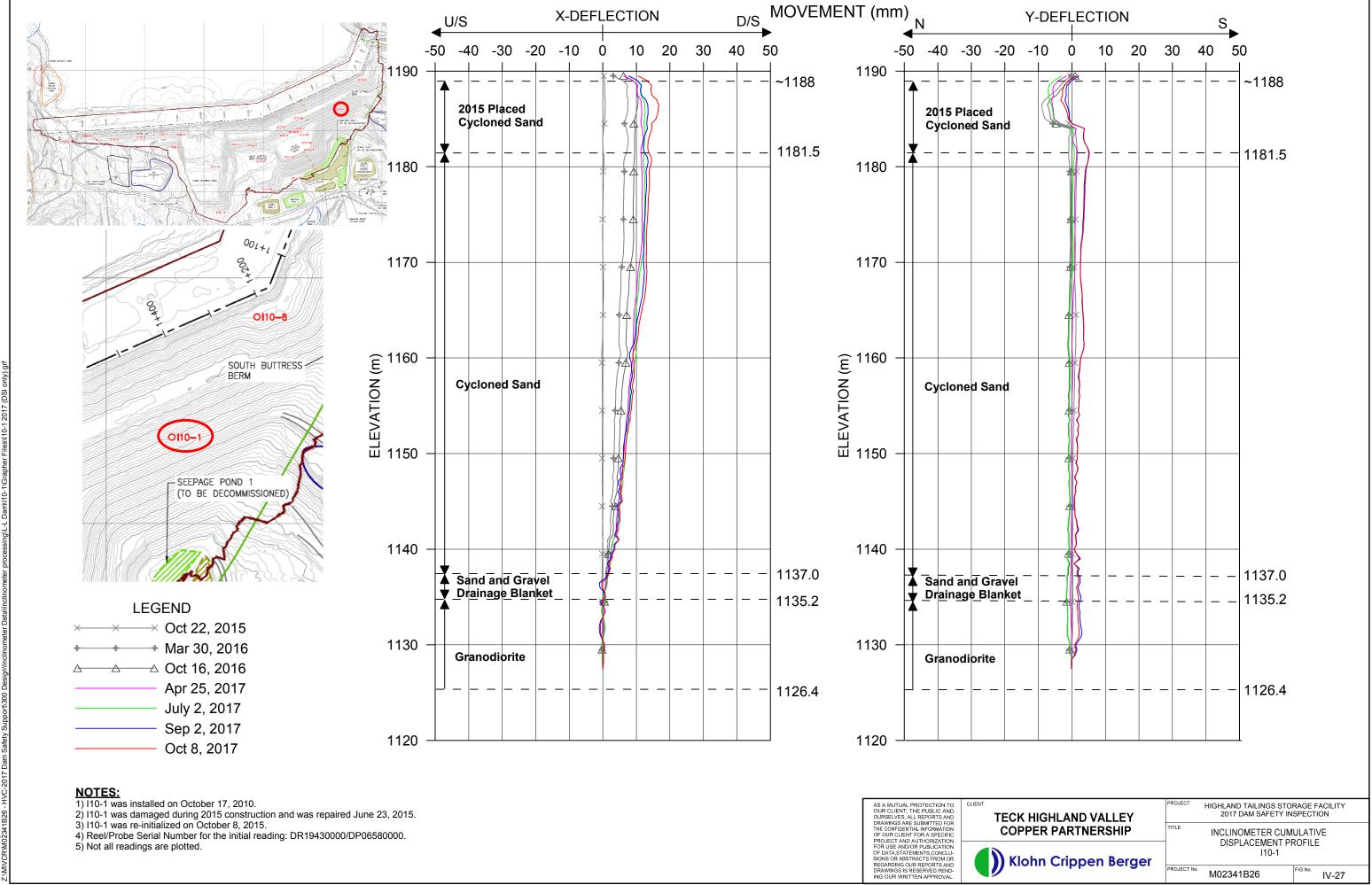




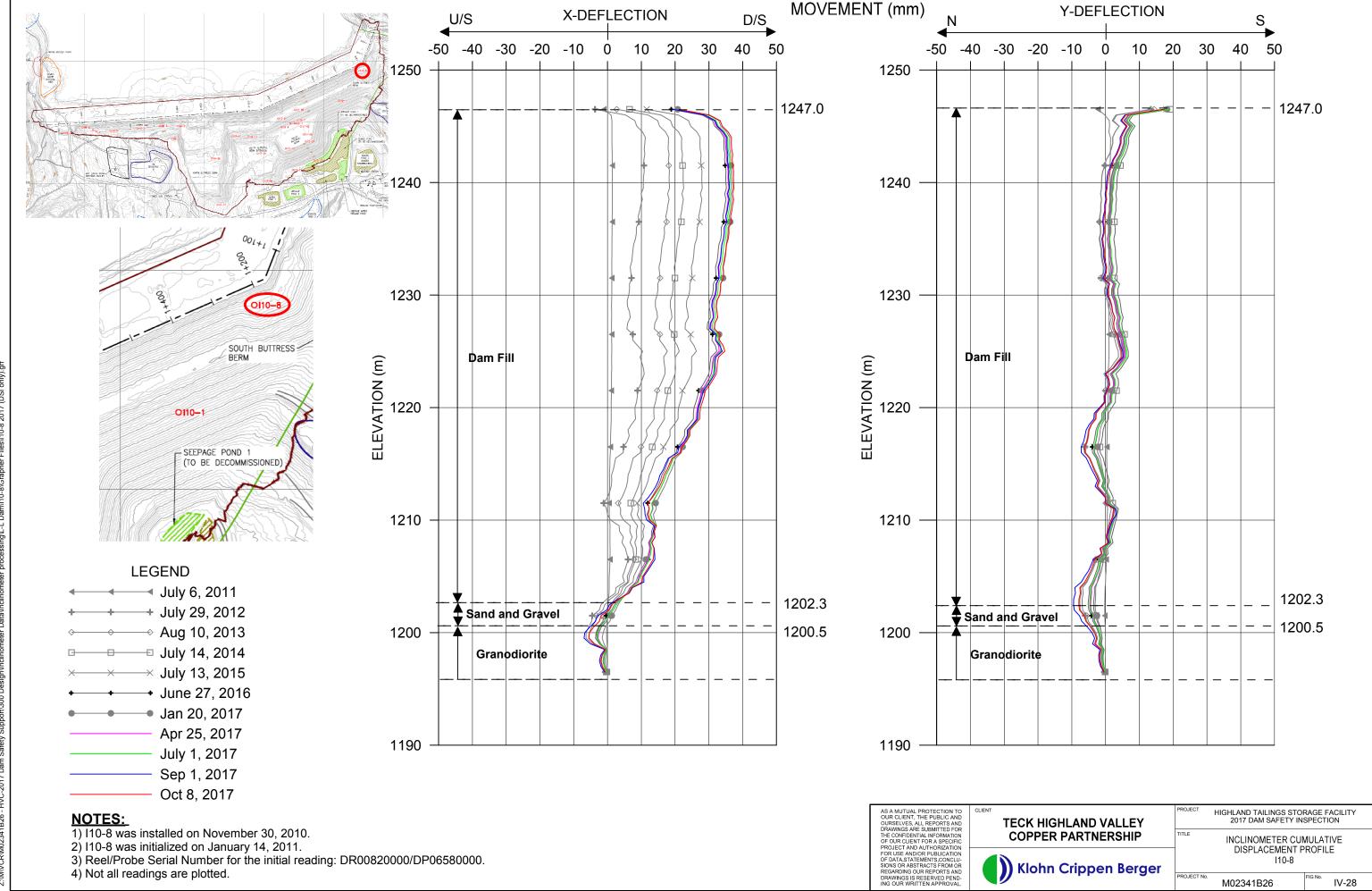






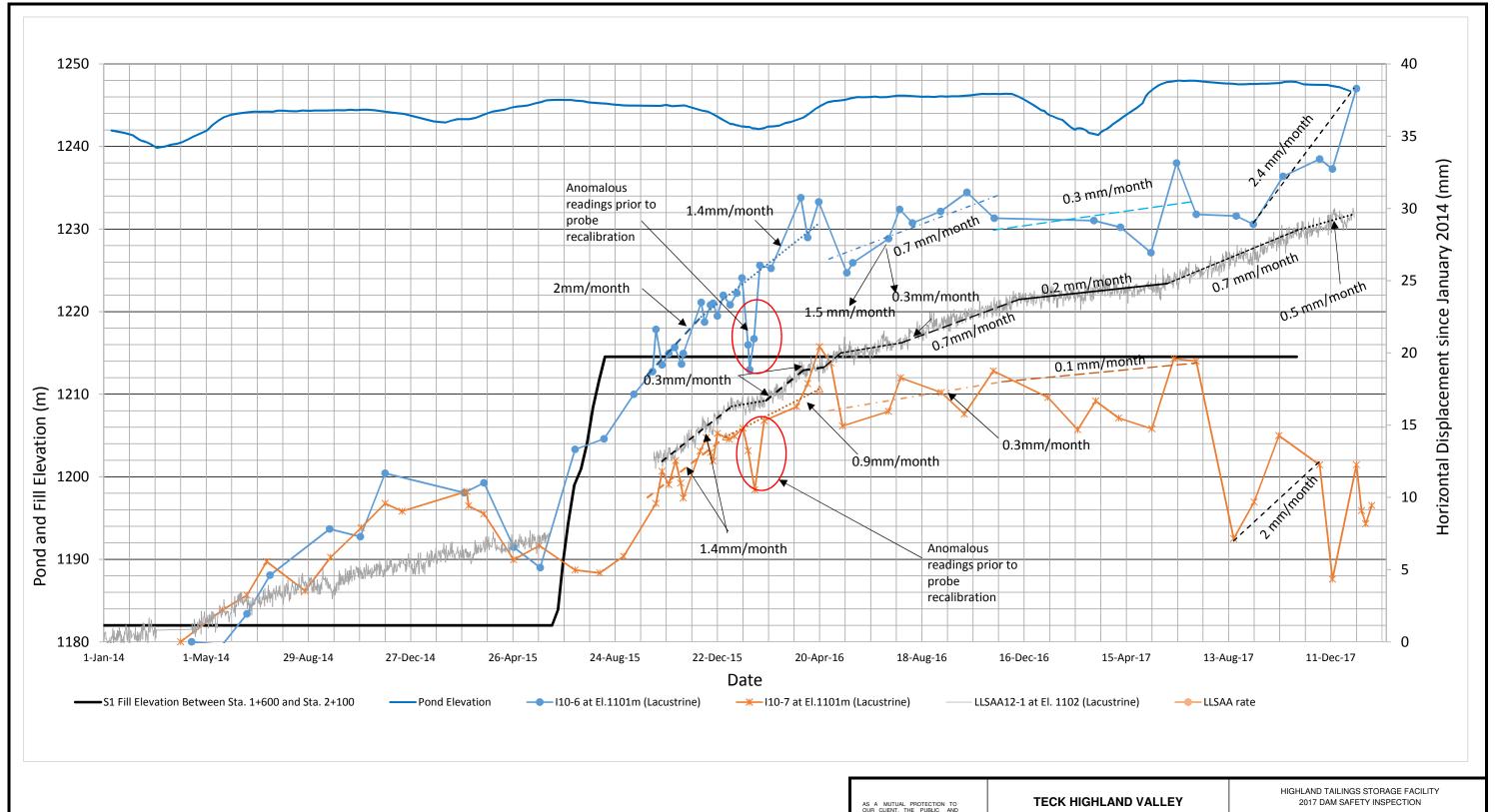


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IV-28

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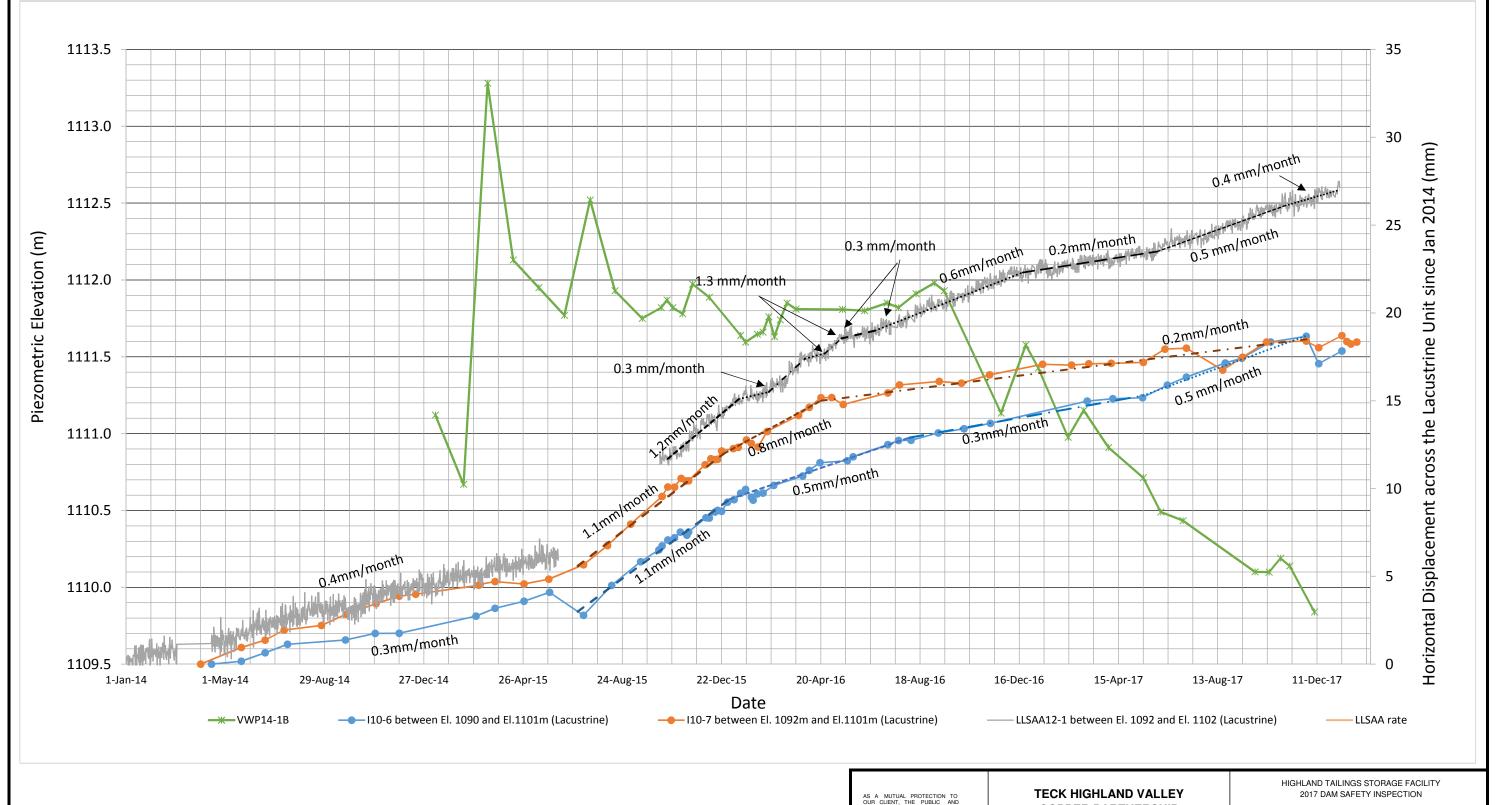


COPPER PARTNERSHIP

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L-L Dam Horizontal Displacements -Top of Lacustrine Unit 2014 - 2017

M02341B26



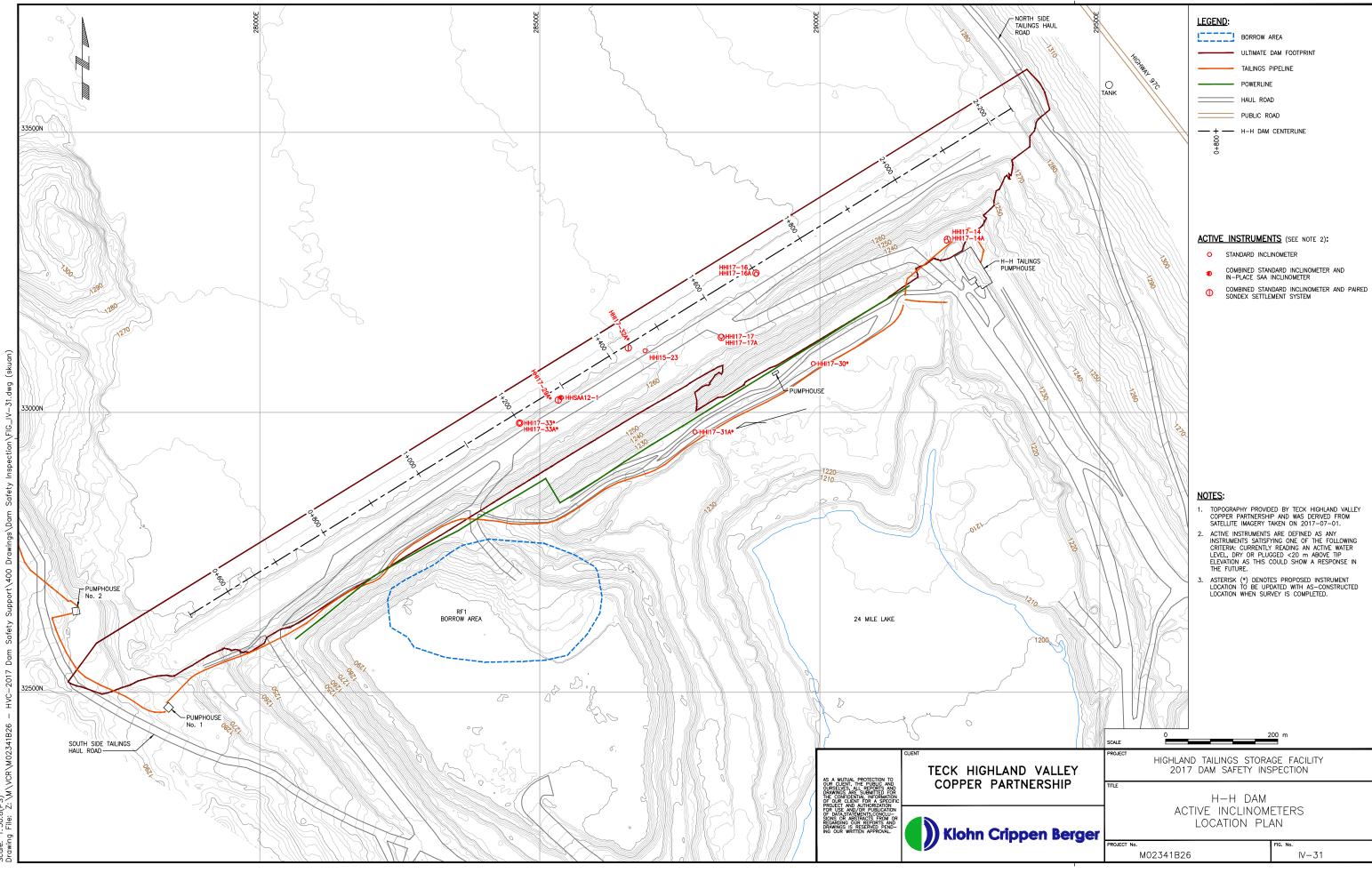
VWP14-1B is a vibrating wire piezometer installed within the Lacustrine Clay at El. 1098.1m in December 2014.

COPPER PARTNERSHIP

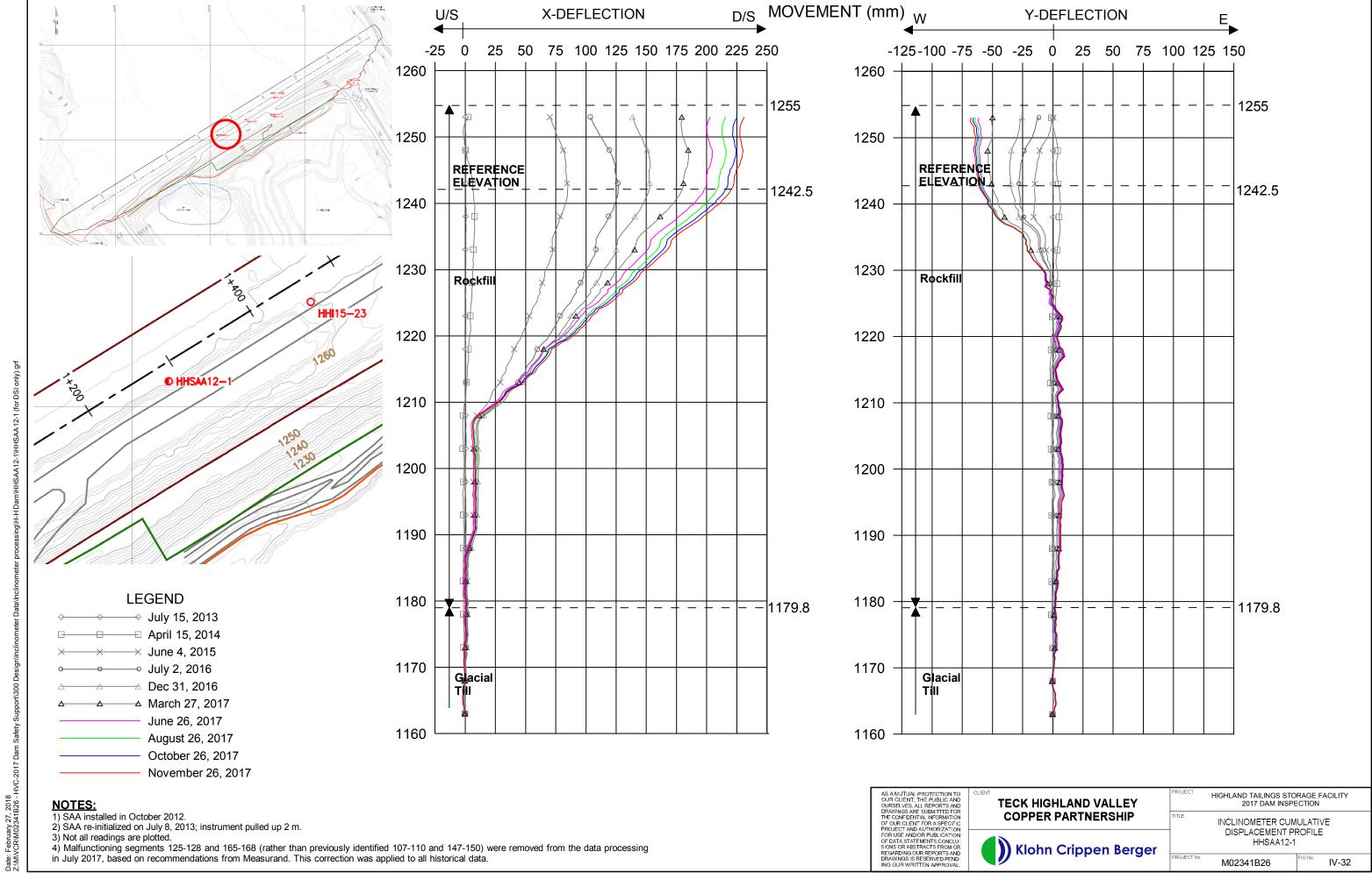


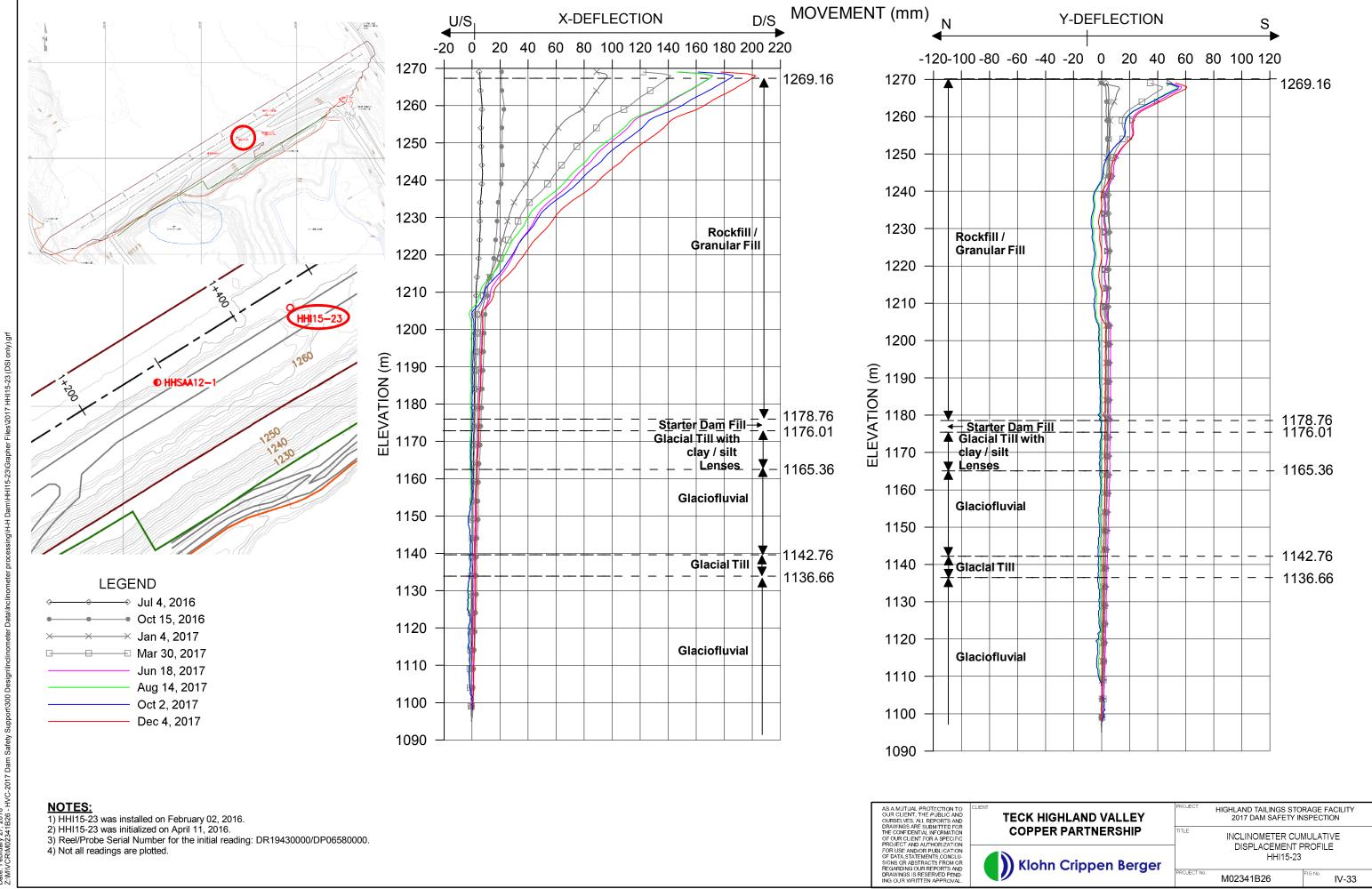
L-L Dam Horizontal Displacements Across Lacustrine Unit 2014 - 2018

M02341B26

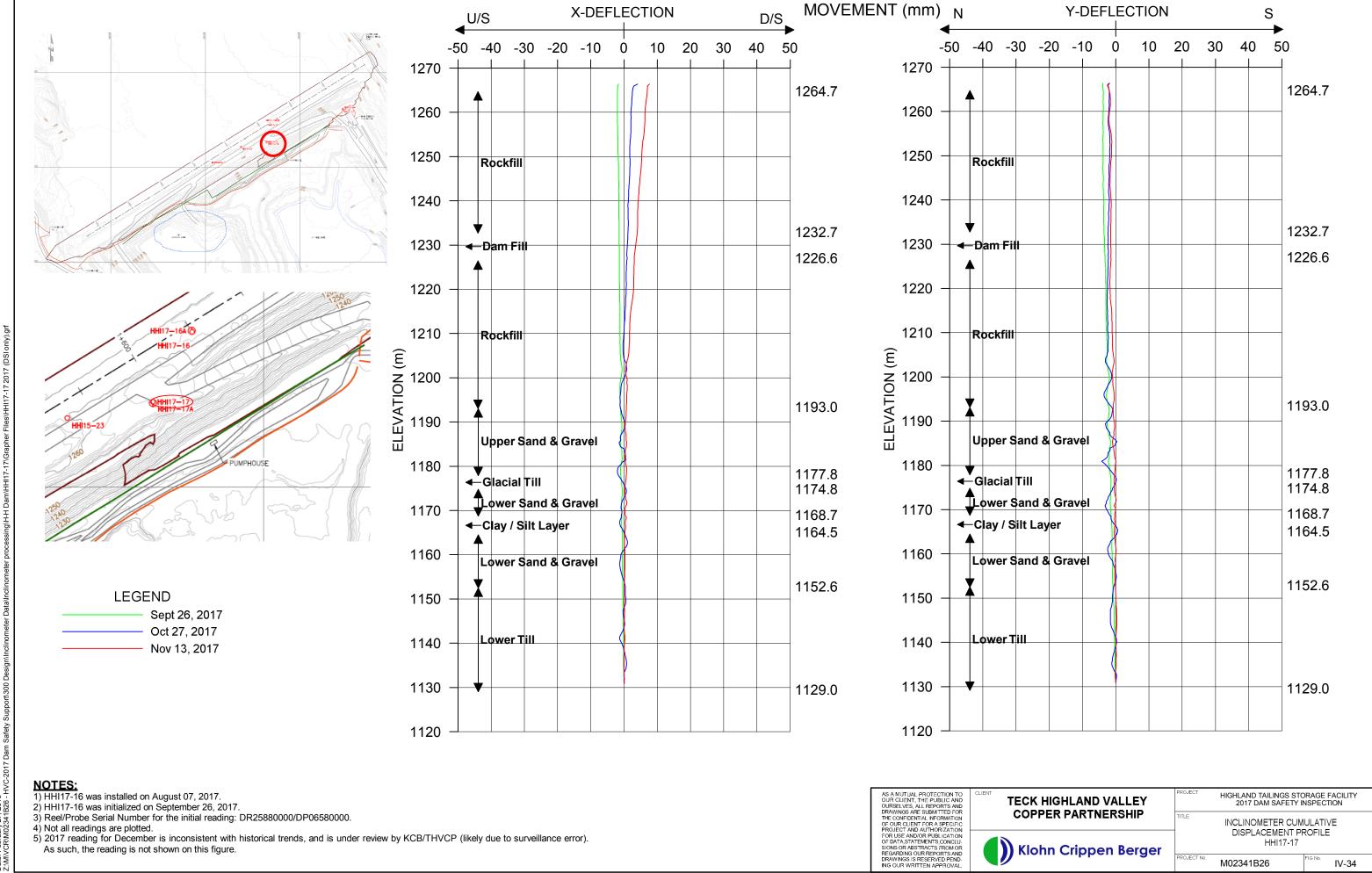


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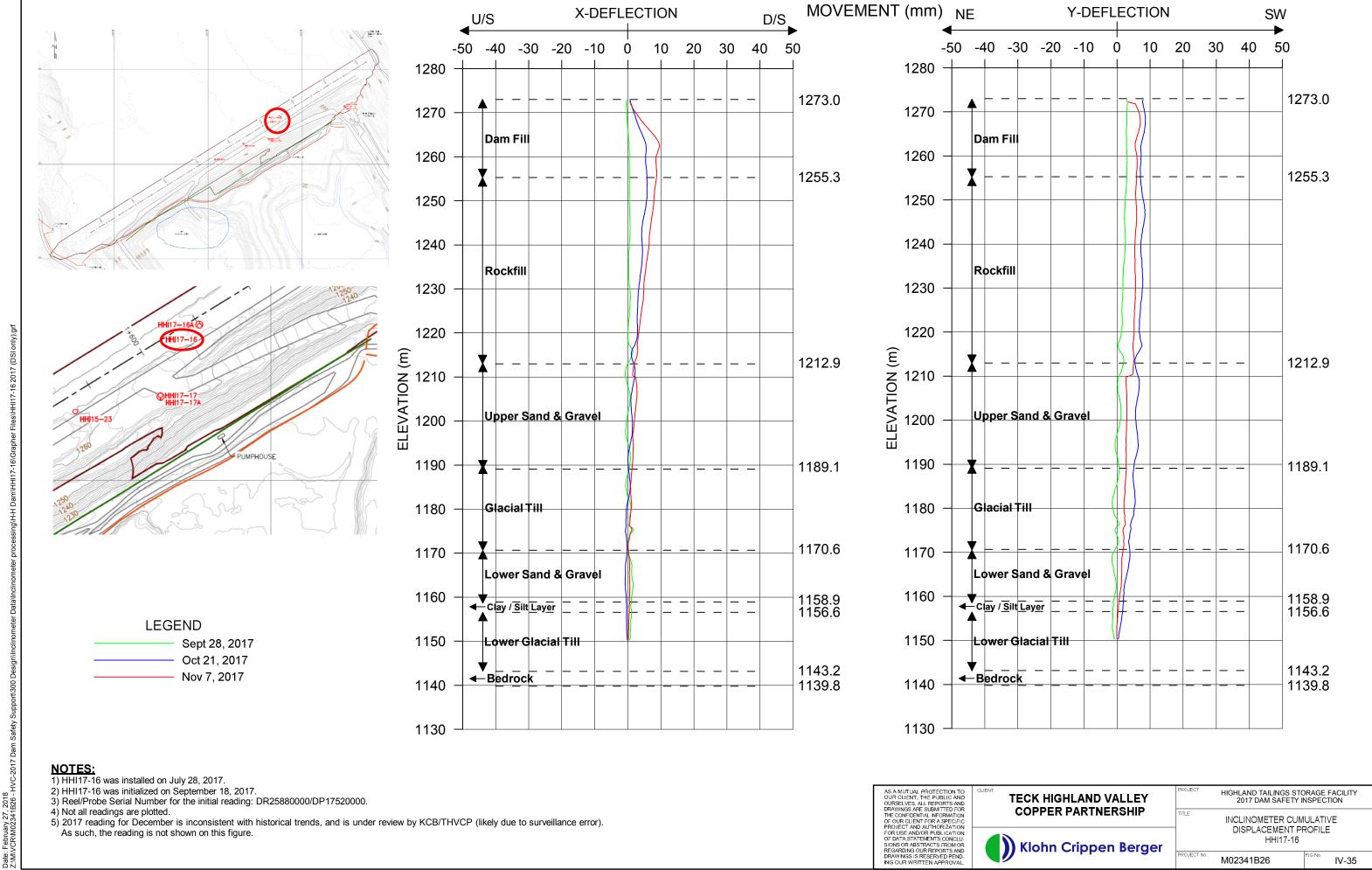


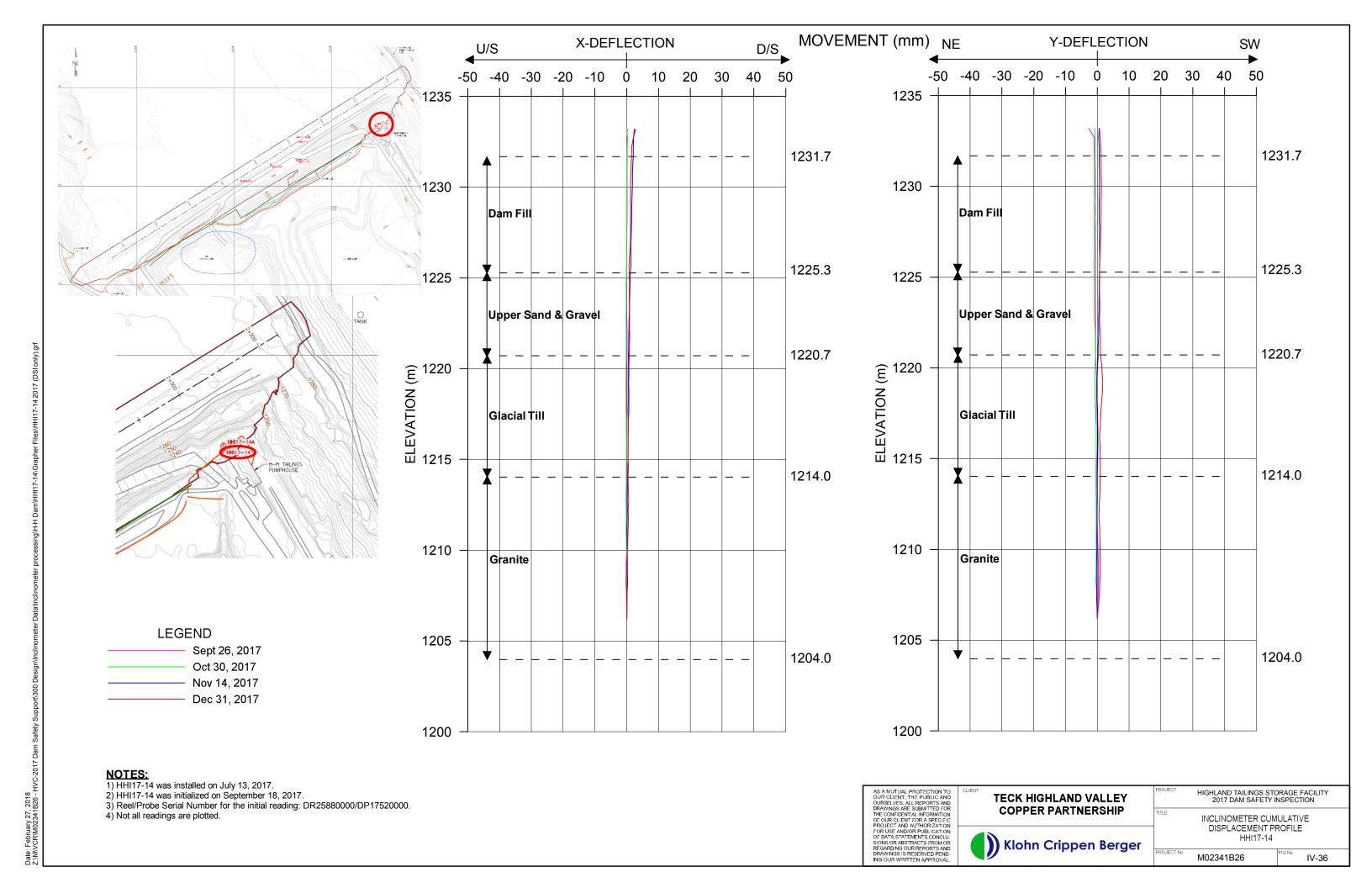


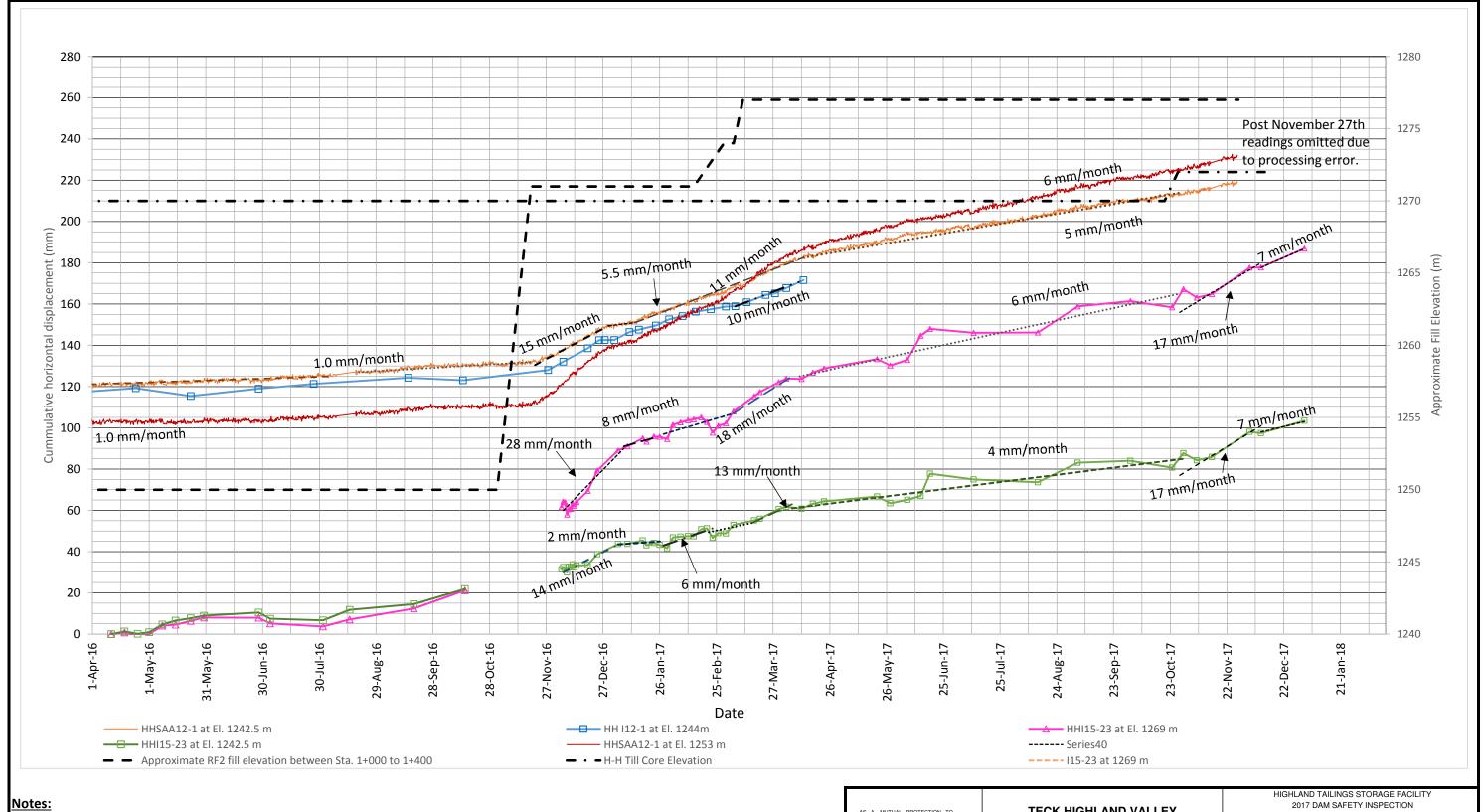
Date: February 27, 2018



Date: February 27, 2018







Dates of RF2 fill placement based on QC checklist information provided by HVC Inclinometer I12-1 damaged during April 2017

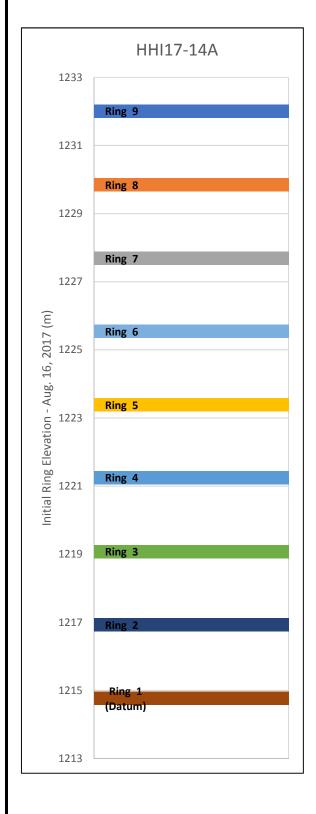
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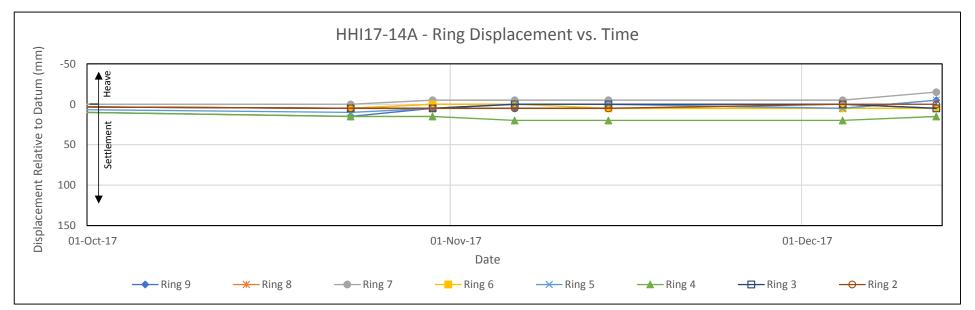
TECK HIGHLAND VALLEY COPPER PARTNERSHIP

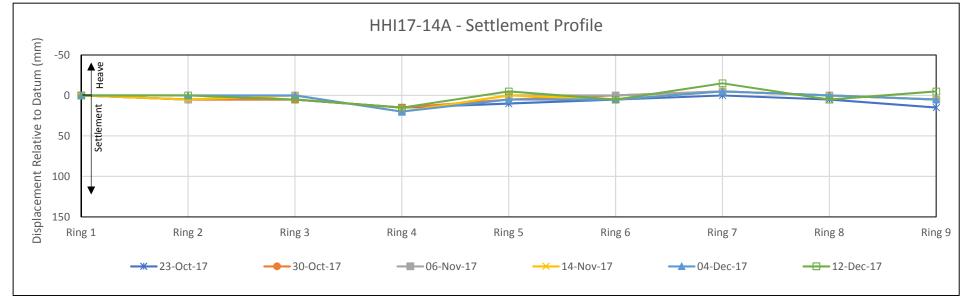
Dam F 2016 - 20 1 Berger M02341B26

| () Klohn Cripp | en Berger |
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H-H Dam Horizontal Displacements -Dam Fill 2016 - 2018







Notes

- 1. Ring displacements are calculated relative to the initial elevations for each ring. The datum is the bottom ring (Ring 1).
- 2. Positive displacements represent settlement. Negative displacements represent heave.

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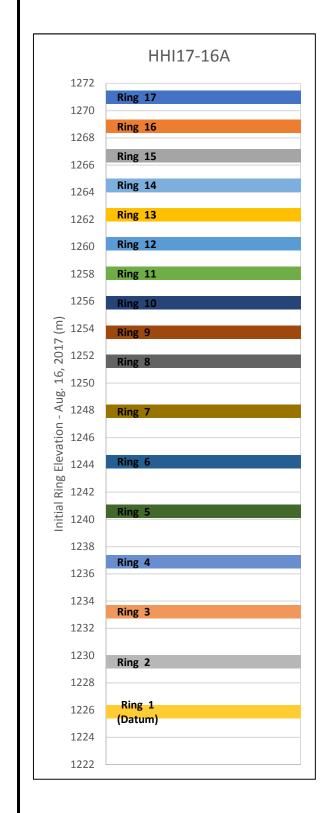
2017 DAM SAFETY INSPECTION REPORT

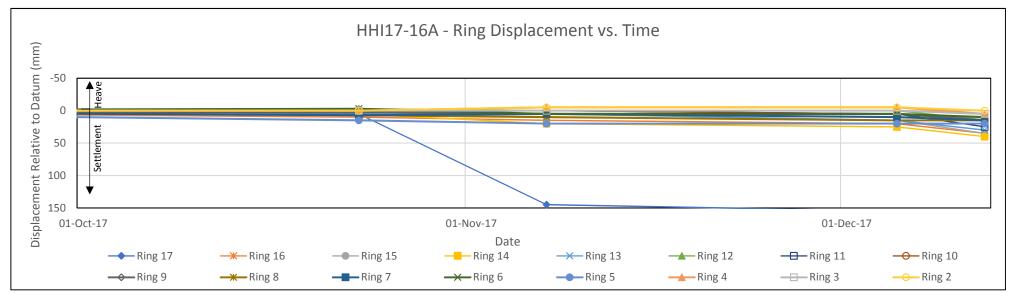
HIGHLAND TAILINGS STORAGE FACILITY

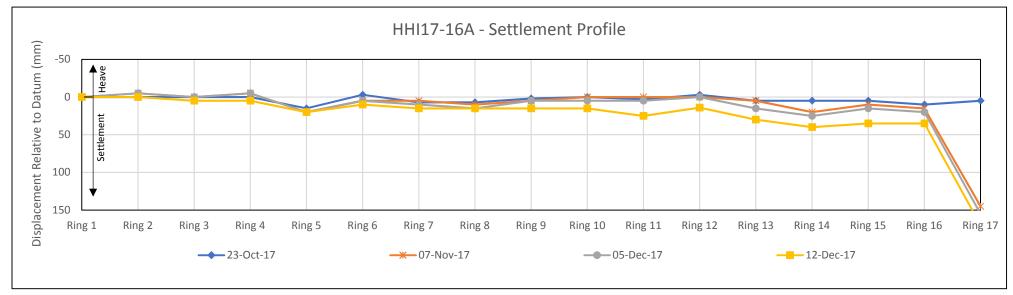


H-H Dam Vertical Displacements -HHI17-14A (Sondex)

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Notes

- 1. Ring displacements are calculated relative to the initial elevations for each ring. The datum is the bottom ring (Ring 1).
- 2. Positive displacements represent settlement. Negative displacements represent heave.
- 3. The Ring 12 reading from December 11, 2017 was not consistent with readings in the rings above and below, due to a suspected data entry error. The reading was adjusted to match the trends of Ring 11 and Ring 13.

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TECK HIGHLAND VALLEY COPPER PARTNERSHIP

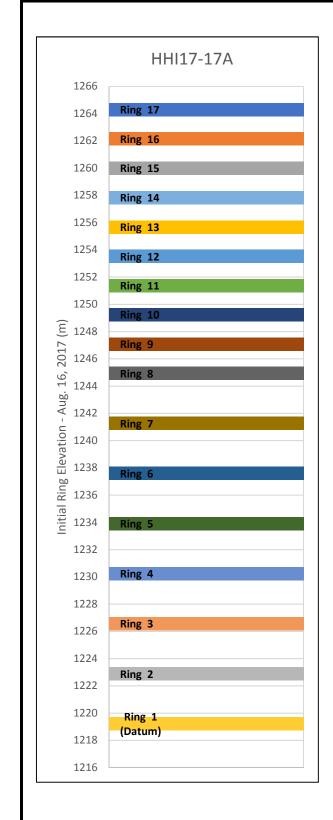
H-H Dam Vertical Displacements -HHI17-16A (Sondex)

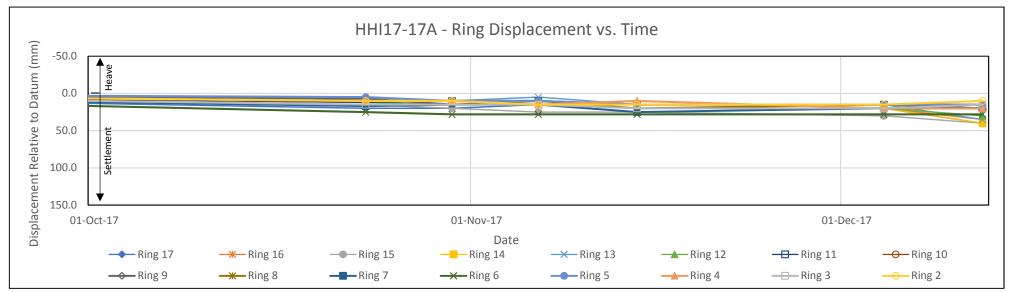
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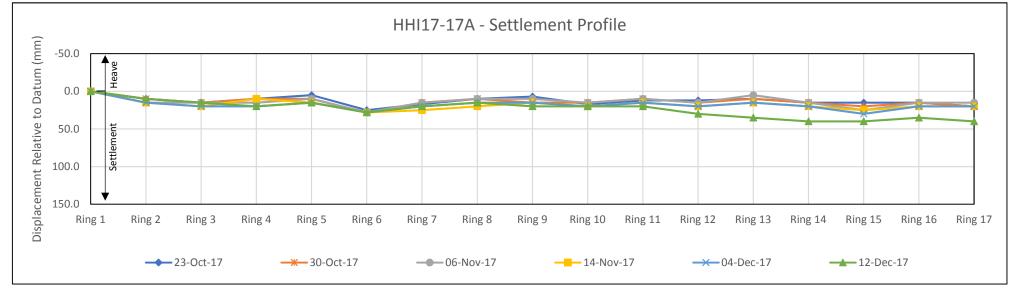
2017 DAM SAFETY INSPECTION REPORT

Klohn Crippen Berger

M02341B26 IV-39







Notes:

- 1. Ring displacements are calculated relative to the initial elevations for each ring. The datum is the bottom ring (Ring 1).
- 2. Positive displacements represent settlement. Negative displacements represent heave.
- 3. The Ring 5 reading from December 11, 2017 was not consistent with readings in the rings above and below, due to a suspected data entry error. The reading was adjusted to match the trends of Ring 4 and Ring 6.

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HIGHLAND TAILINGS STORAGE FACILITY 2017 DAM SAFETY INSPECTION REPORT

H-H Dam Vertical Displacements -HHI17-17A (Sondex)

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APPENDIX V

Piezometer Plots

Appendix V Piezometer Plots

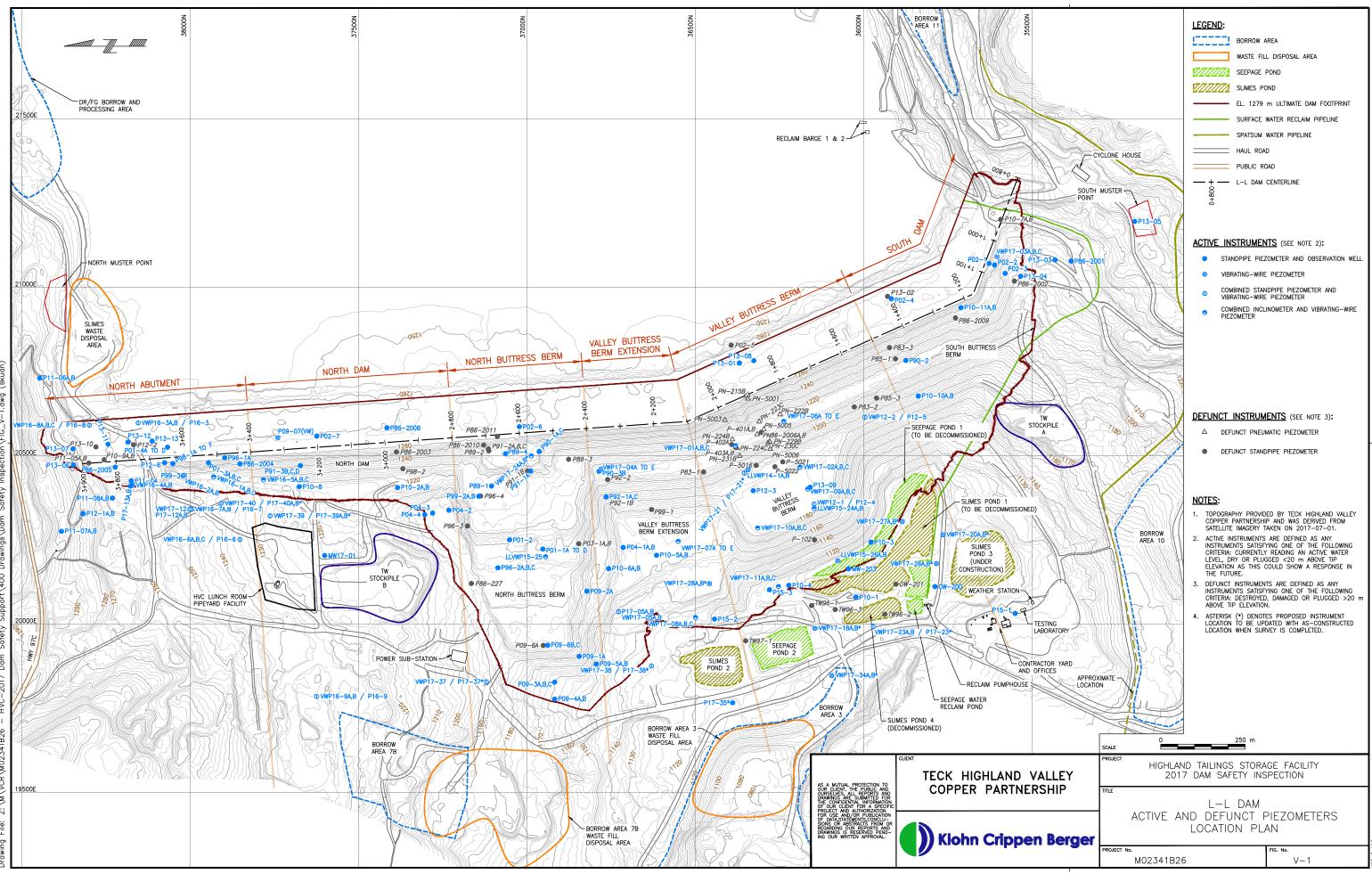
L-L Dam

| Figure V-1 | L-L Dam Active and Defunct Piezometers Location Plan |
|--------------|--|
| Figure V-2a | L-L Dam Piezometric Data U/S Overburden and Bedrock - 2001-2017 - North Abutment (North of STN 3+400 m) |
| Figure V-2b | L-L Dam Piezometric Data U/S Overburden and Bedrock - 2012-2017 - North Abutment (North of STN 3+400 m) |
| Figure V-3a | L-L Dam Piezometric Data D/S Overburden - 1994-2017 - North Abutment (North of STN 3+400 m) |
| Figure V-3b | L-L Dam Piezometric Data D/S Overburden - 2012-2017 - North Abutment (North of STN 3+400 m) |
| Figure V-4a | L-L Dam Piezometric Data D/S Bedrock - 1994-2017 - North Abutment (North of STN 3+400 m) |
| Figure V-4b | L-L Dam Piezometric Data D/S Bedrock - 2012-2017 - North Abutment (North of STN 3+400 m) |
| Figure V-5a | L-L Dam Piezometric Data U/S Damfill and Overburden - 1994-2017 - North Dam (STNS 2+800 m to 3+400 m) |
| Figure V-5b | L-L Dam Piezometric Data U/S Damfill and Overburden - 2012-2017 - North Dam (STNS 2+800 m to 3+400 m) |
| Figure V-6a | L-L Dam Piezometric Data Bedrock - 1994-2017 - North Dam (STNS 2+800 m to 3+400 m) |
| Figure V-6b | L-L Dam Piezometric Data Bedrock - 2012-2017 - North Dam (STNS 2+800 m to 3+400 m) |
| Figure V-7a | L-L Dam Piezometric Data U/S Dam Fill - 1994-2017 - North Buttress Berm (STNS 2+400 m to 2+800 m) |
| Figure V-7b | L-L Dam Piezometric Data U/S Dam Fill - 2012-2017 - North Buttress Berm (STNS 2+400 m to 2+800 m) |
| Figure V-8a | L-L Dam Piezometric Data Overburden - 1994-2017 - North Buttress Berm (STNS 2+400 m to 2+800 m) |
| Figure V-8b | L-L Dam Piezometric Data Overburden - 2012-2017 - North Buttress Berm (STNS 2+400 m to 2+800 m) |
| Figure V-9a | L-L Dam Piezometric Data Volcanic Bedrock - 1994-2017 - North Buttress Berm (STNS 2+400 m to 2+800 m) |
| Figure V-9b | L-L Dam Piezometric Data Volcanic Bedrock - 2012-2017 - North Buttress Berm (STNS 2+400 m to 2+800 m) |
| Figure V-10a | L-L Dam Piezometric Data Sedimentary Bedrock - 1994-2017 - North Buttress Berm (STNS 2+400 m to 2+800 m) |

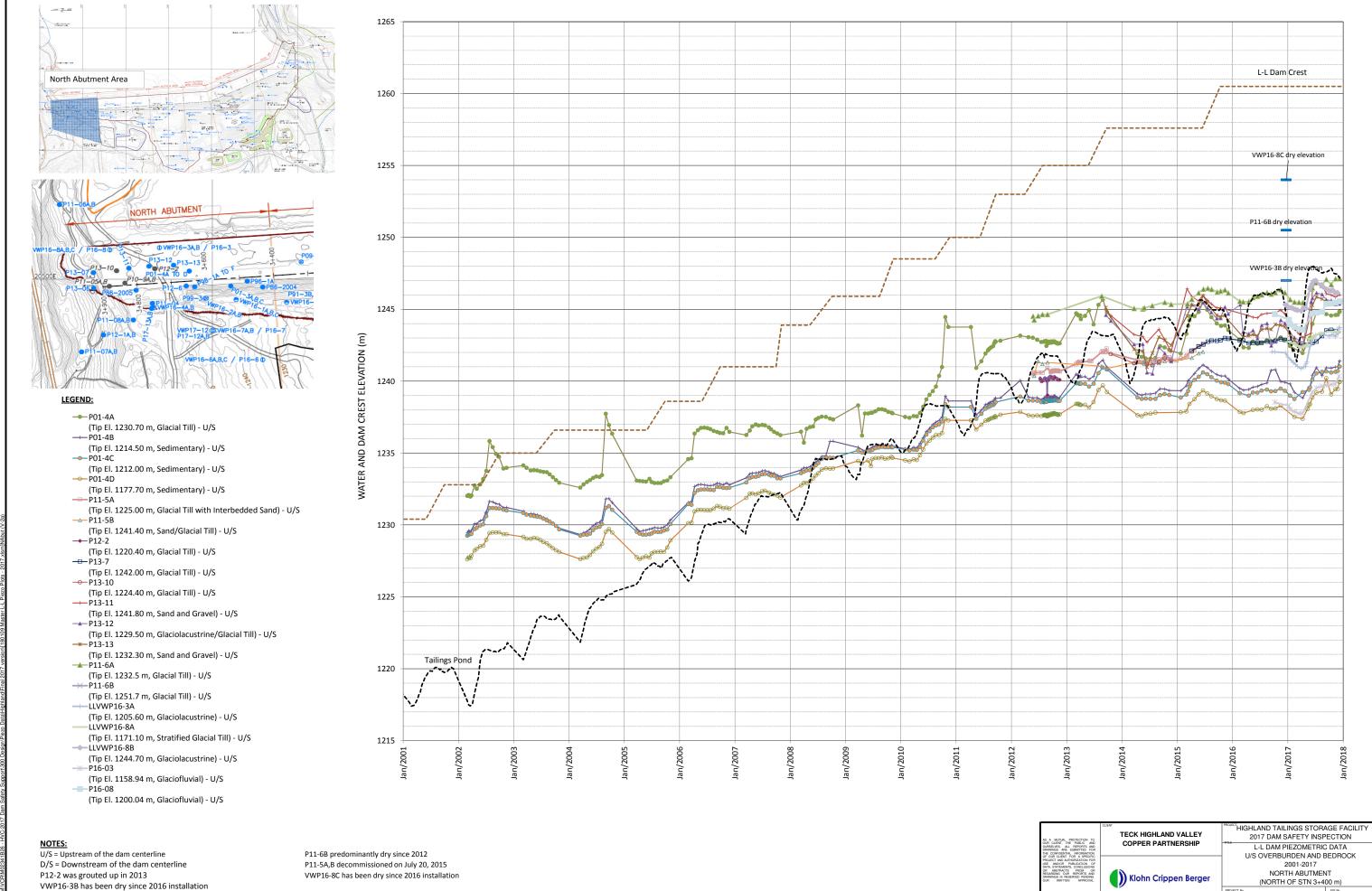
- Figure V-10b L-L Dam Piezometric Data Sedimentary Bedrock 2012-2017 North Buttress Berm (STNS 2+400 m to 2+800 m)
- Figure V-11a L-L Dam Piezometric Data Overburden 1994-2017 Valley Buttress Berm Extension (STNS 2+100 m to 2+400 m)
- Figure V-11b L-L Dam Piezometric Data Overburden 2012-2017 Valley Buttress Berm Extension (STNS 2+100 m to 2+400 m)
- Figure V-12a L-L Dam Piezometric Data Bedrock 1994-2017 Valley Buttress Berm Extension (STNS 2+100 m to 2+400 m)
- Figure V-12b L-L Dam Piezometric Data Bedrock 2012-2017 Valley Buttress Berm Extension (STNS 2+100 m to 2+400 m)
- Figure V-13a L-L Dam Piezometric Data Dam Fill 1989-2017 Valley Buttress Berm (STNS 1+500 m to 2+100 m)
- Figure V-13b L-L Dam Piezometric Data Dam Fill 2012-2017 Valley Buttress Berm (STNS 1+500 m to 2+100 m)
- Figure V-14a L-L Dam Piezometric Data Lacustrine 1976-2017 Valley Buttress Berm (STNS 1+500 m to 2+100 m)
- Figure V-14b L-L Dam Piezometric Data Lacustrine 2012-2017 Valley Buttress Berm (STNS 1+500 m to 2+100 m)
- Figure V-15 L-L Dam Piezometric Data Glaciolacustrine 2017 Valley Buttress Berm (STNS 1+500 m to 2+100 m)
- Figure V-16a L-L Dam Piezometric Data Glacial Till 1976-2017 Valley Buttress Berm (STNS 1+500 m to 2+100 m)
- Figure V-16b L-L Dam Piezometric Data Glacial Till 2012-2017 Valley Buttress Berm (STNS 1+500 m to 2+100 m)
- Figure V-17a L-L Dam Piezometric Data Dam Fill 2002-2017 South Dam (STNS 0+800 m to 1+500 m)
- Figure V-17b L-L Dam Piezometric Data Dam Fill 2012-2017 South Dam (STNS 0+800 m to 1+500 m)
- Figure V-18a L-L Dam Piezometric Data Granodiorite Bedrock and Glacial Till 1994-2017 South Dam (STNS 0+800 m to 1+500 m)
- Figure V-18b L-L Dam Piezometric Data Granodiorite Bedrock and Glacial Till 2012-2017 South Dam (STNS 0+800 m to 1+500 m)

H-H Dam

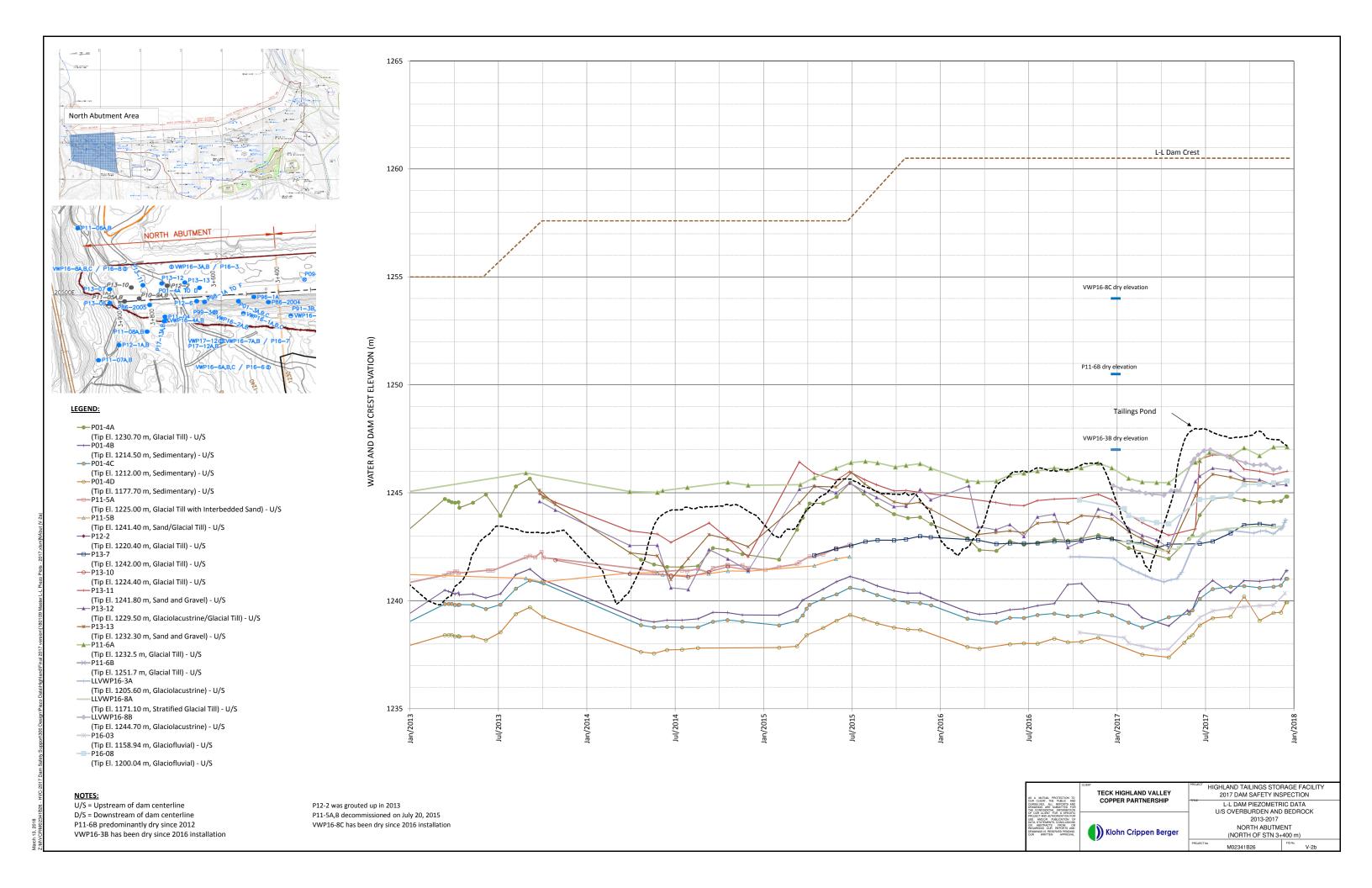
| Figure V-19 | H-H Dam Active and Defunct Piezometers Location Plan |
|--------------|--|
| Figure V-20a | H-H Dam Piezometric Data Upstream and Abutments - 1987-2017 |
| Figure V-20b | H-H Dam Piezometric Data Upstream and Abutments - 2013-2017 |
| Figure V-21a | H-H Dam Piezometric Data - 1994-2017 – Sand and Gravel and Dam Fill |
| Figure V-21b | H-H Dam Piezometric Data - 2013-2017 – Sand and Gravel and Dam Fill |
| Figure V-22a | H-H Dam Piezometric Data - 1987-2017 – Glacial Till and Glaciolacustrine |
| Figure V-22b | H-H Dam Piezometric Data - 2013-2017 – Glacial Till and Glaciolacustrine |
| Figure V-23 | H-H Dam Piezometric Data - 2013-2017 – Bedrock |

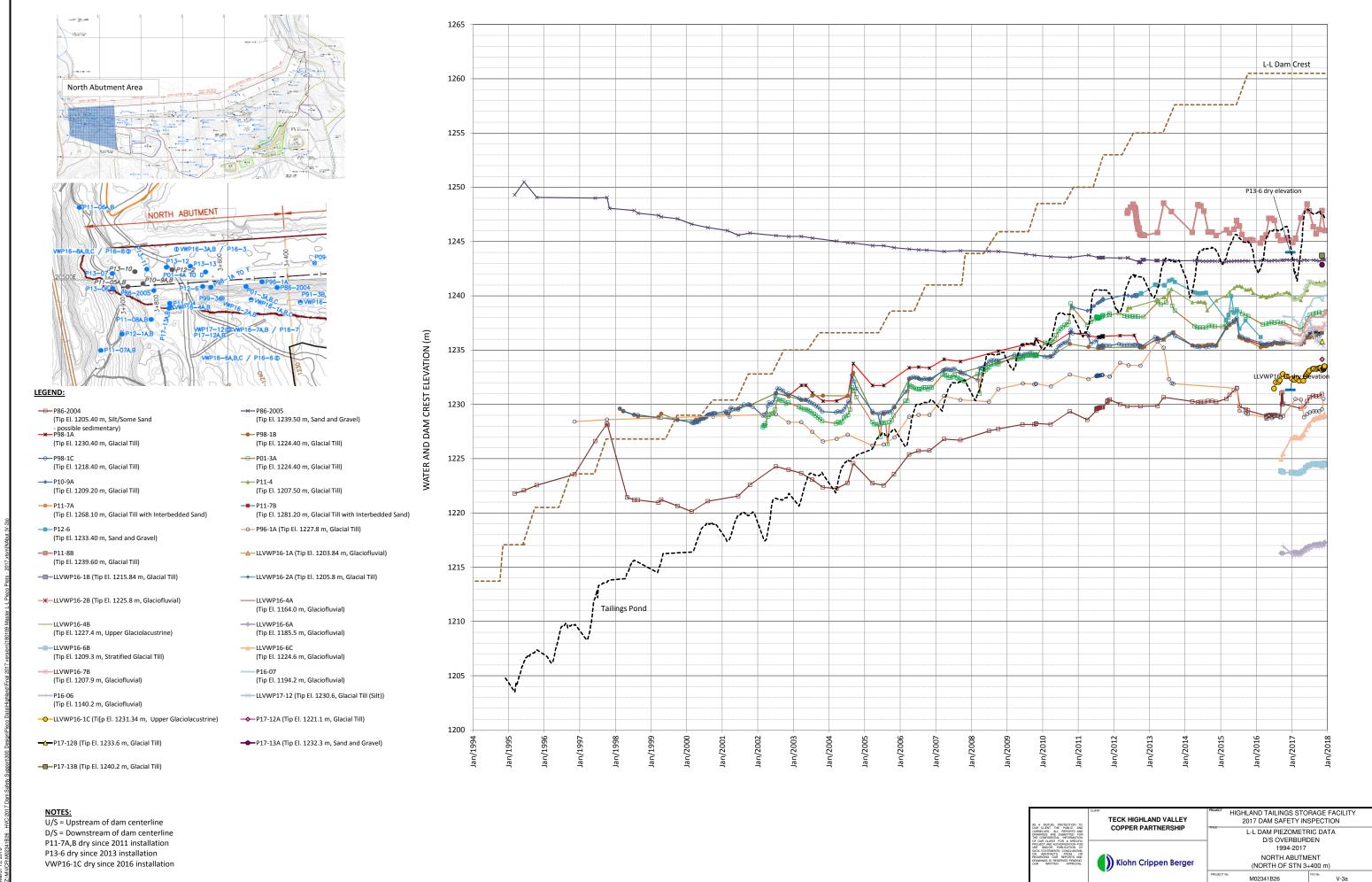


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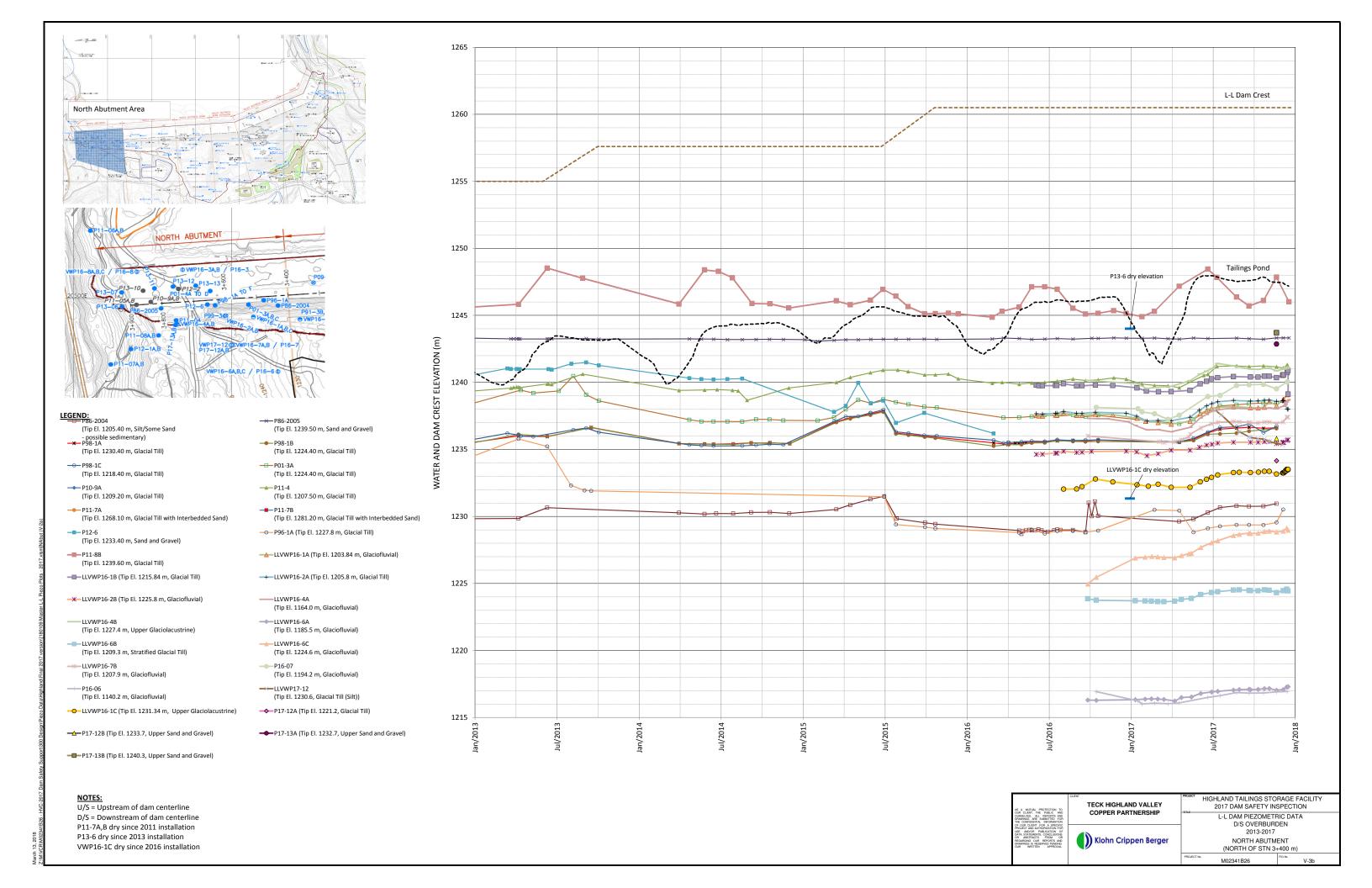


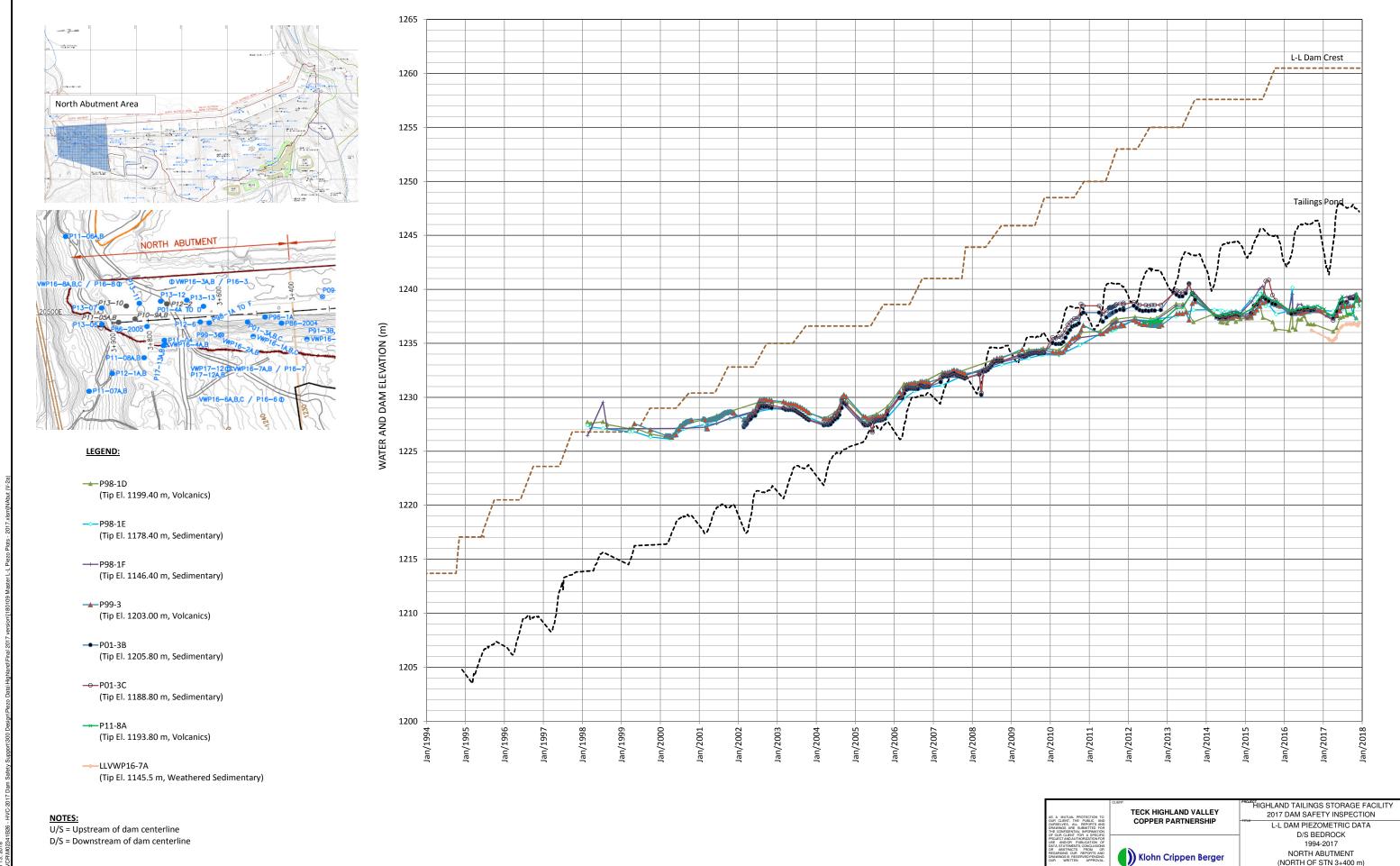
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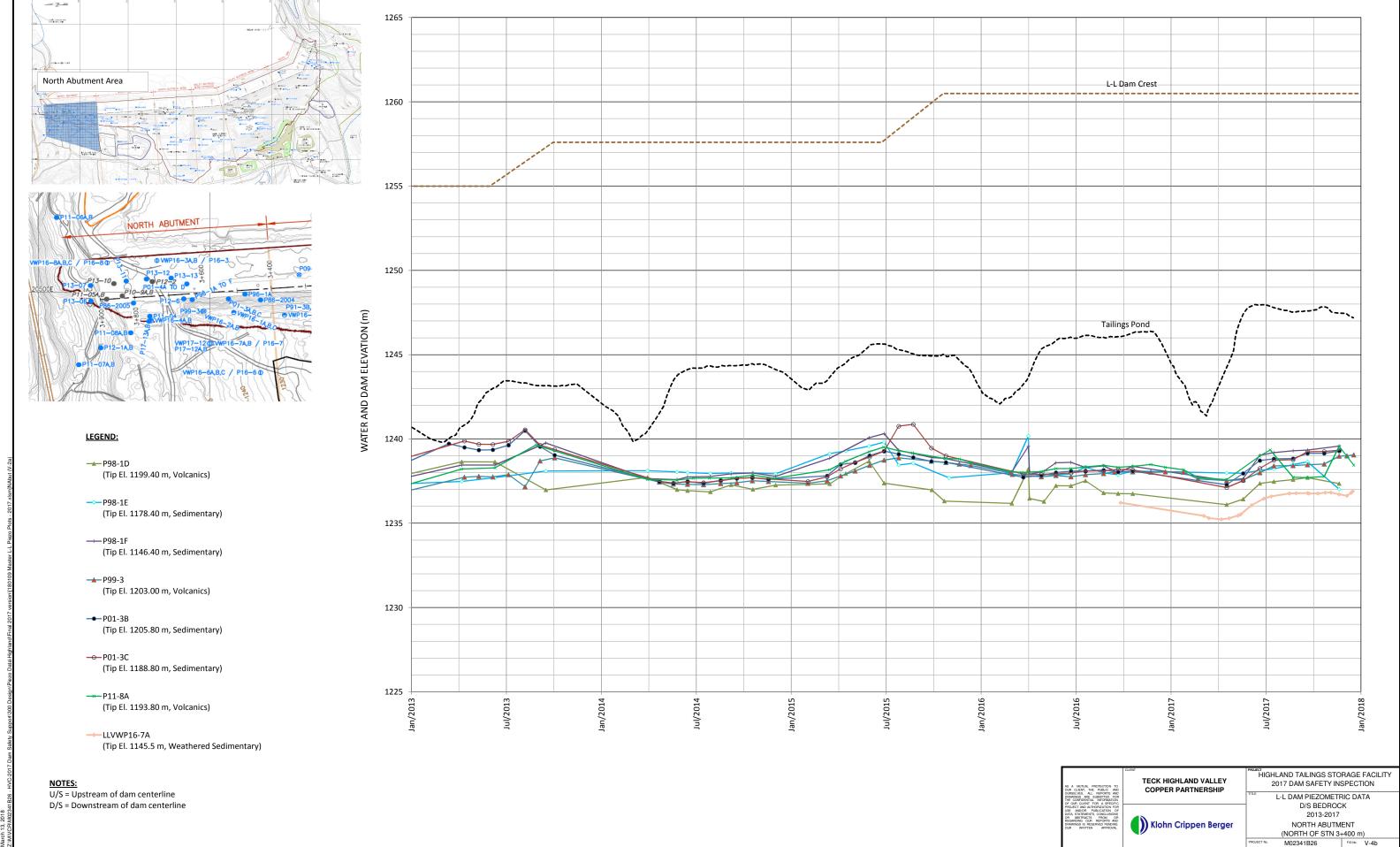


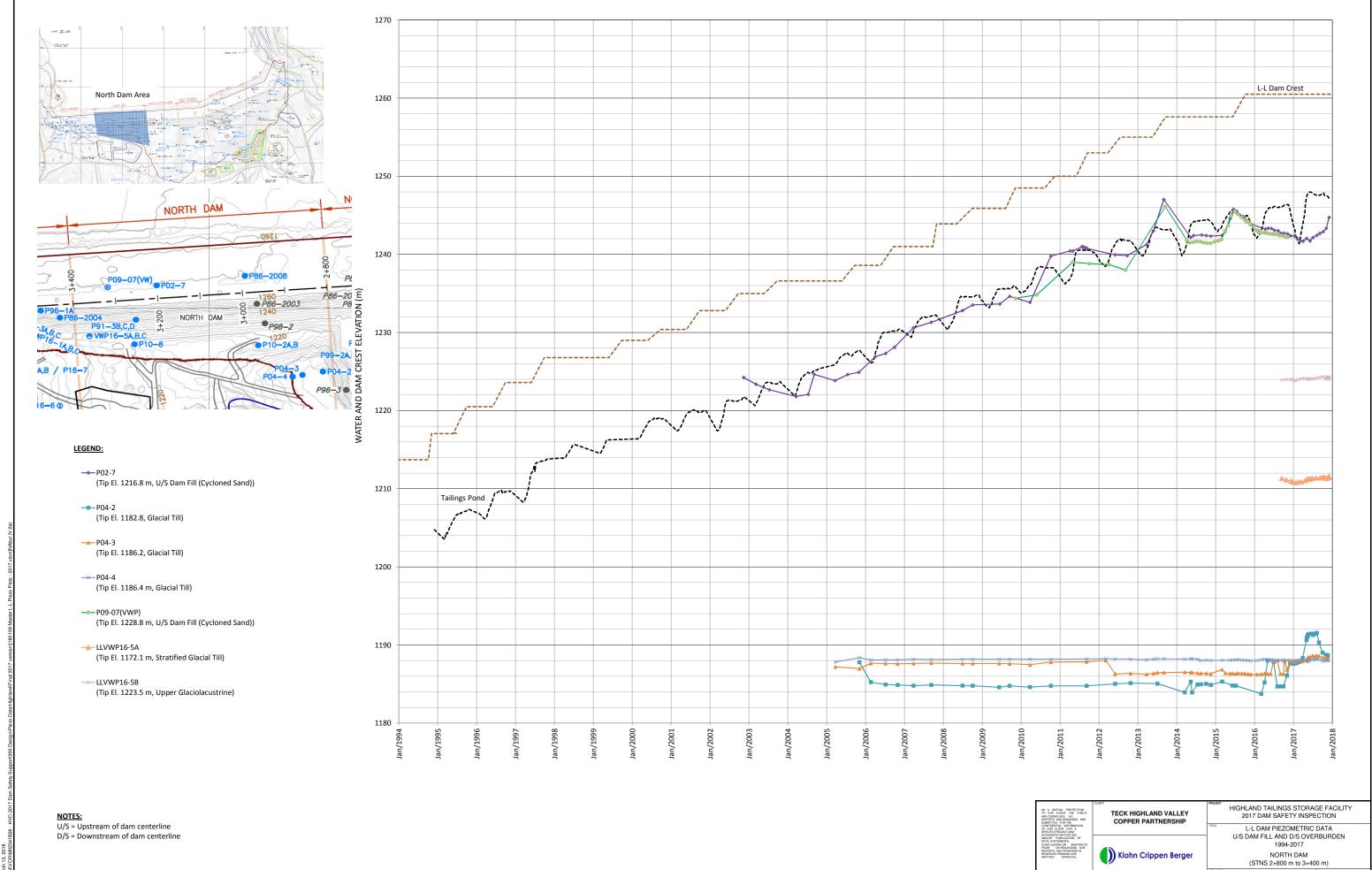
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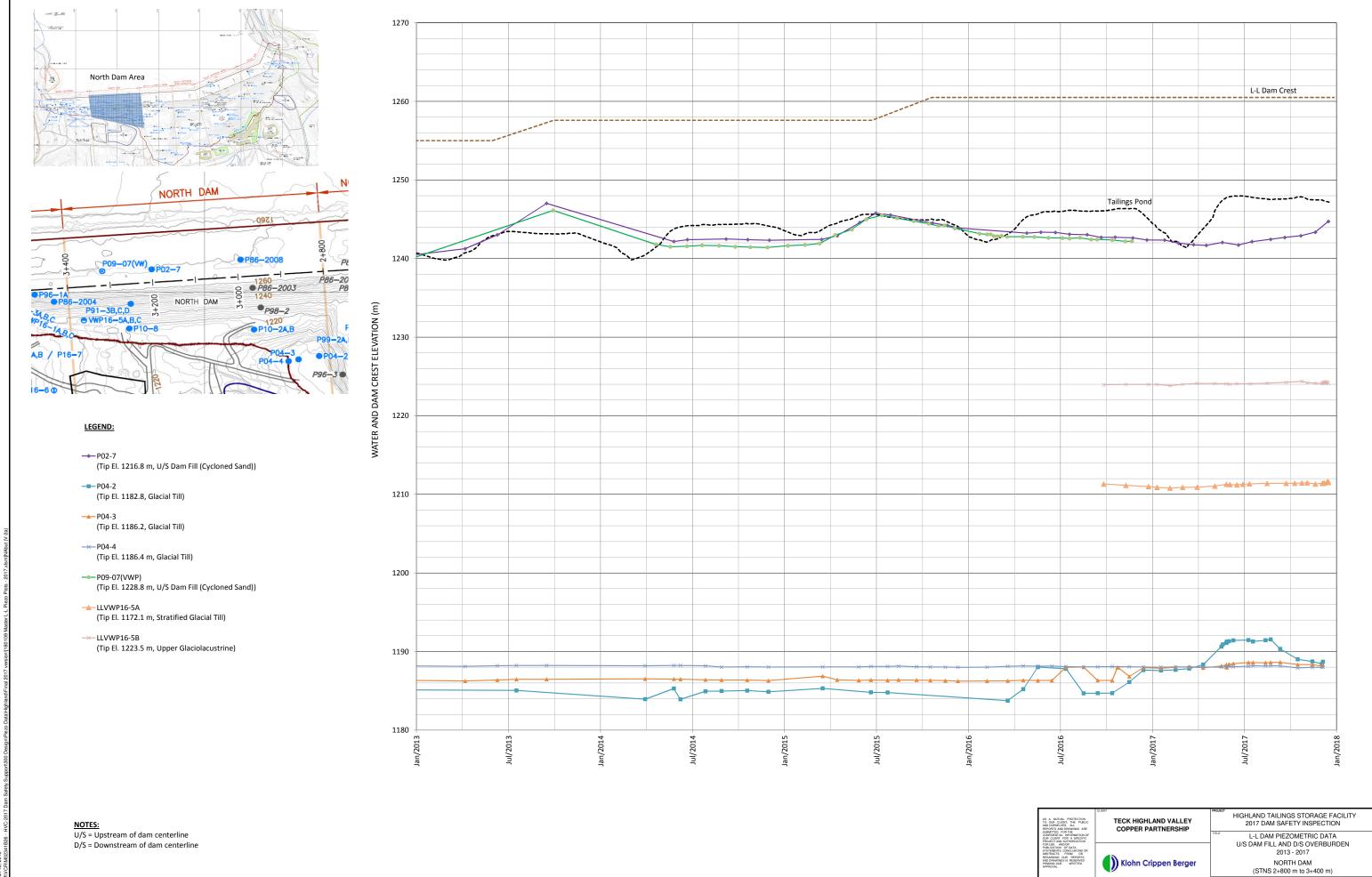




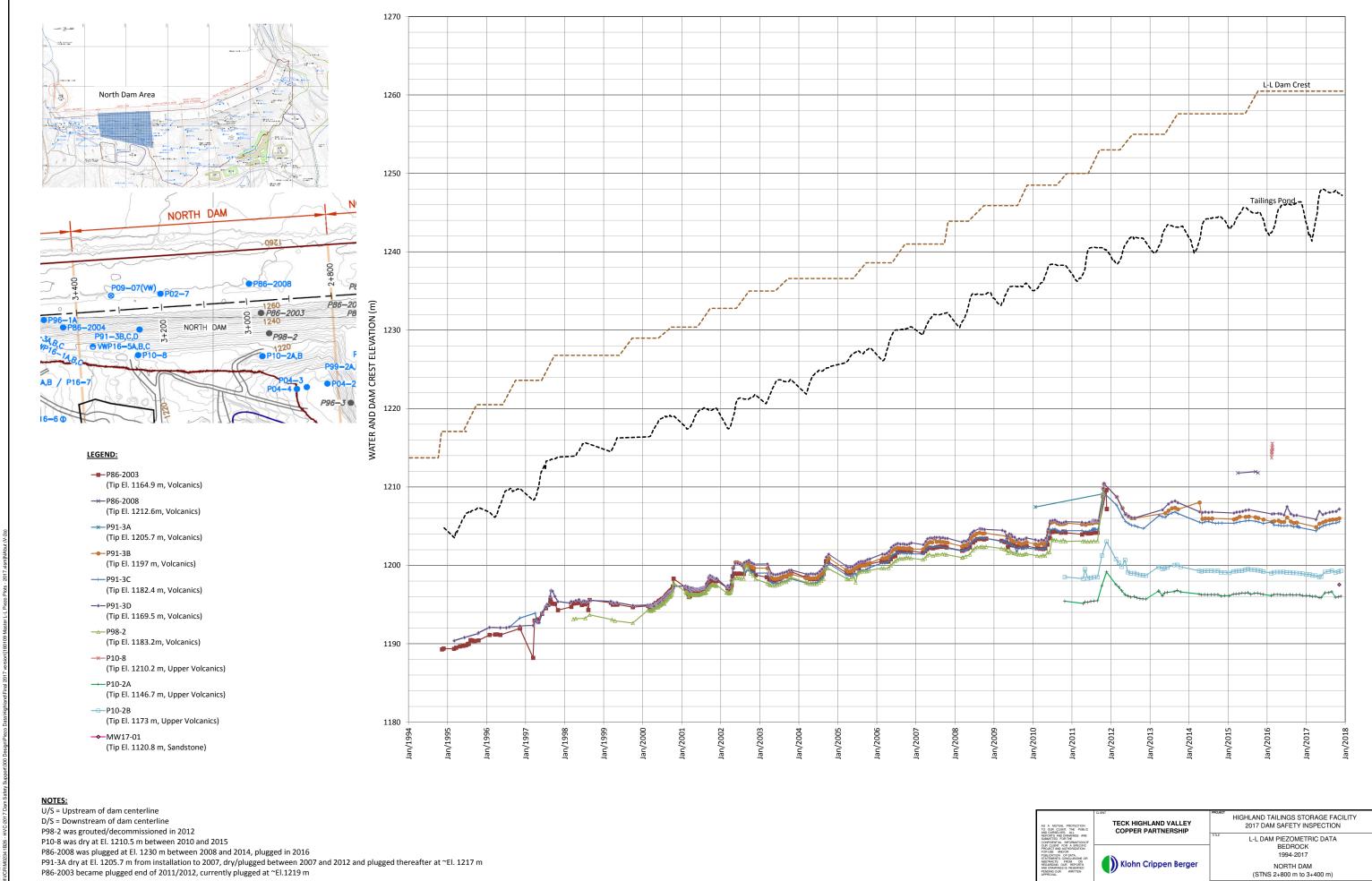
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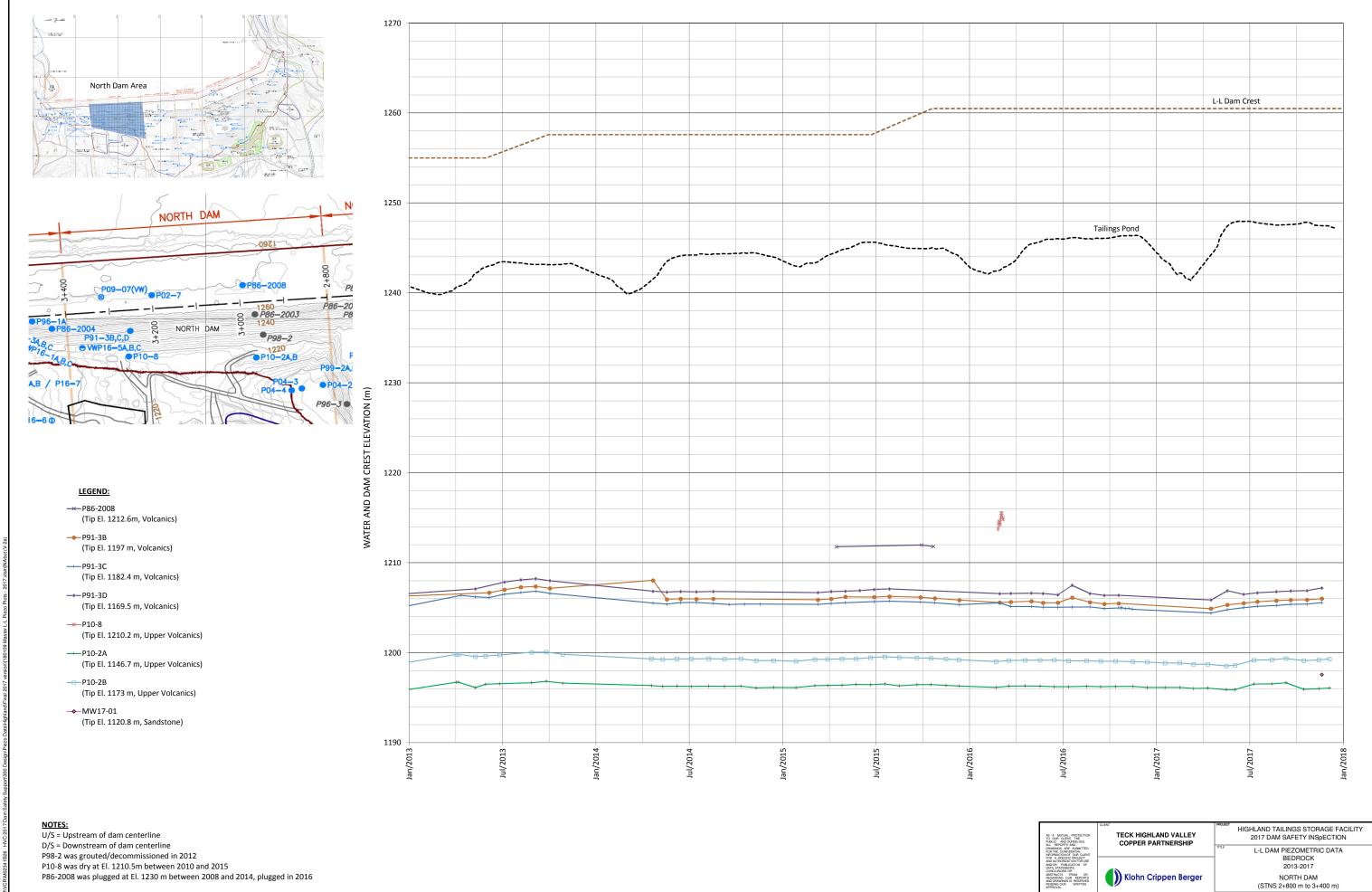


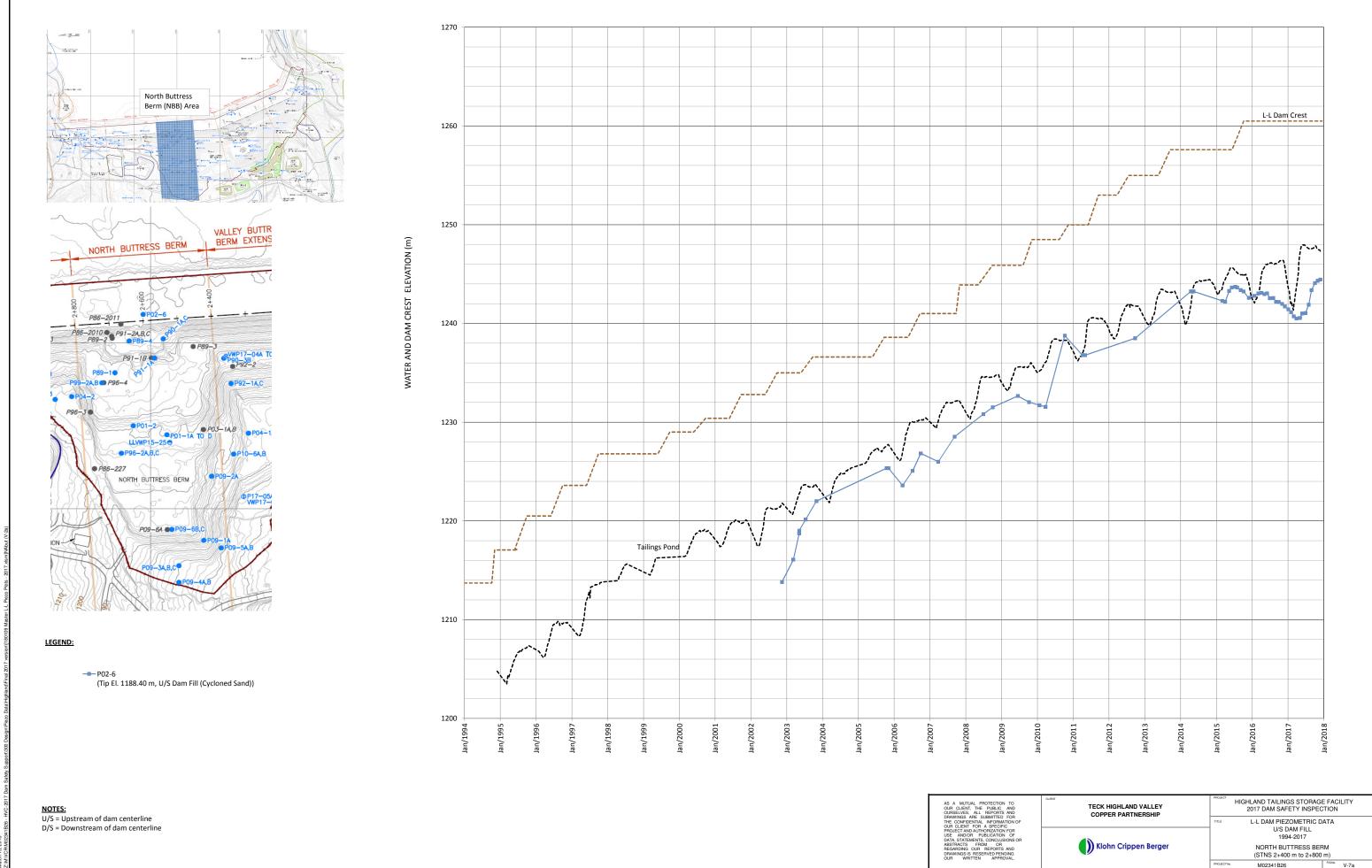




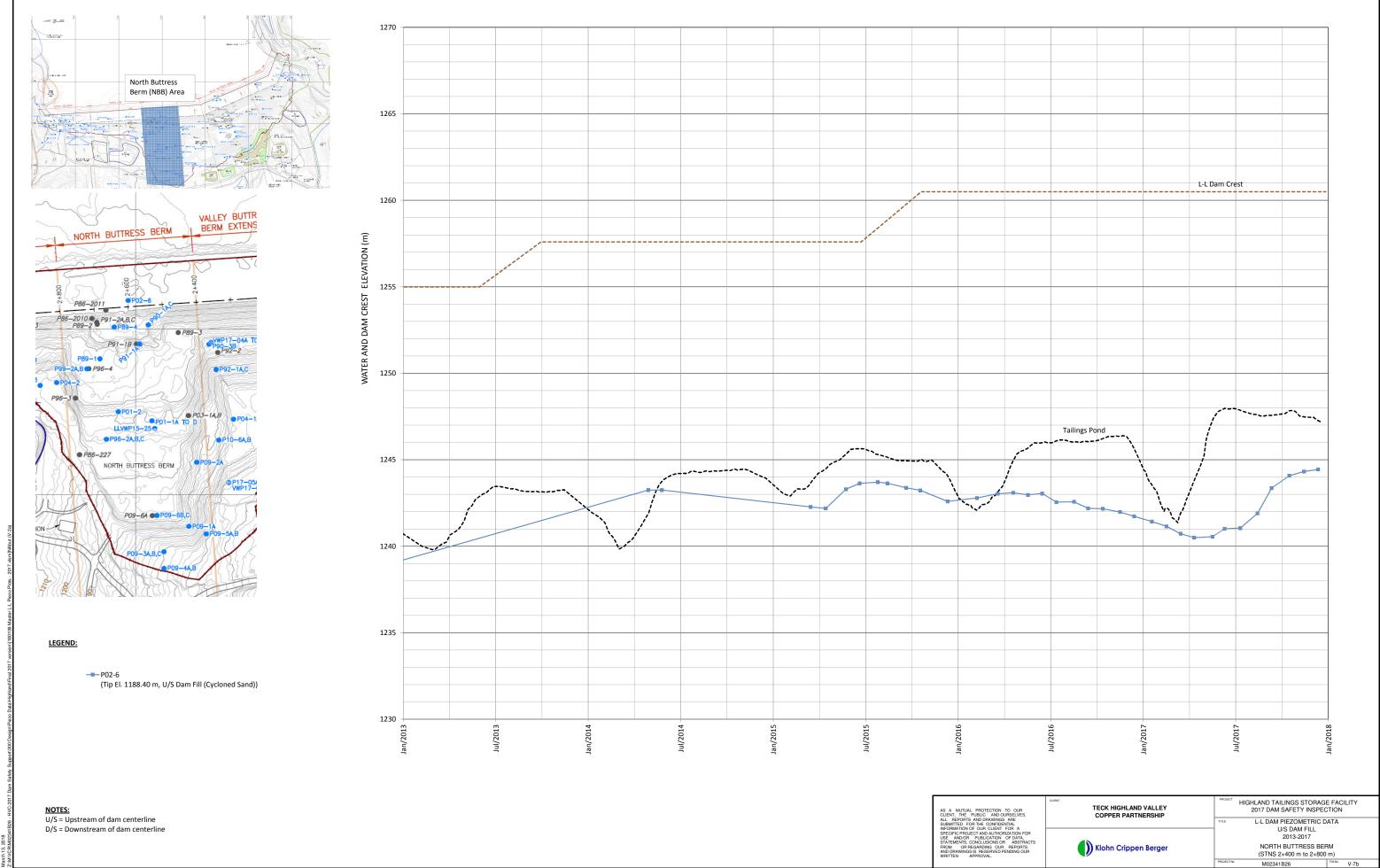
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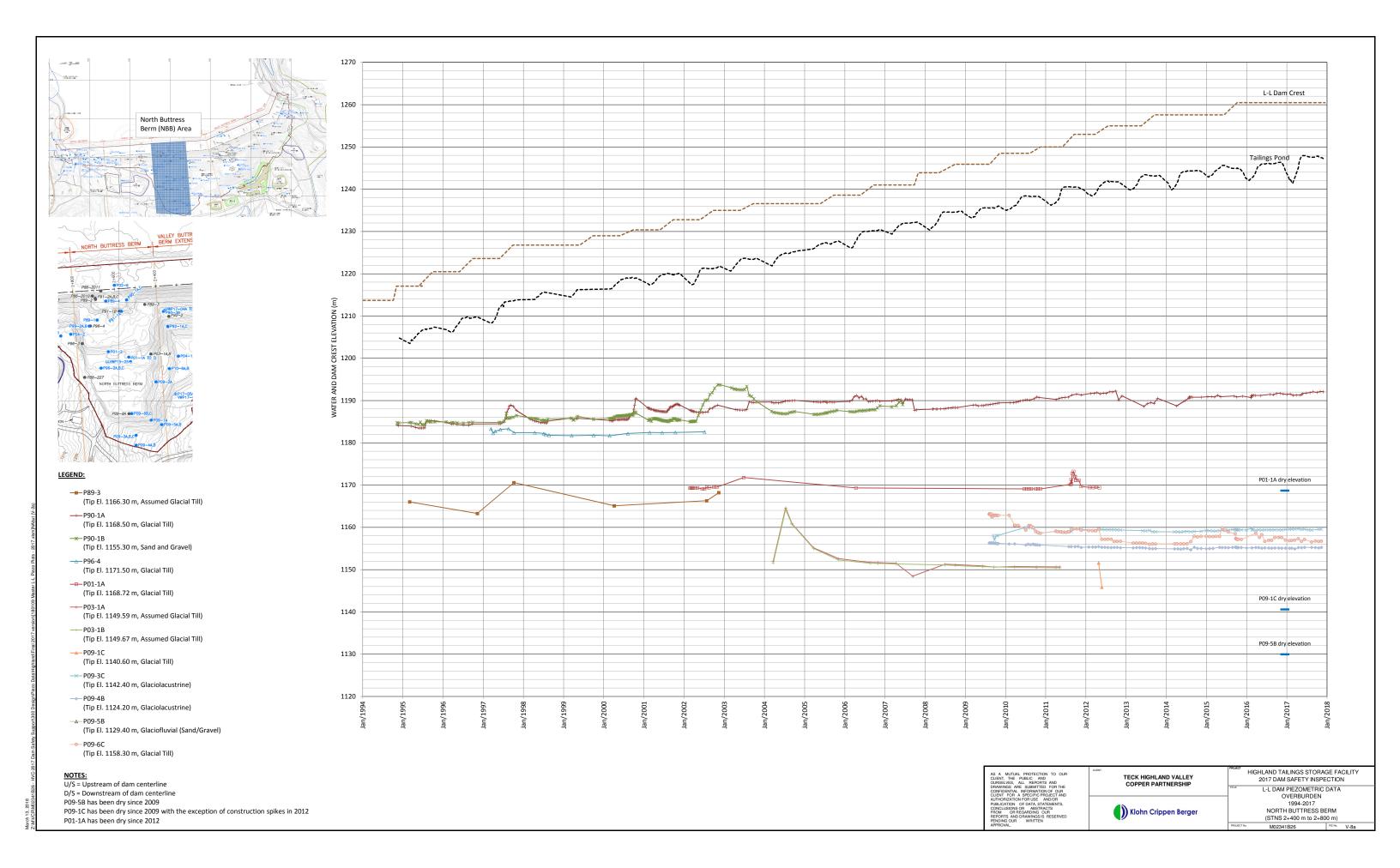


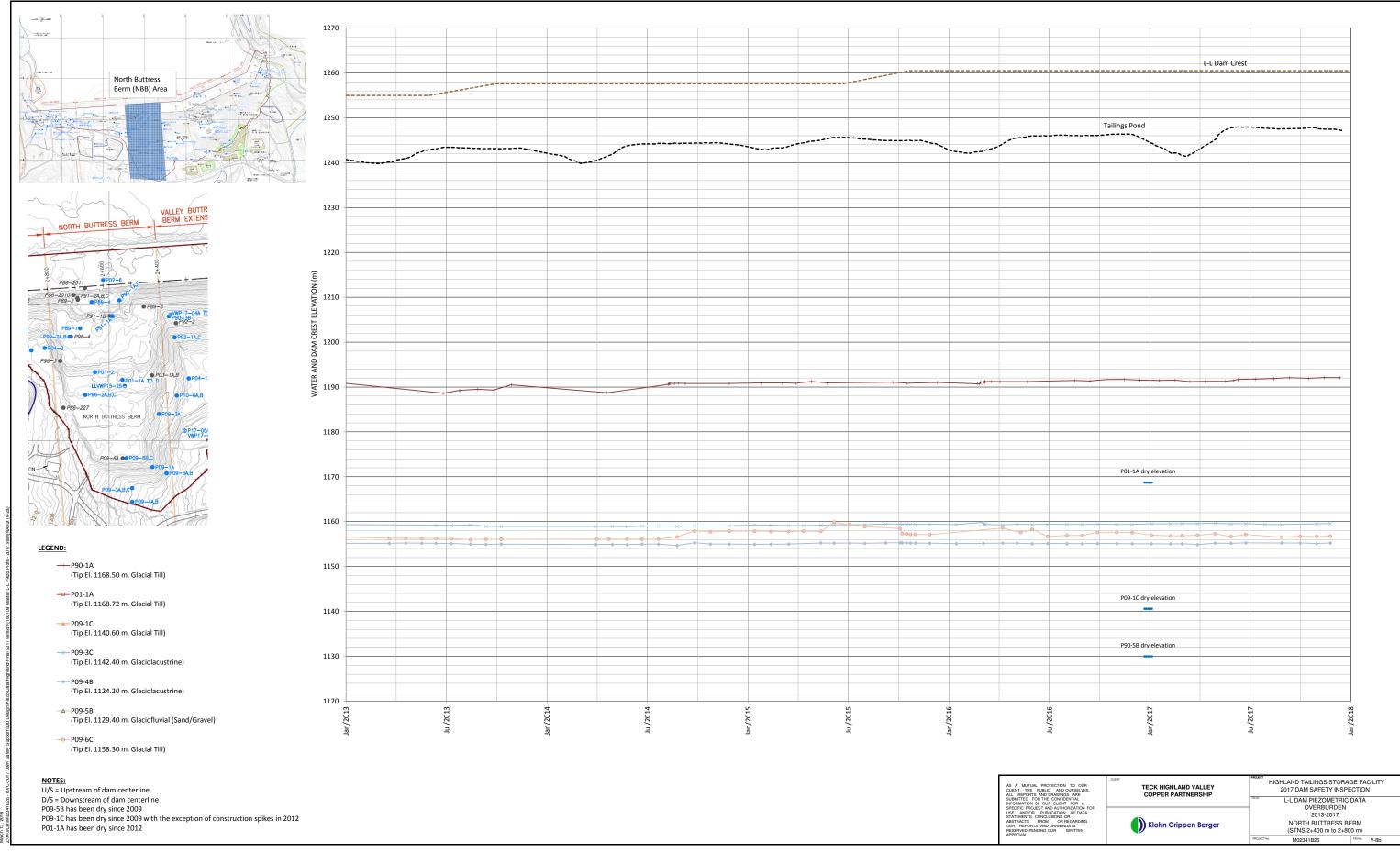


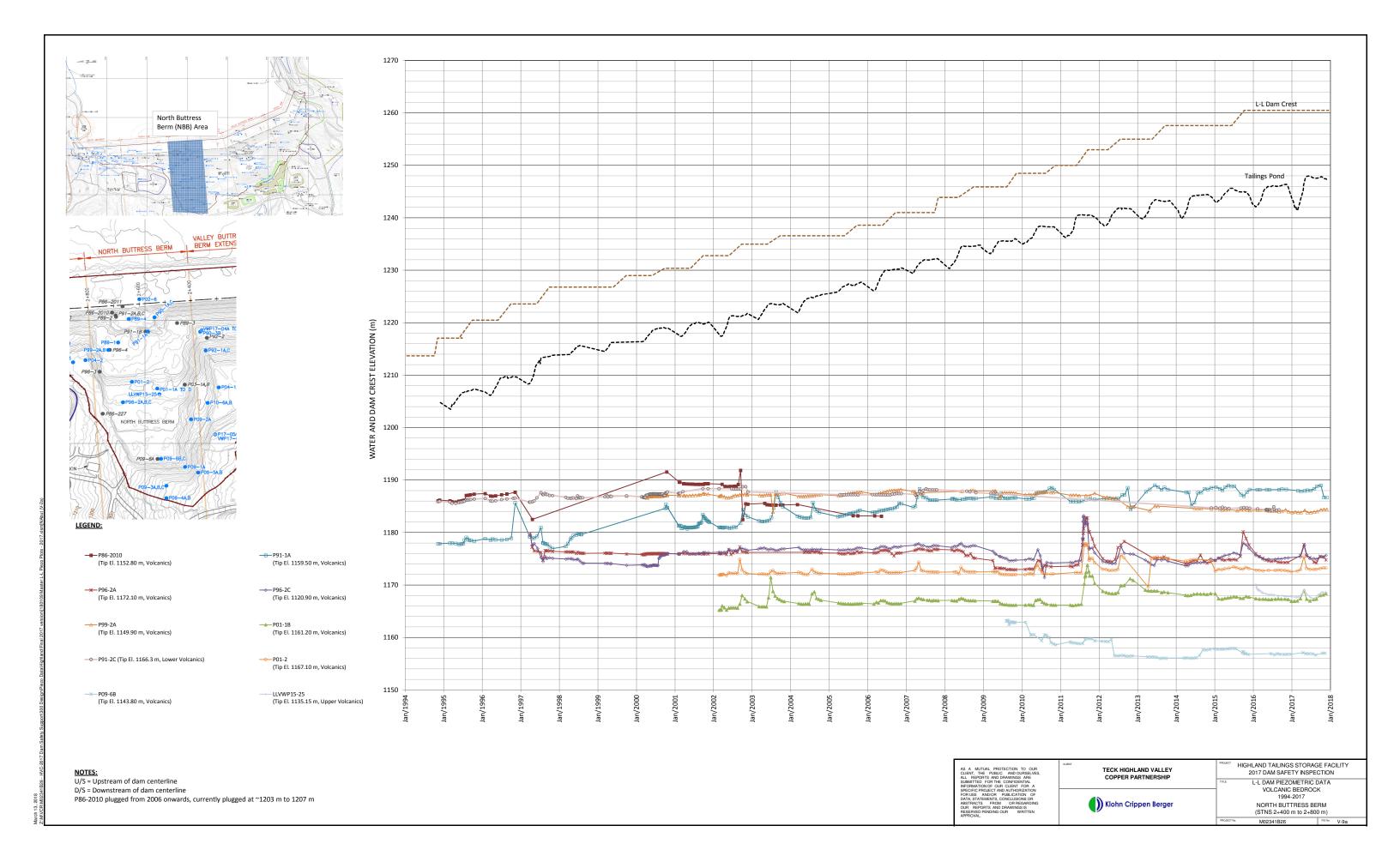


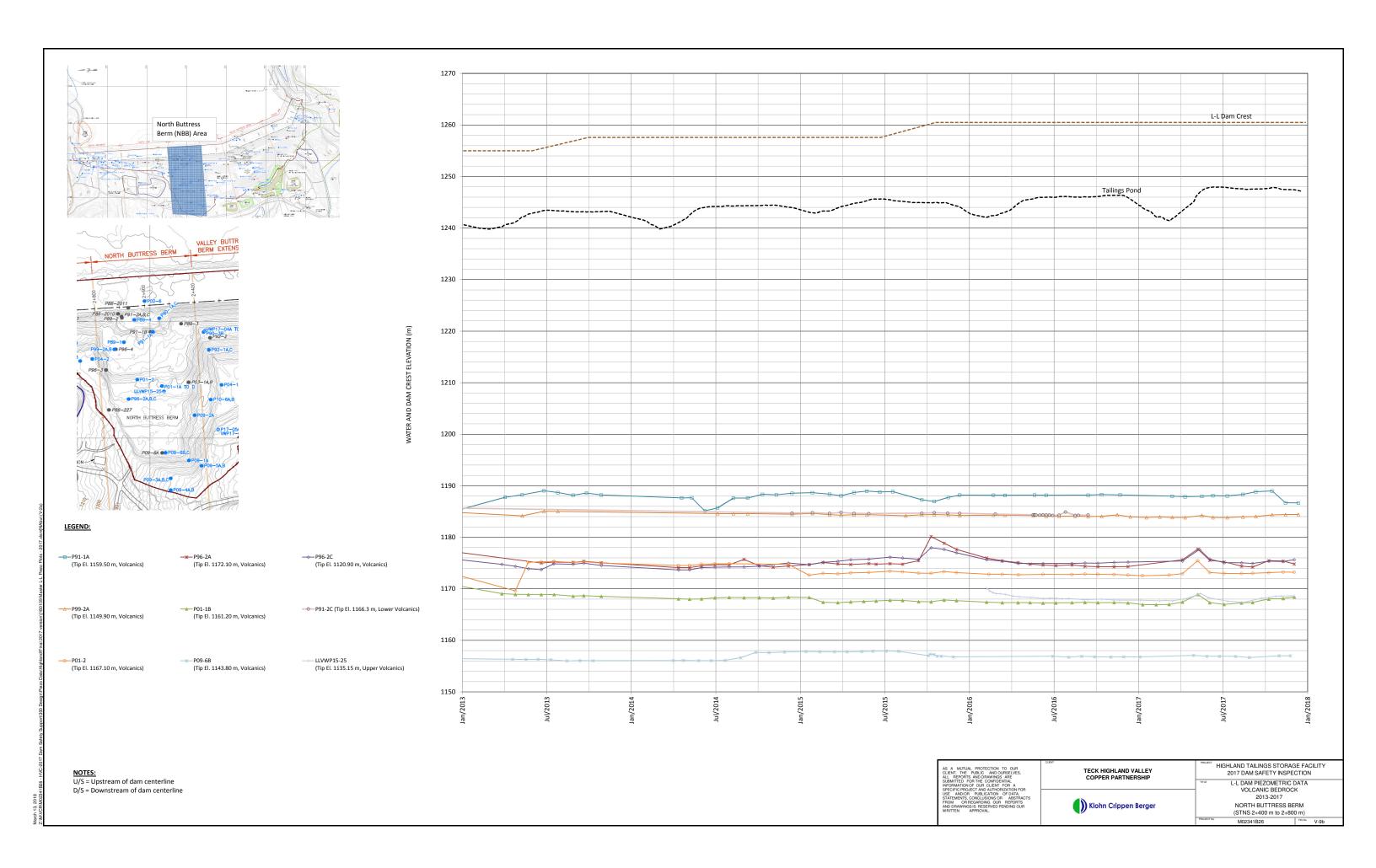
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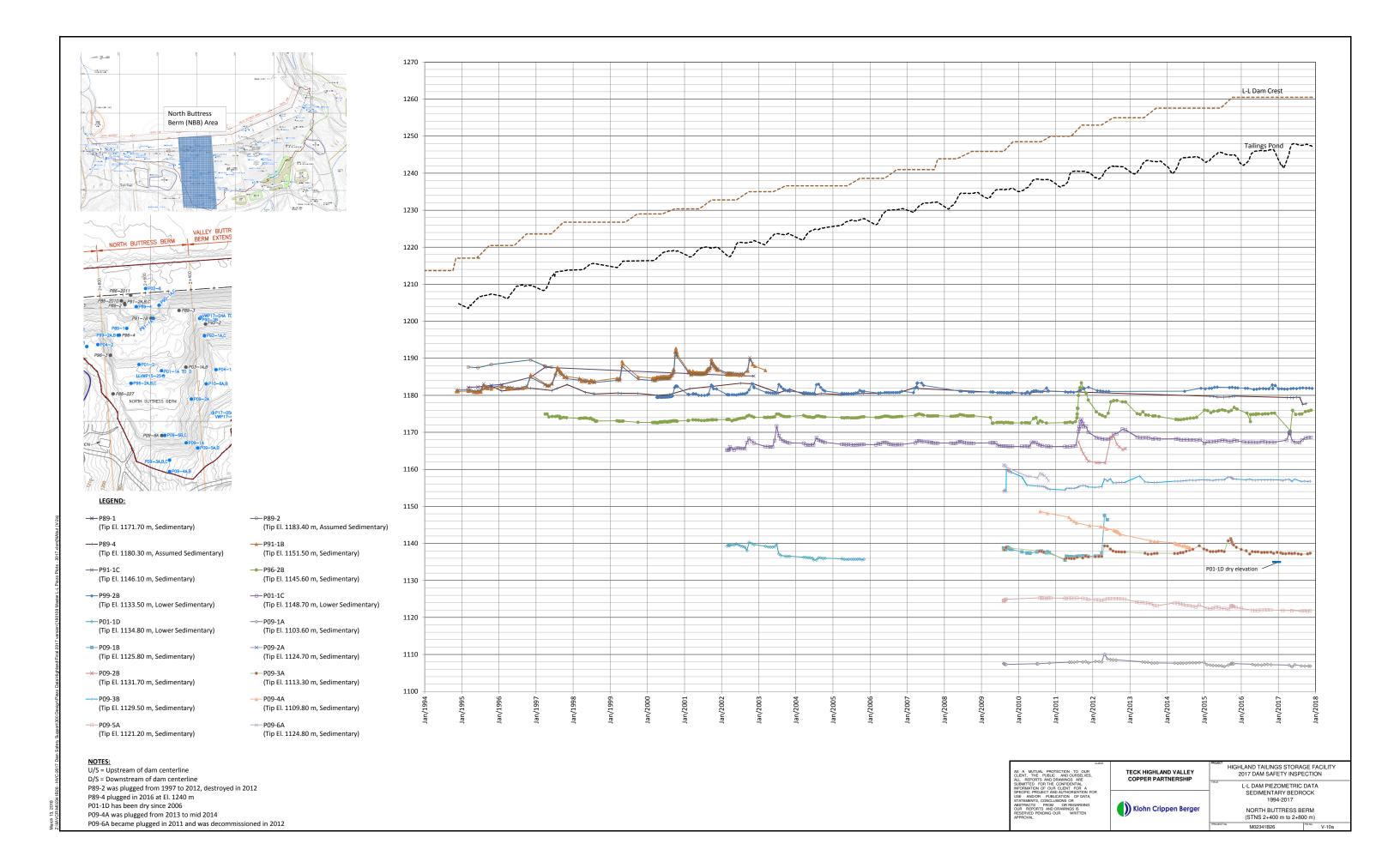


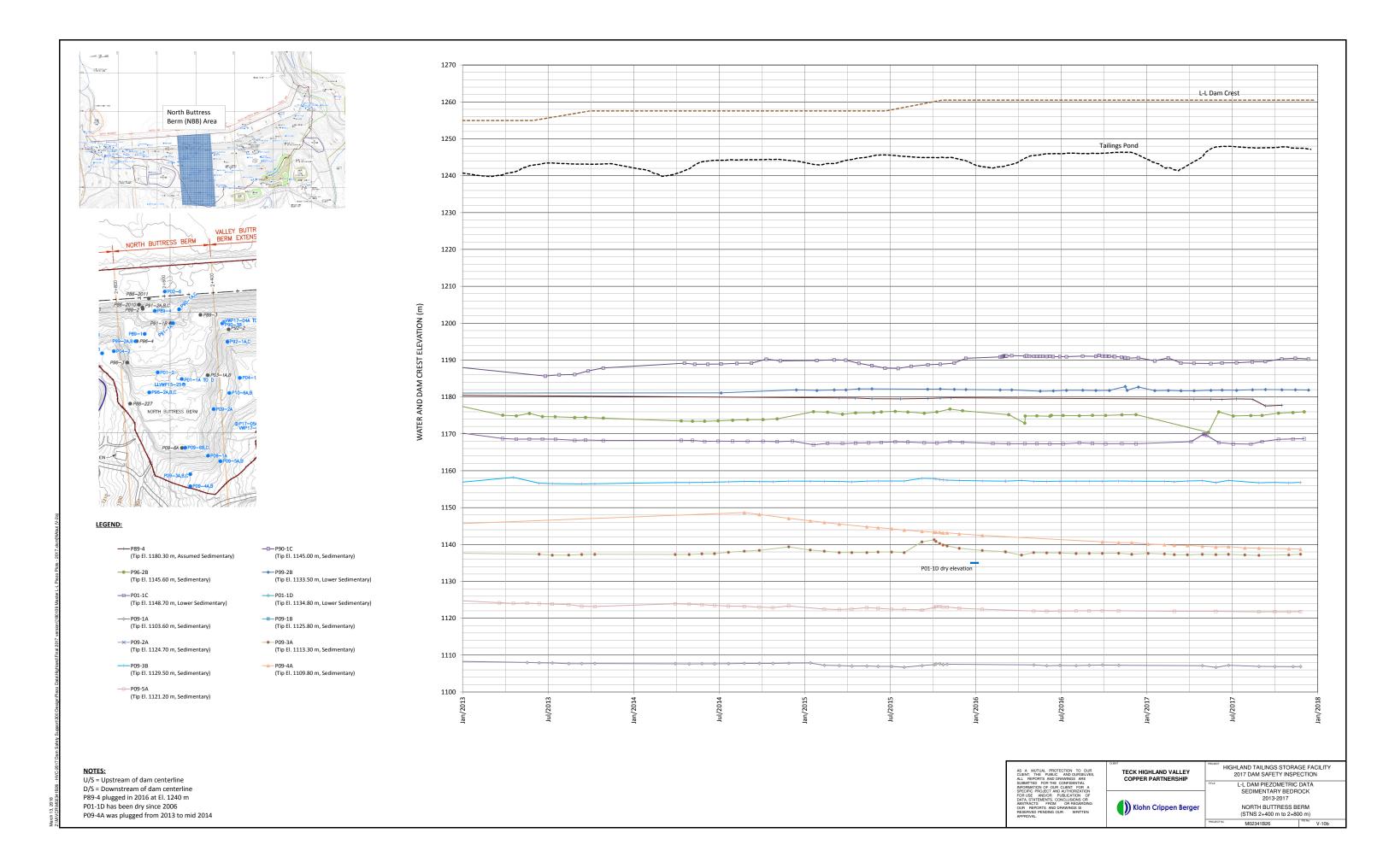


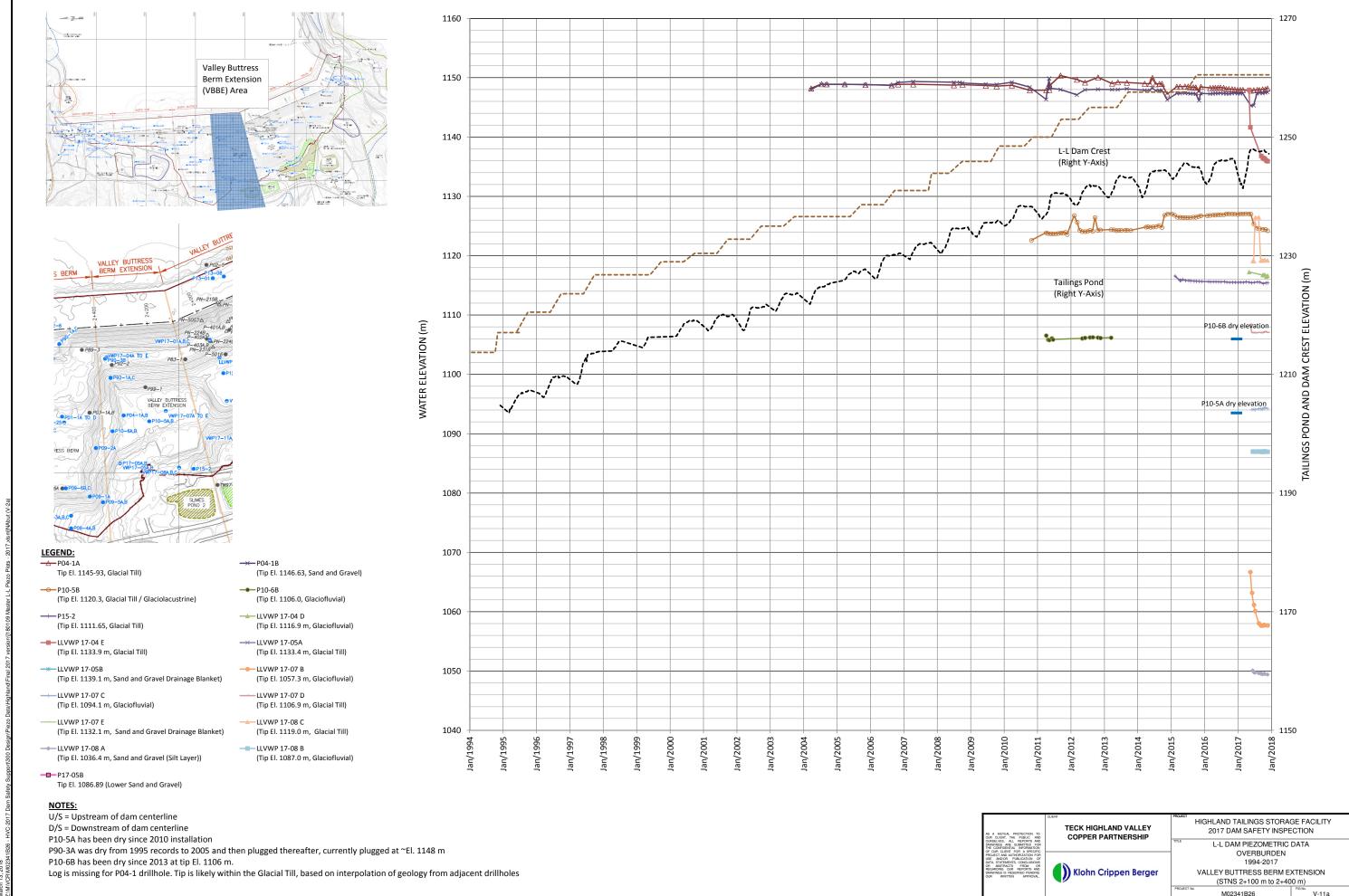


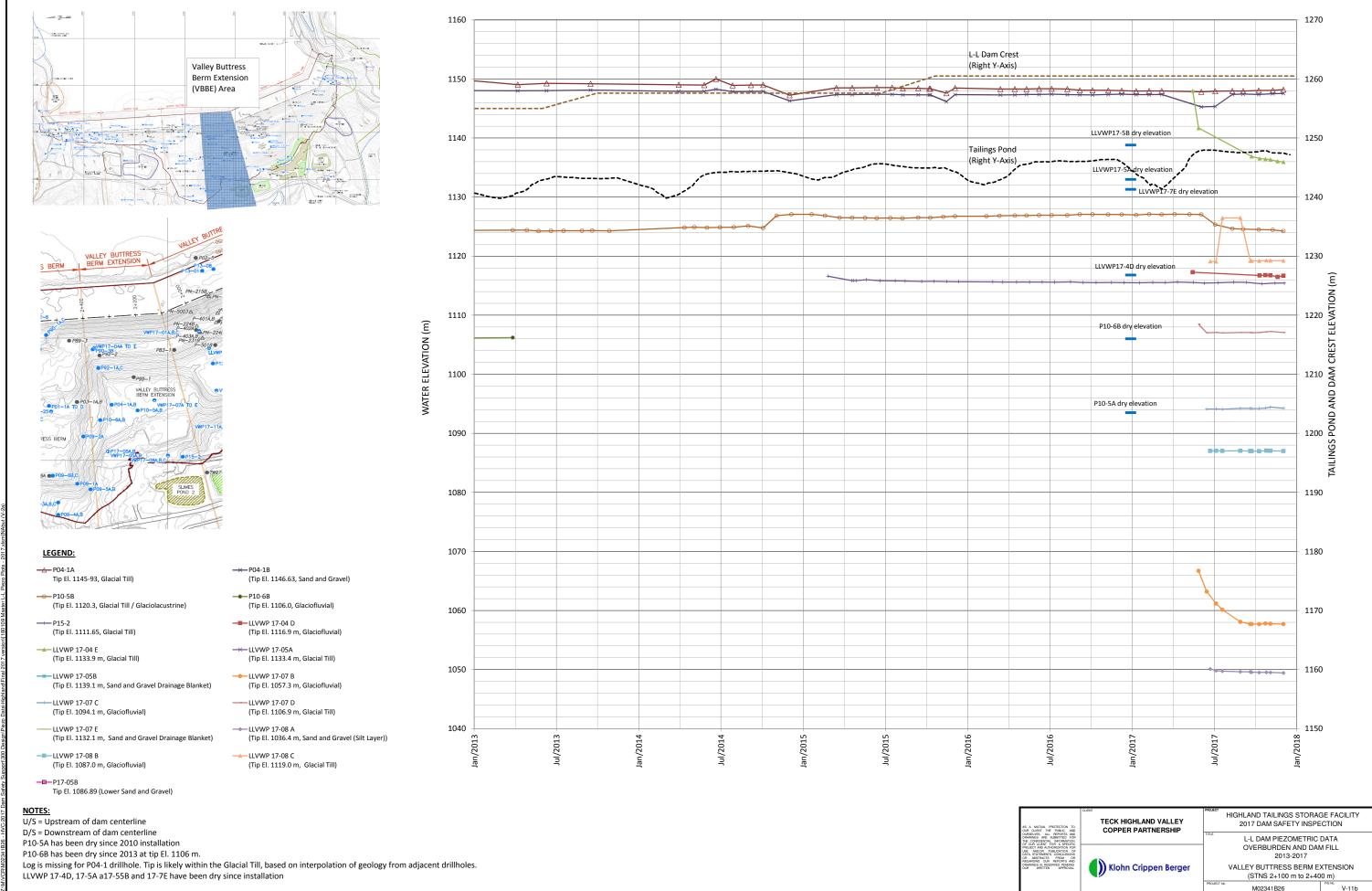


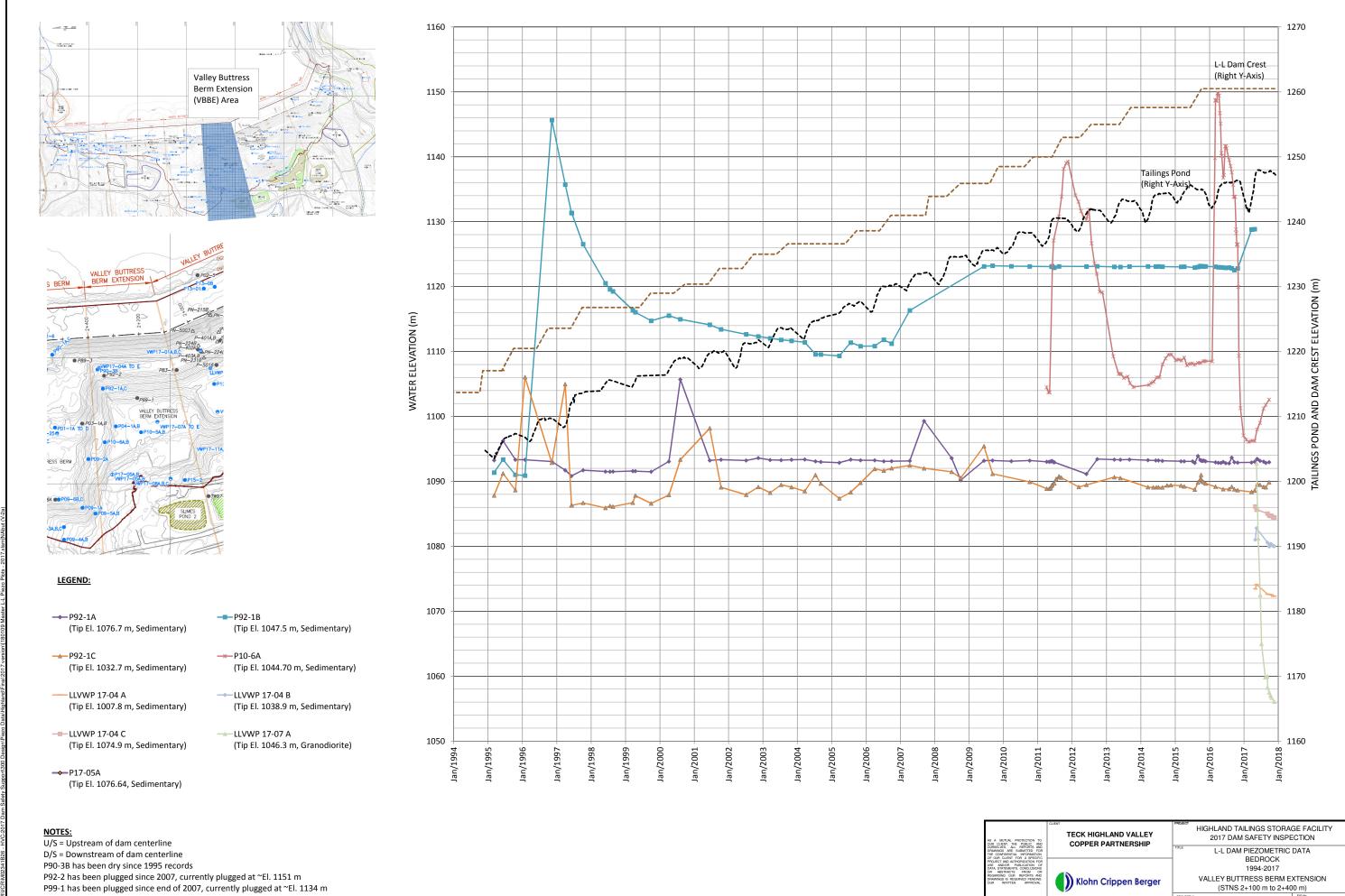


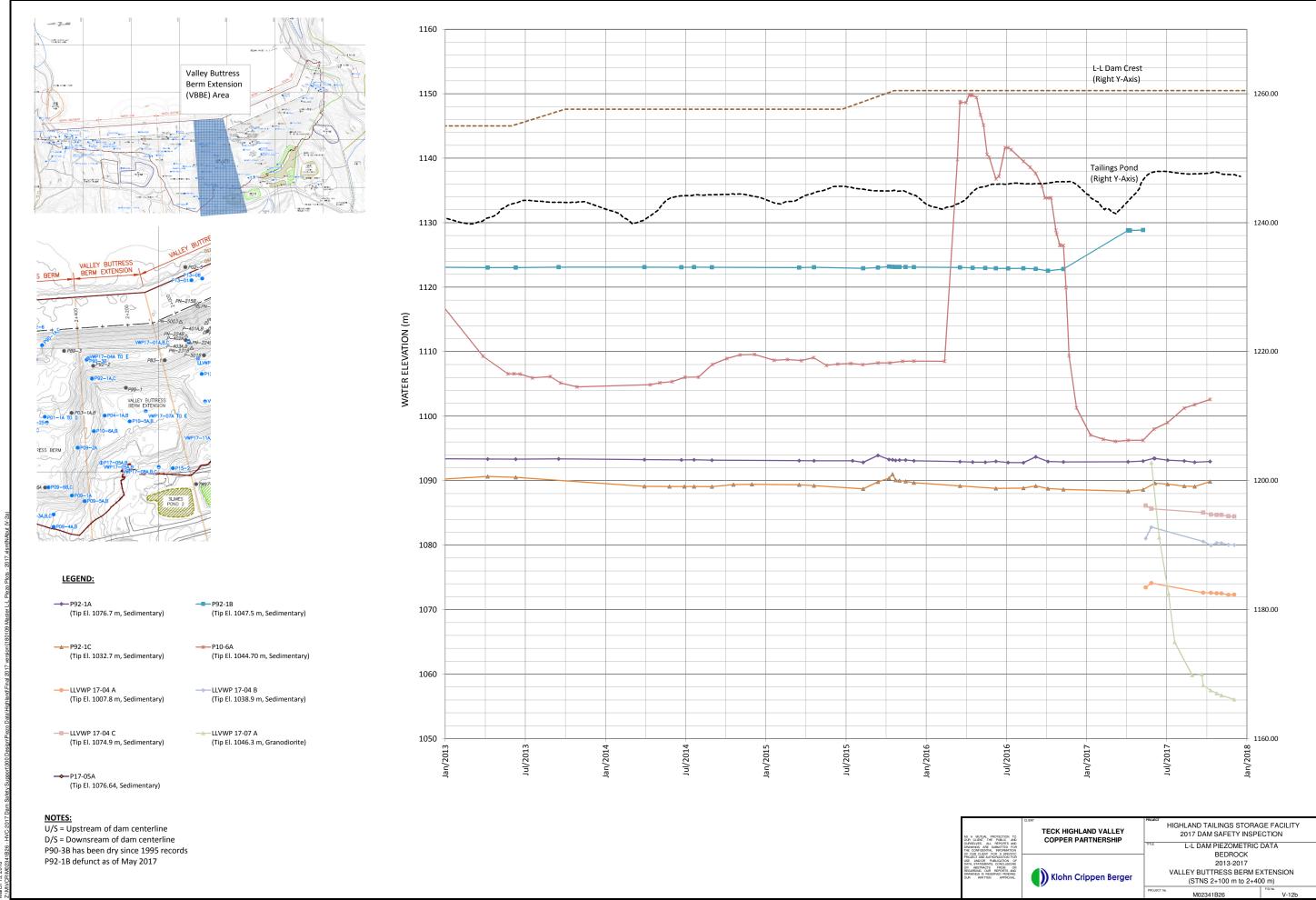


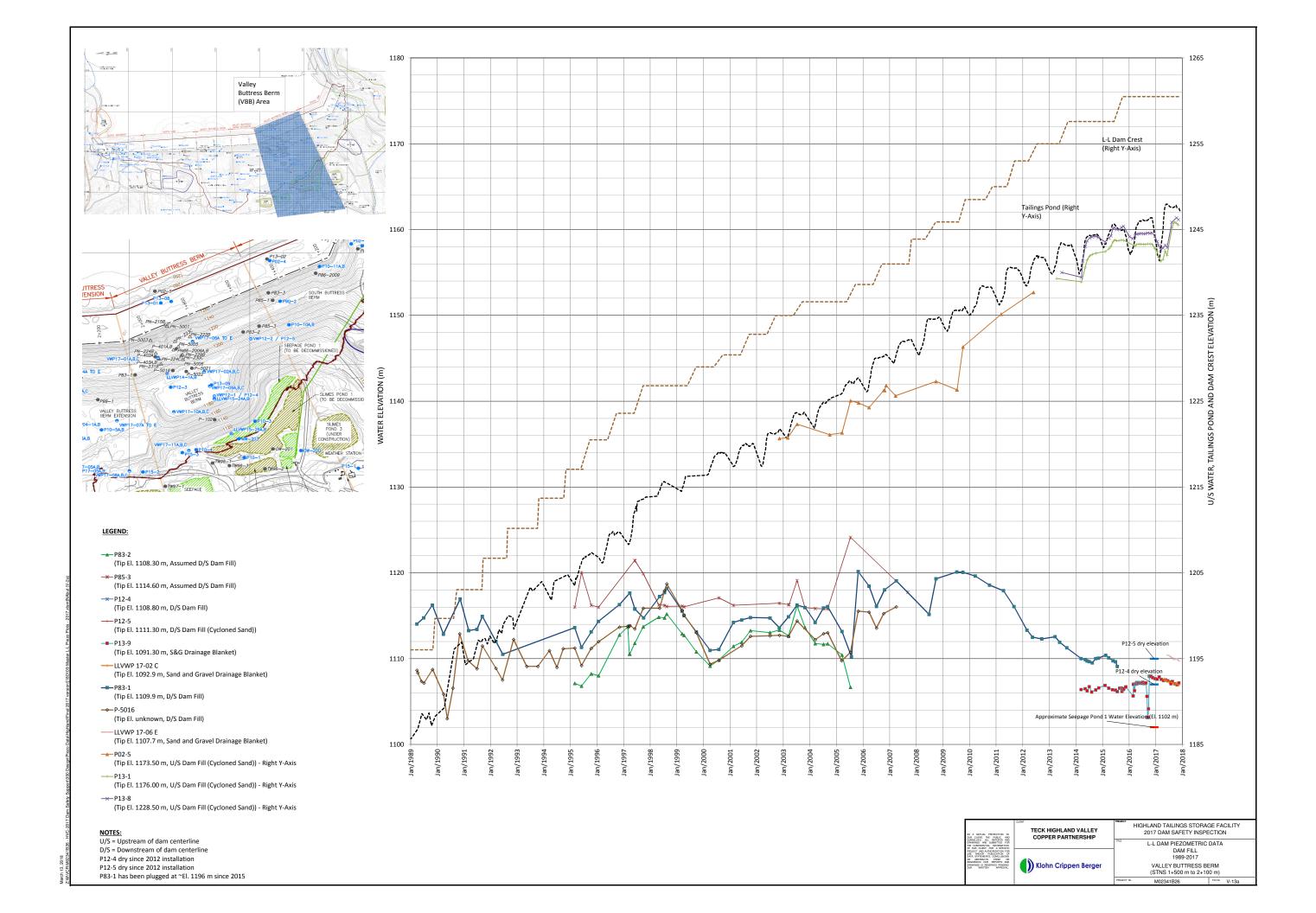


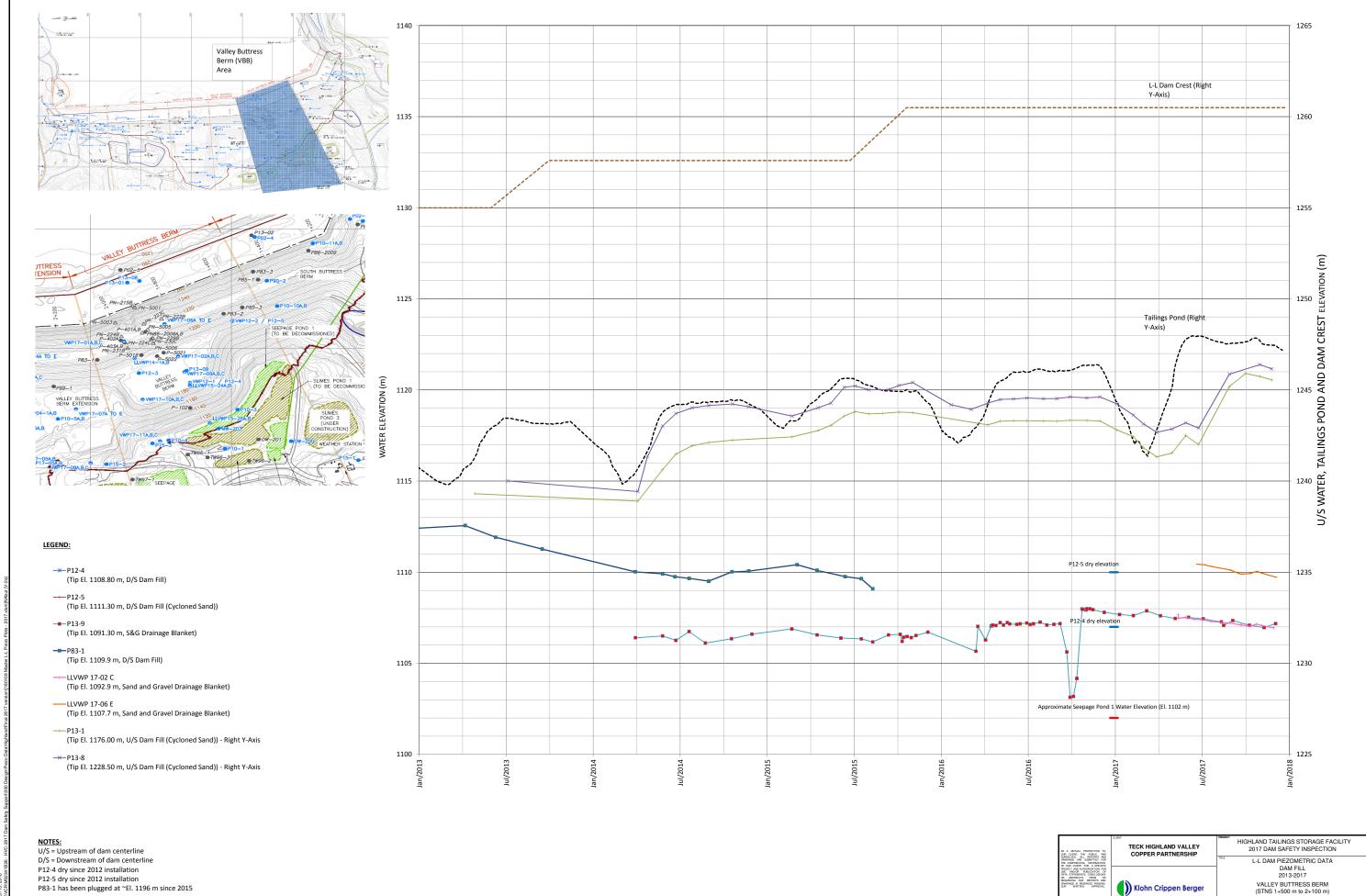




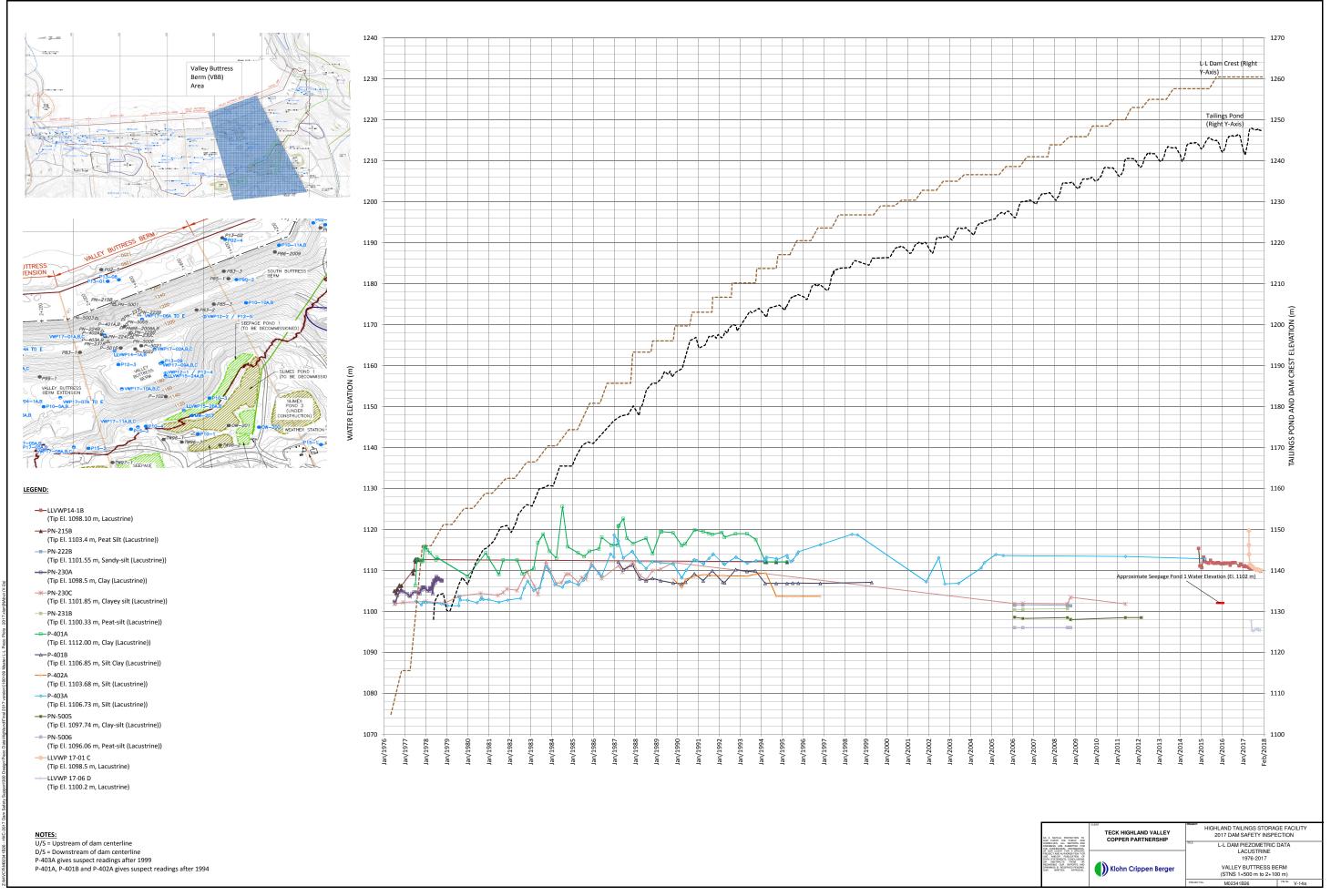


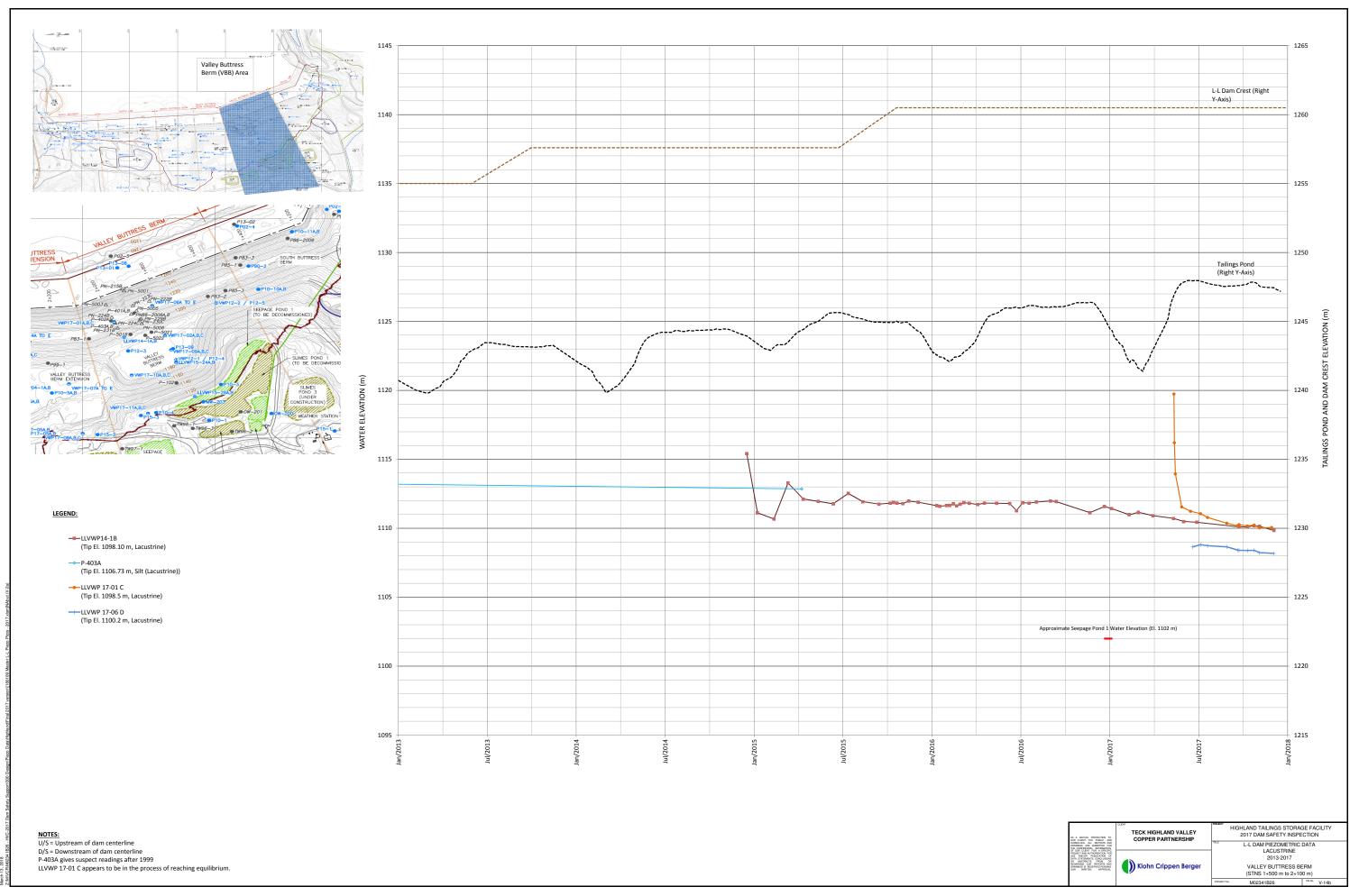


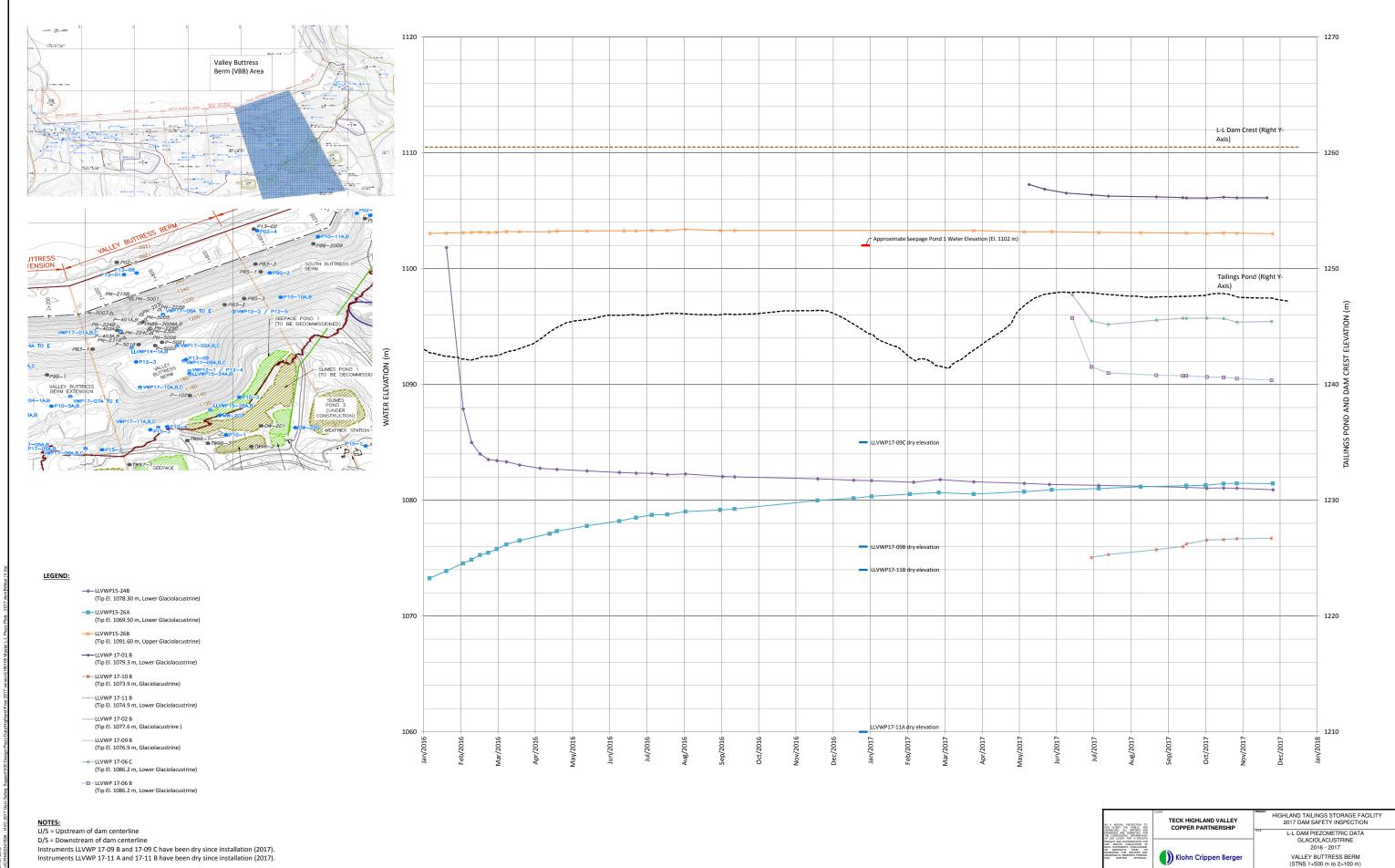




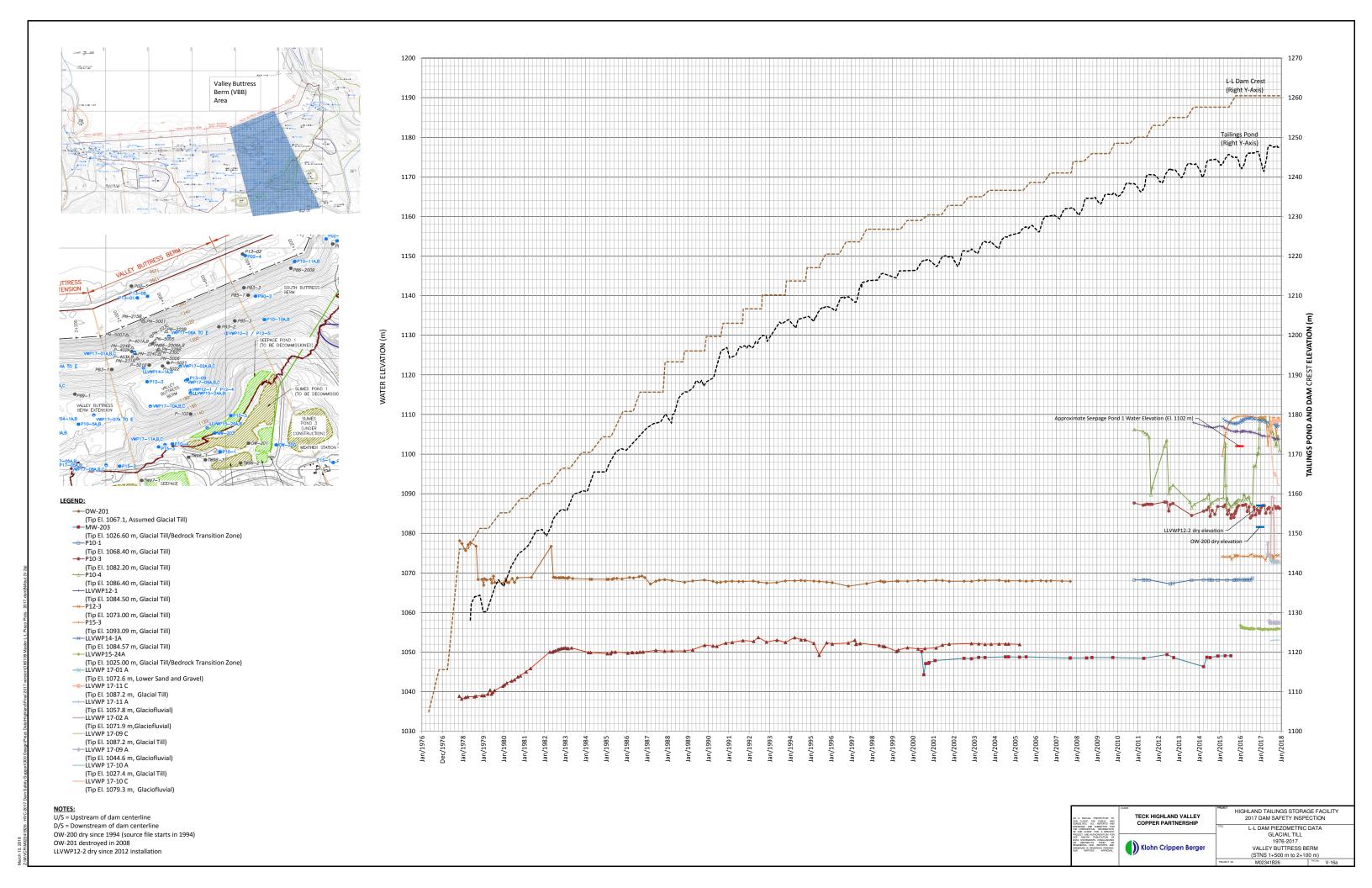
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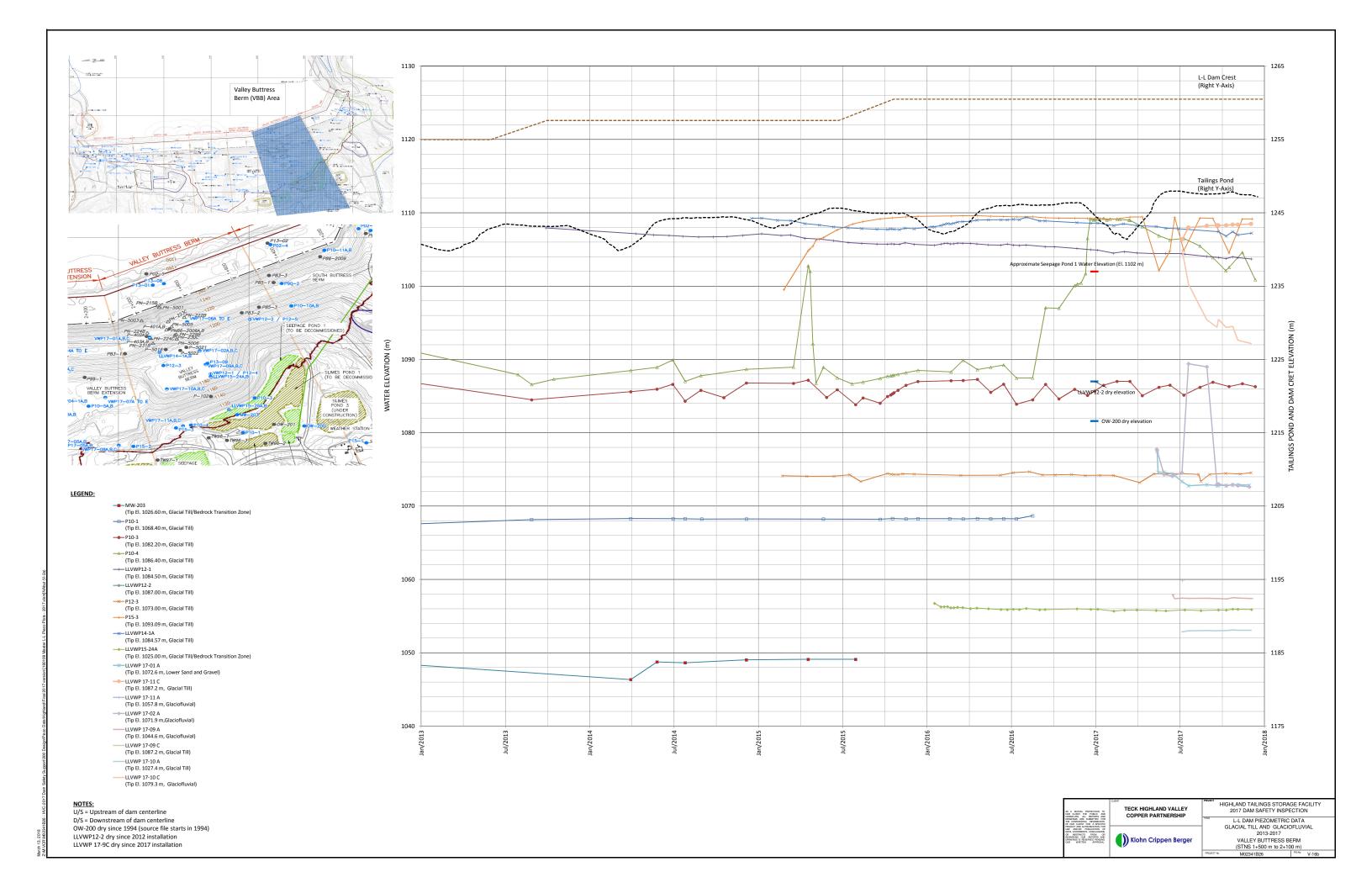


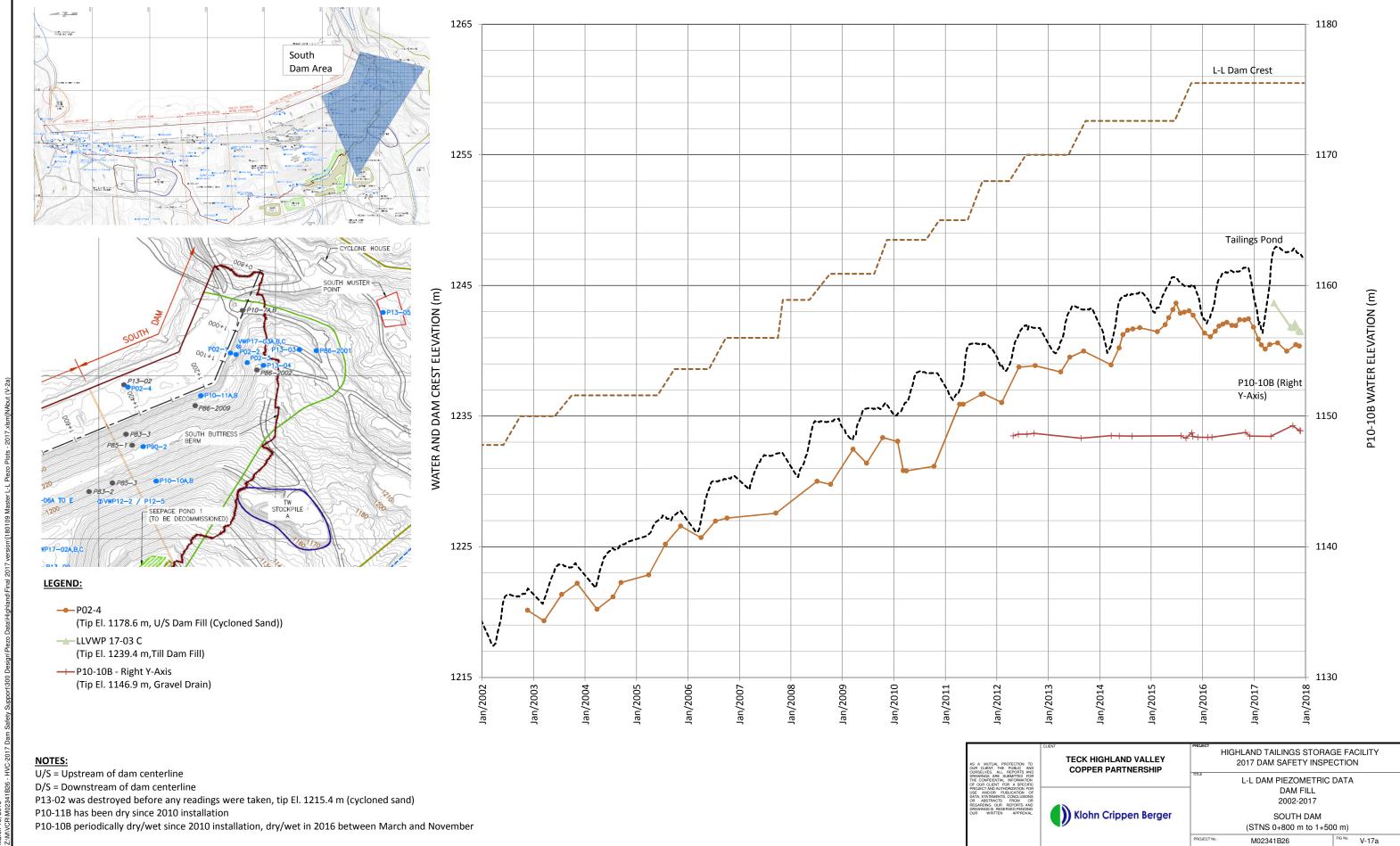


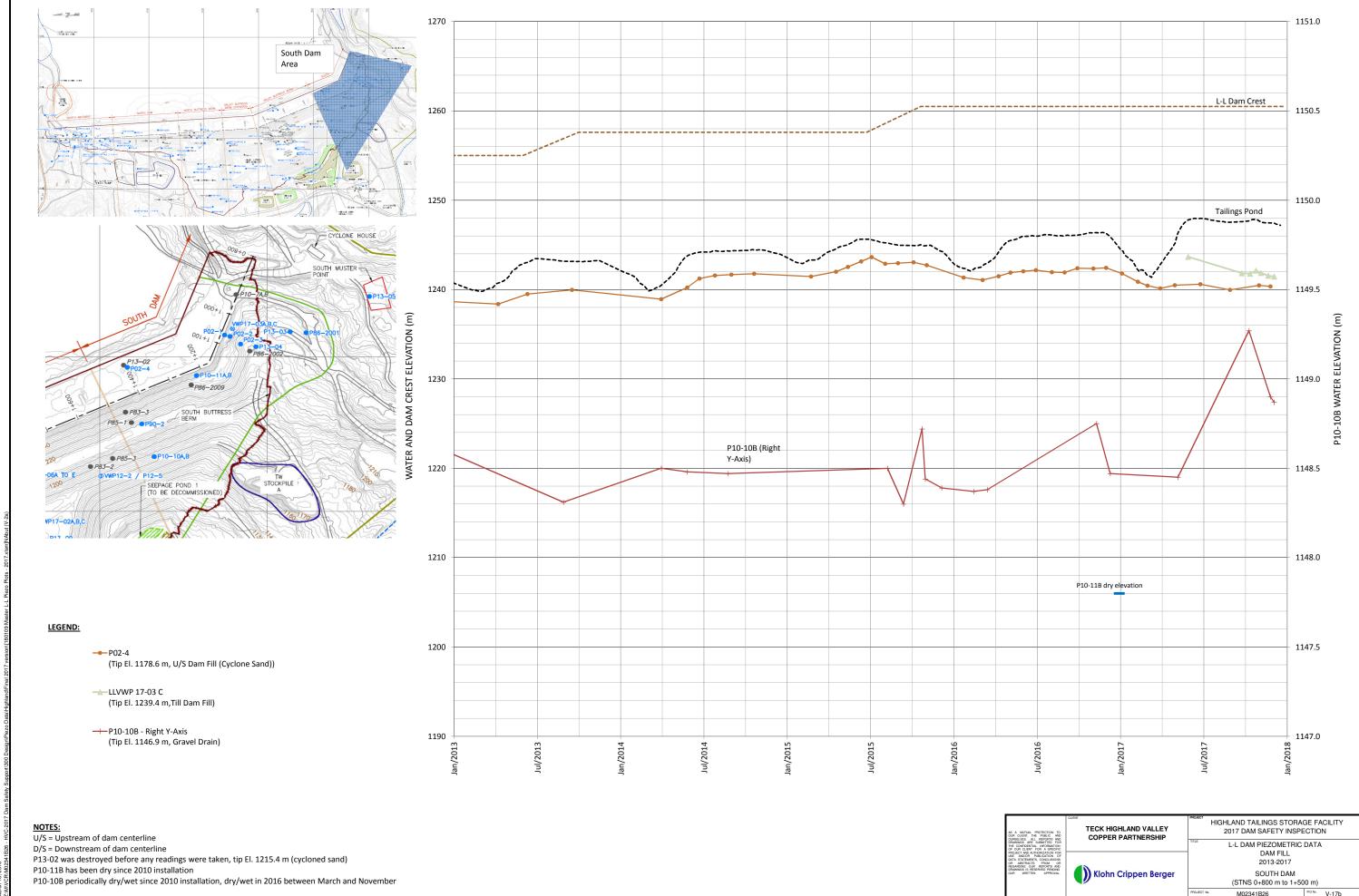


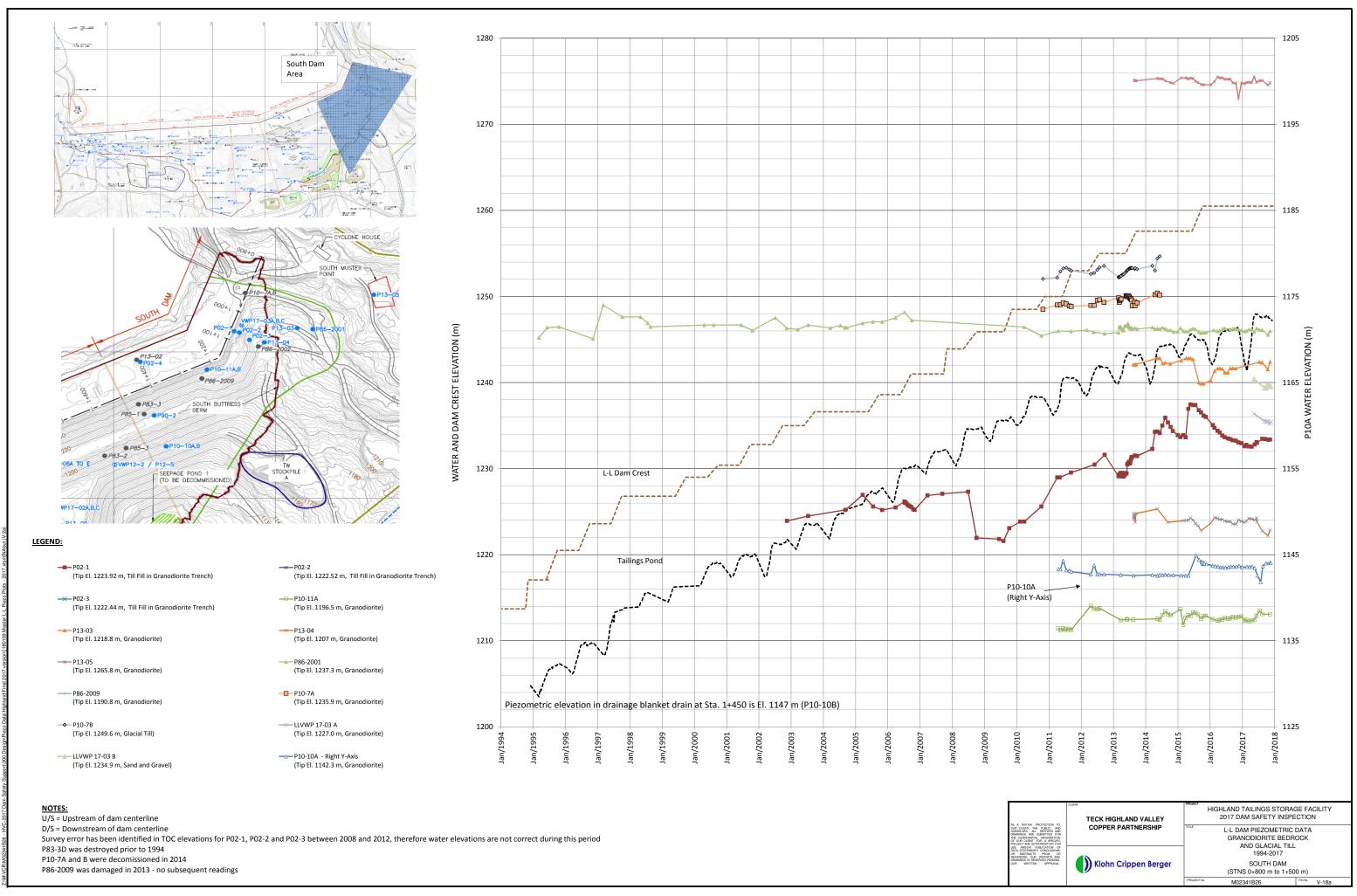
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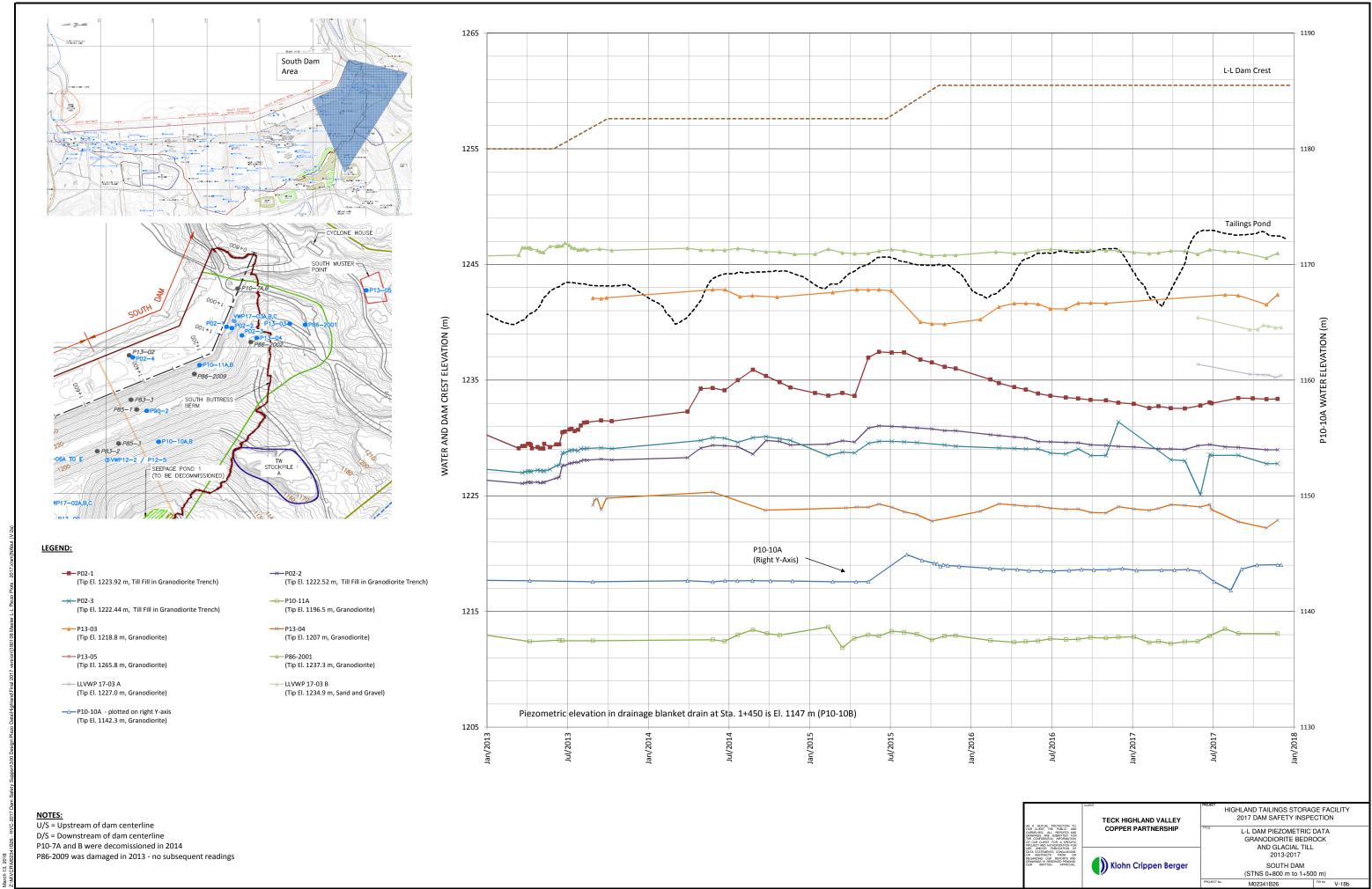


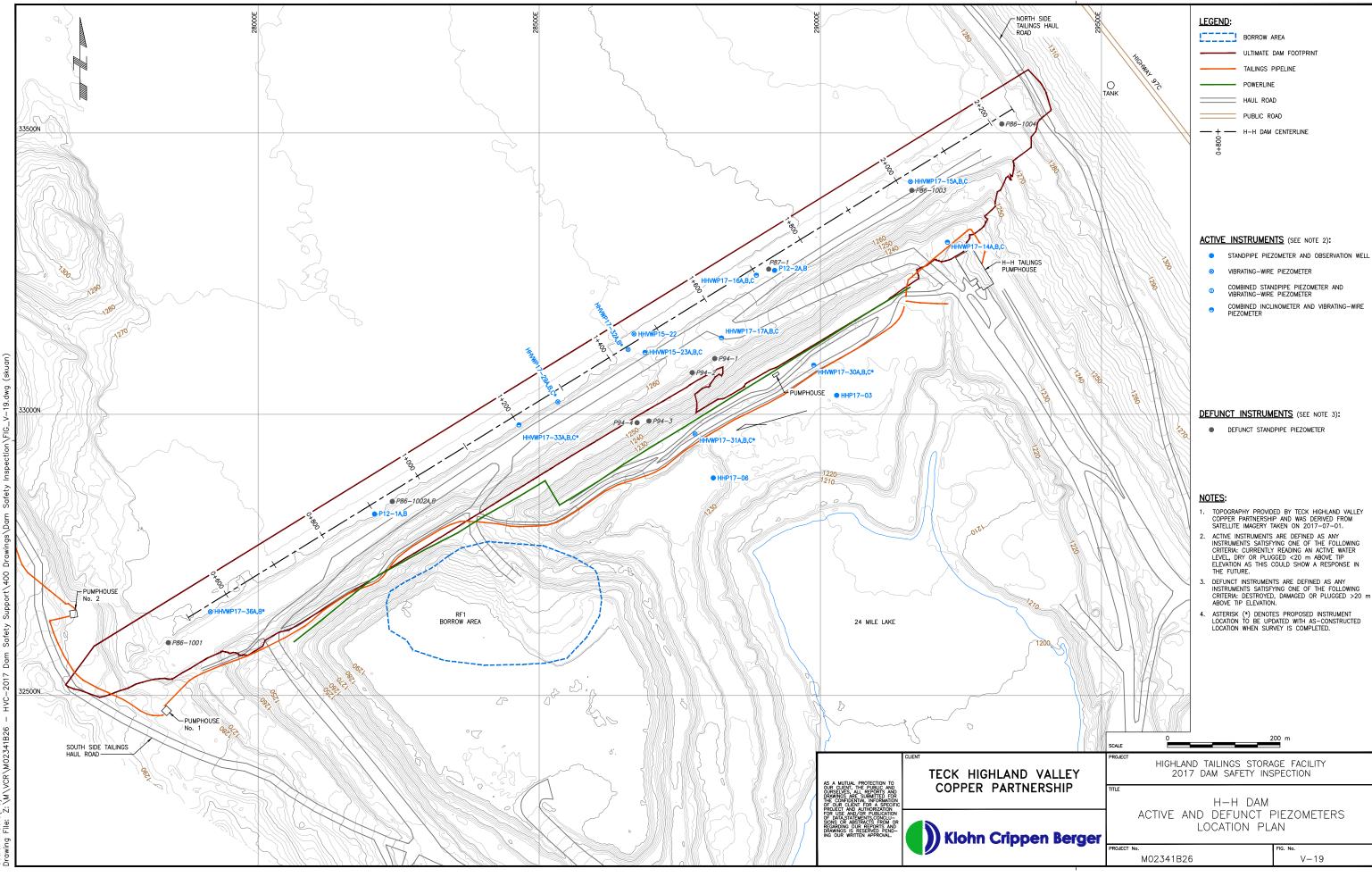




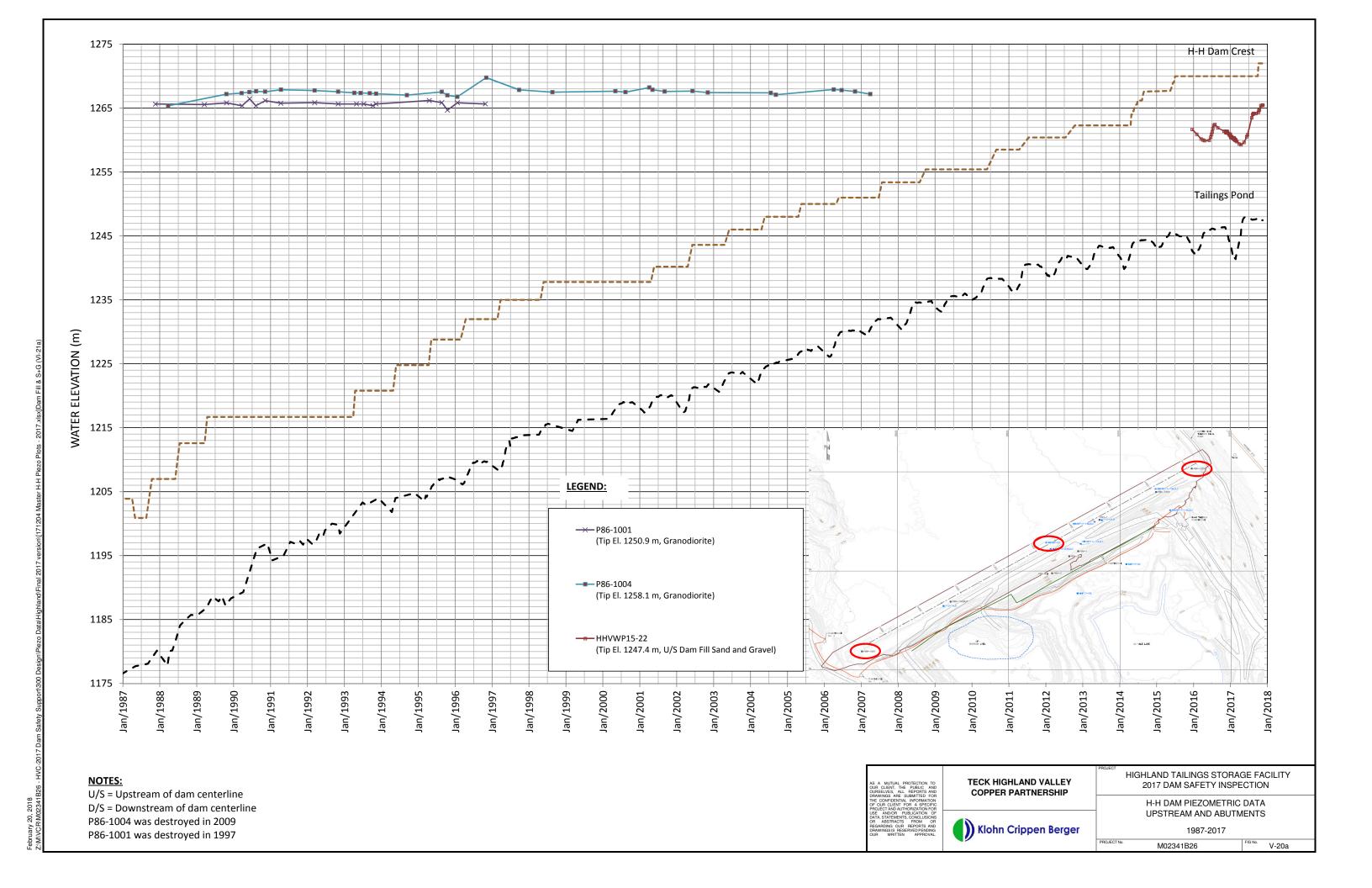


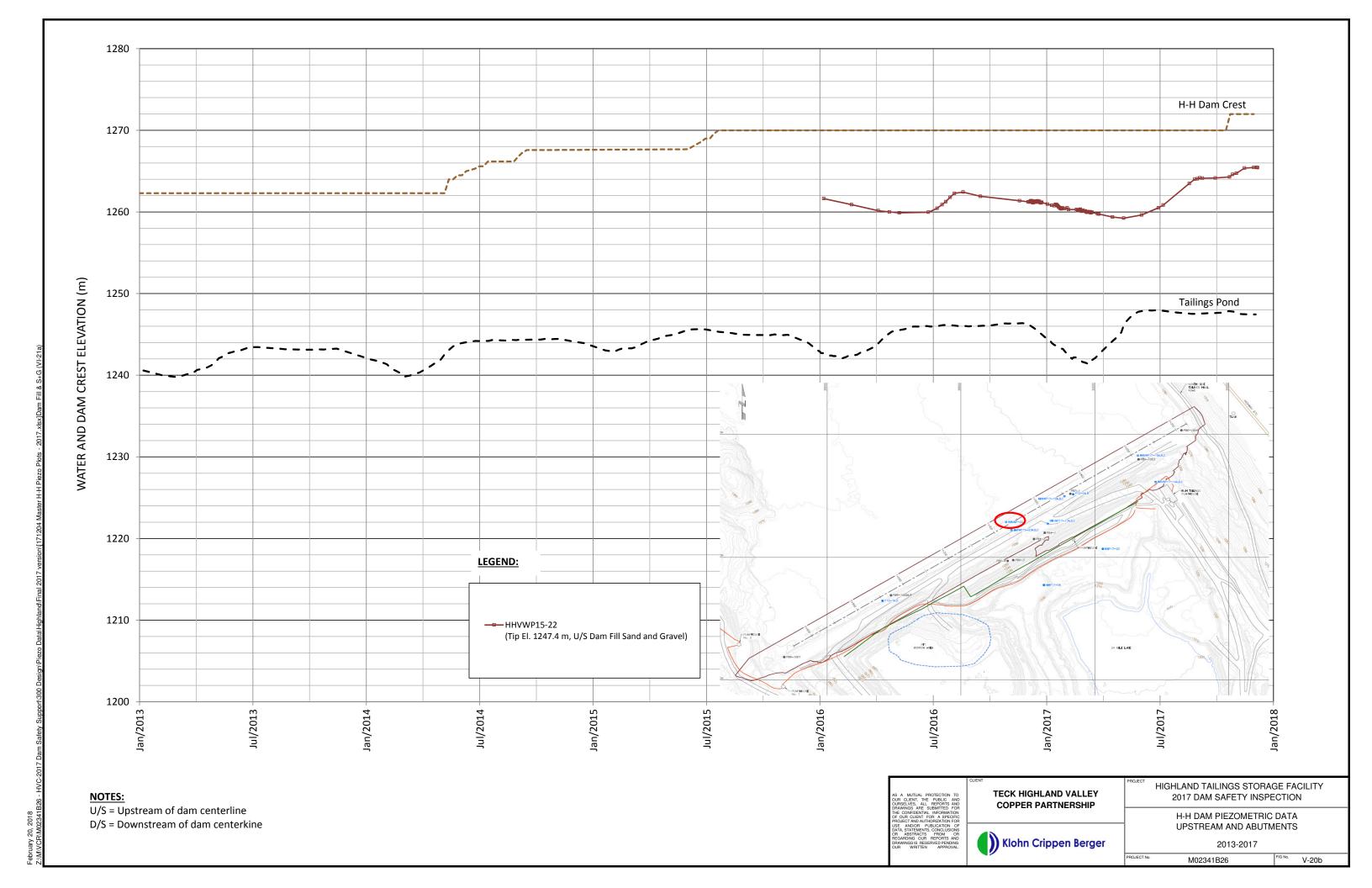


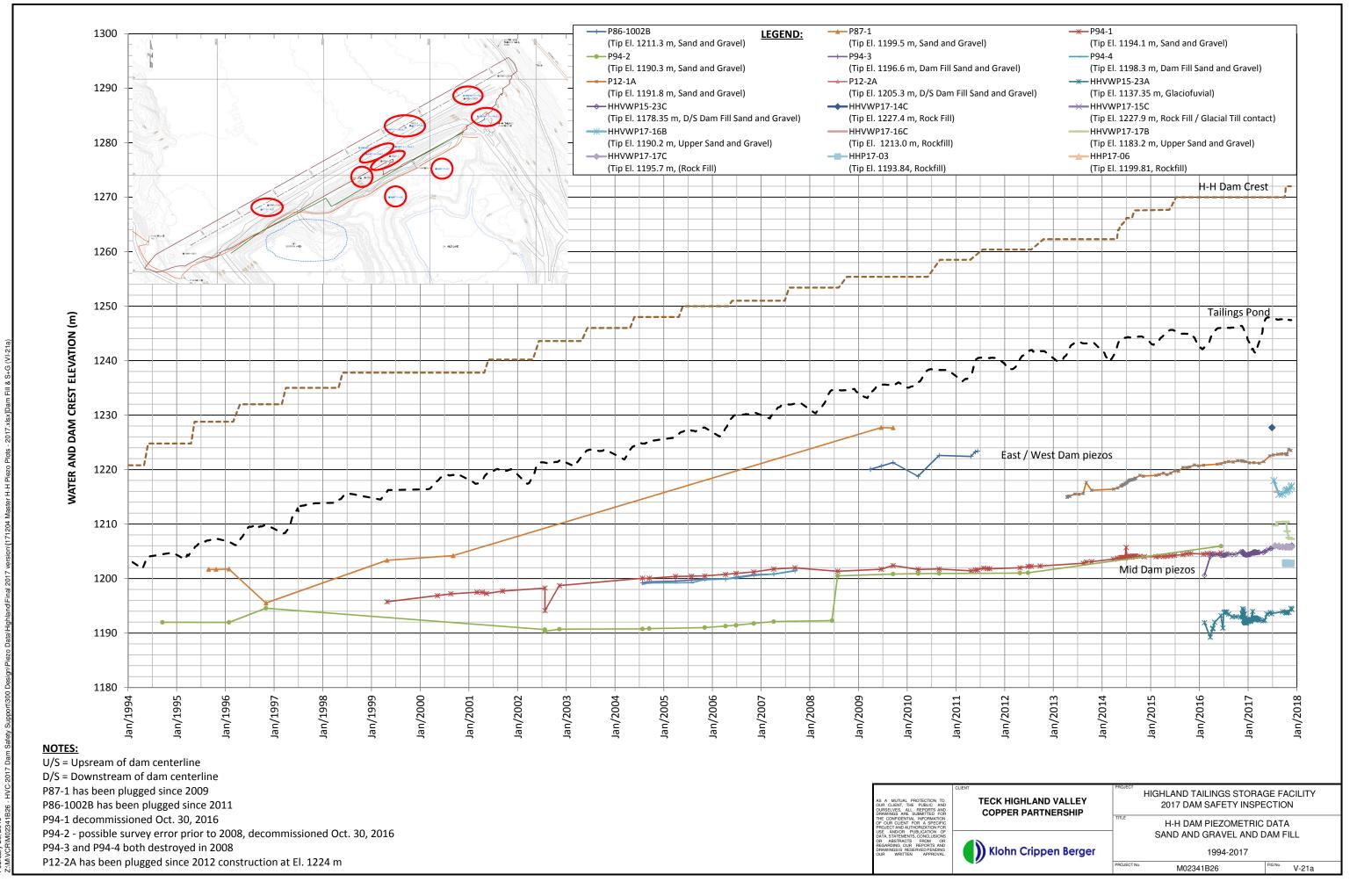




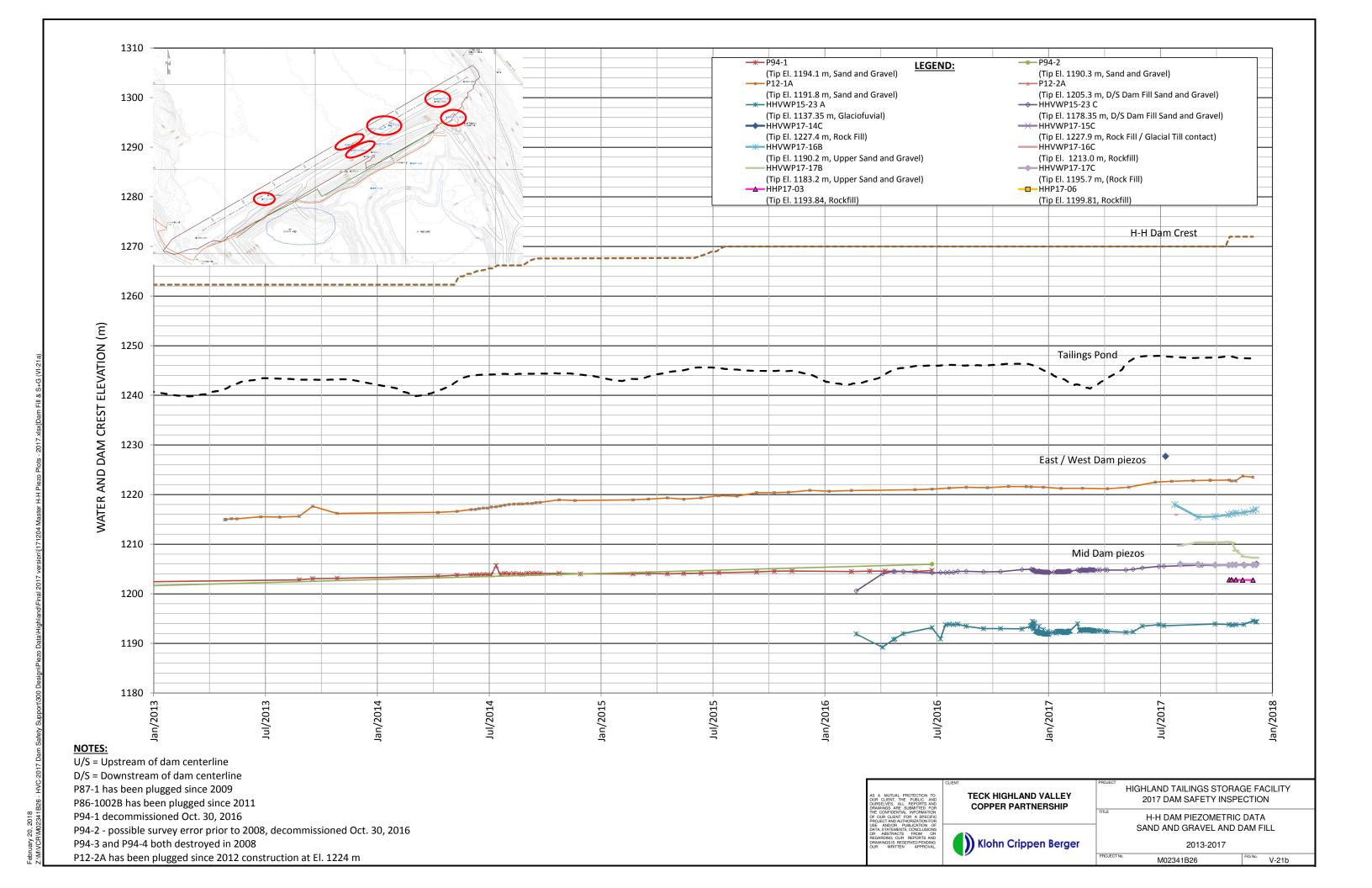
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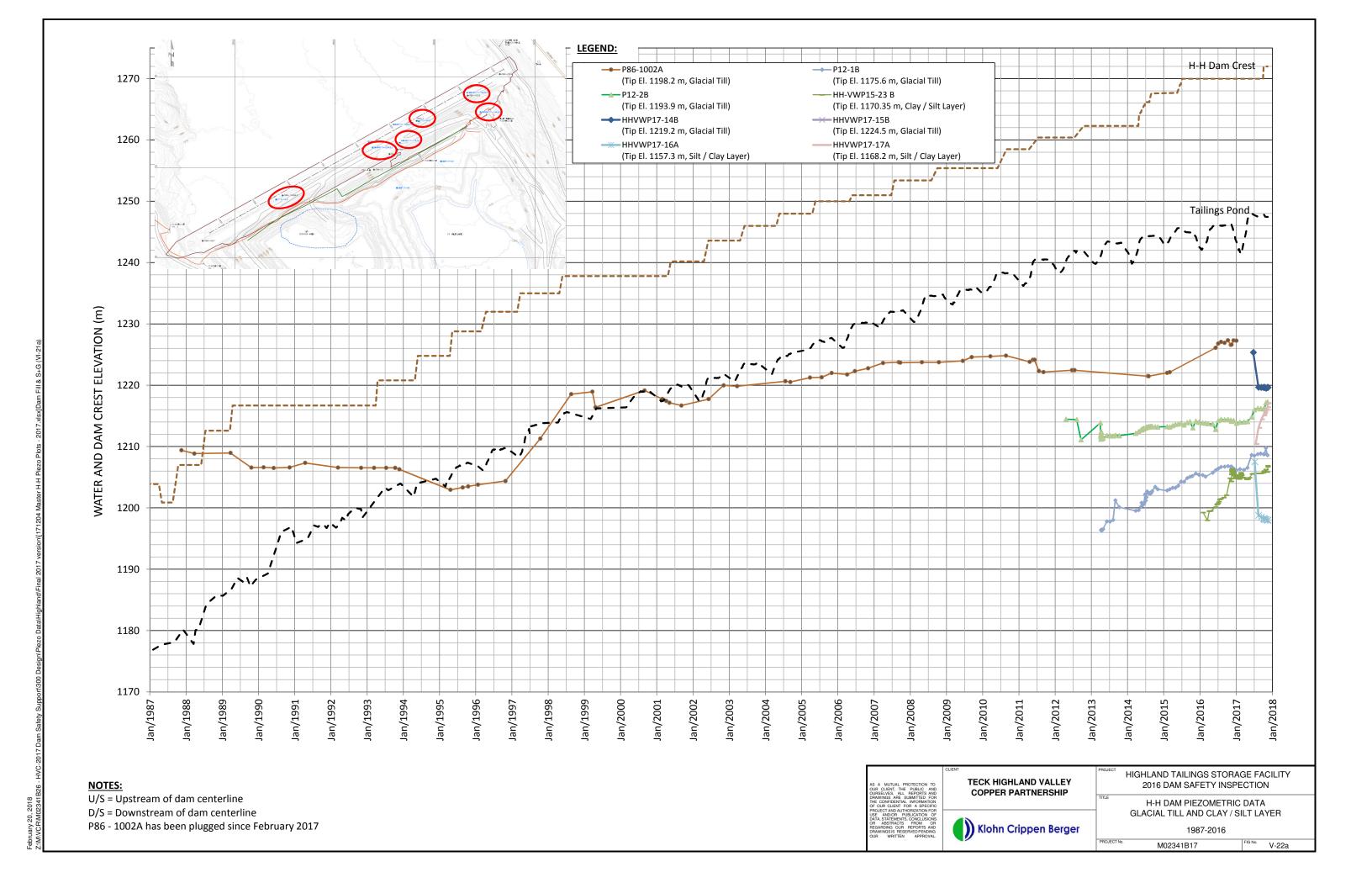


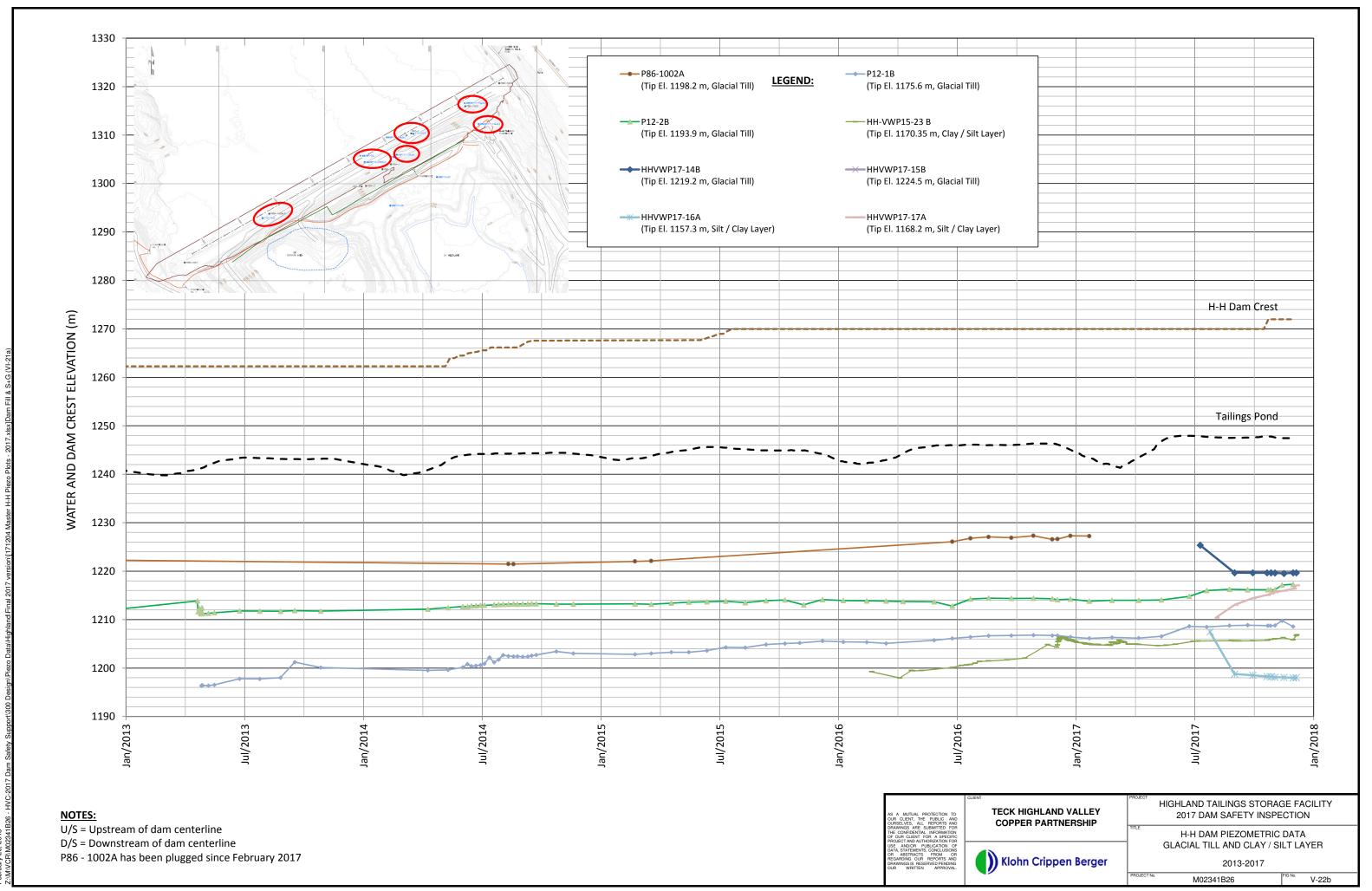




February 20 2018





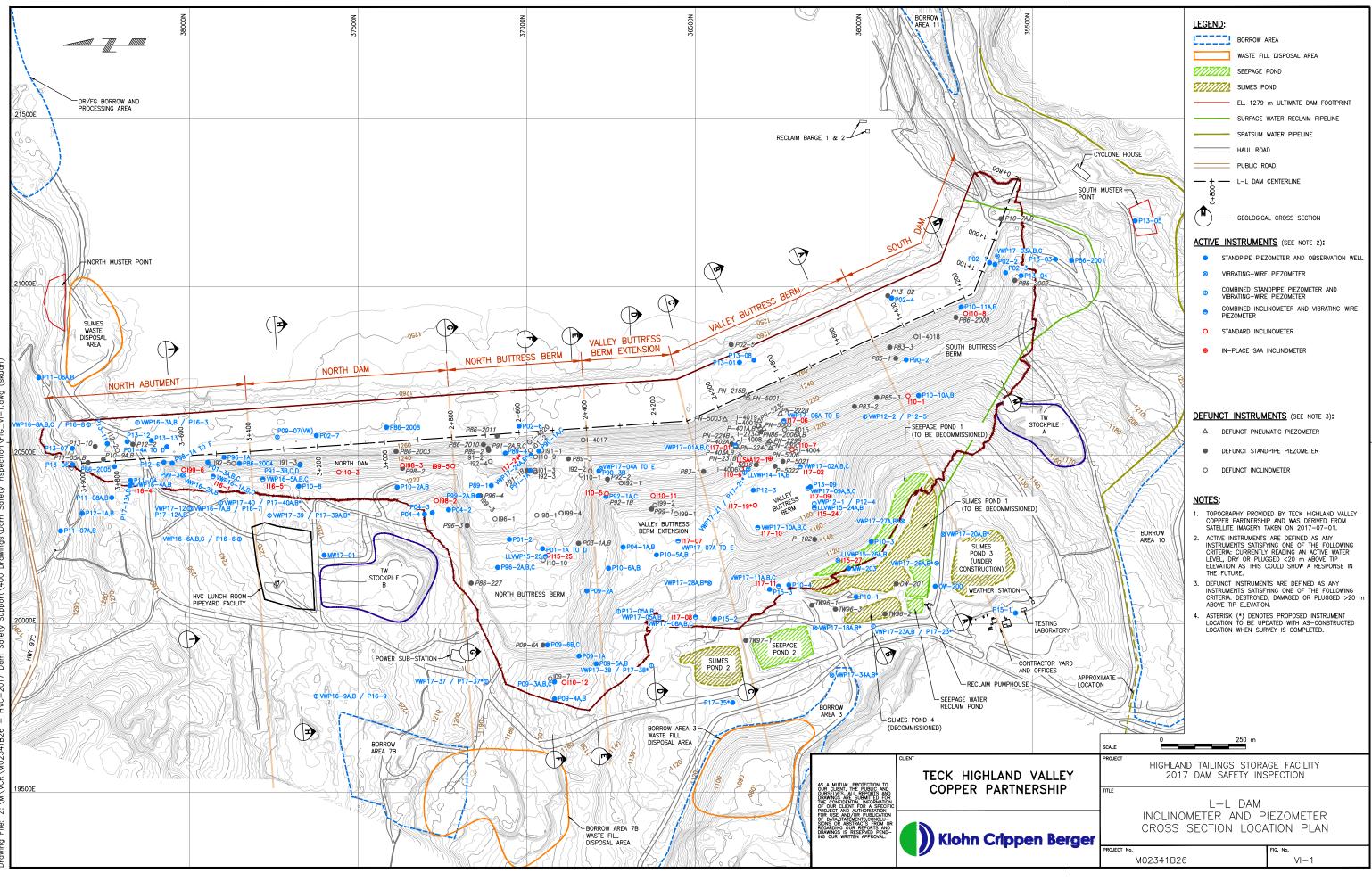


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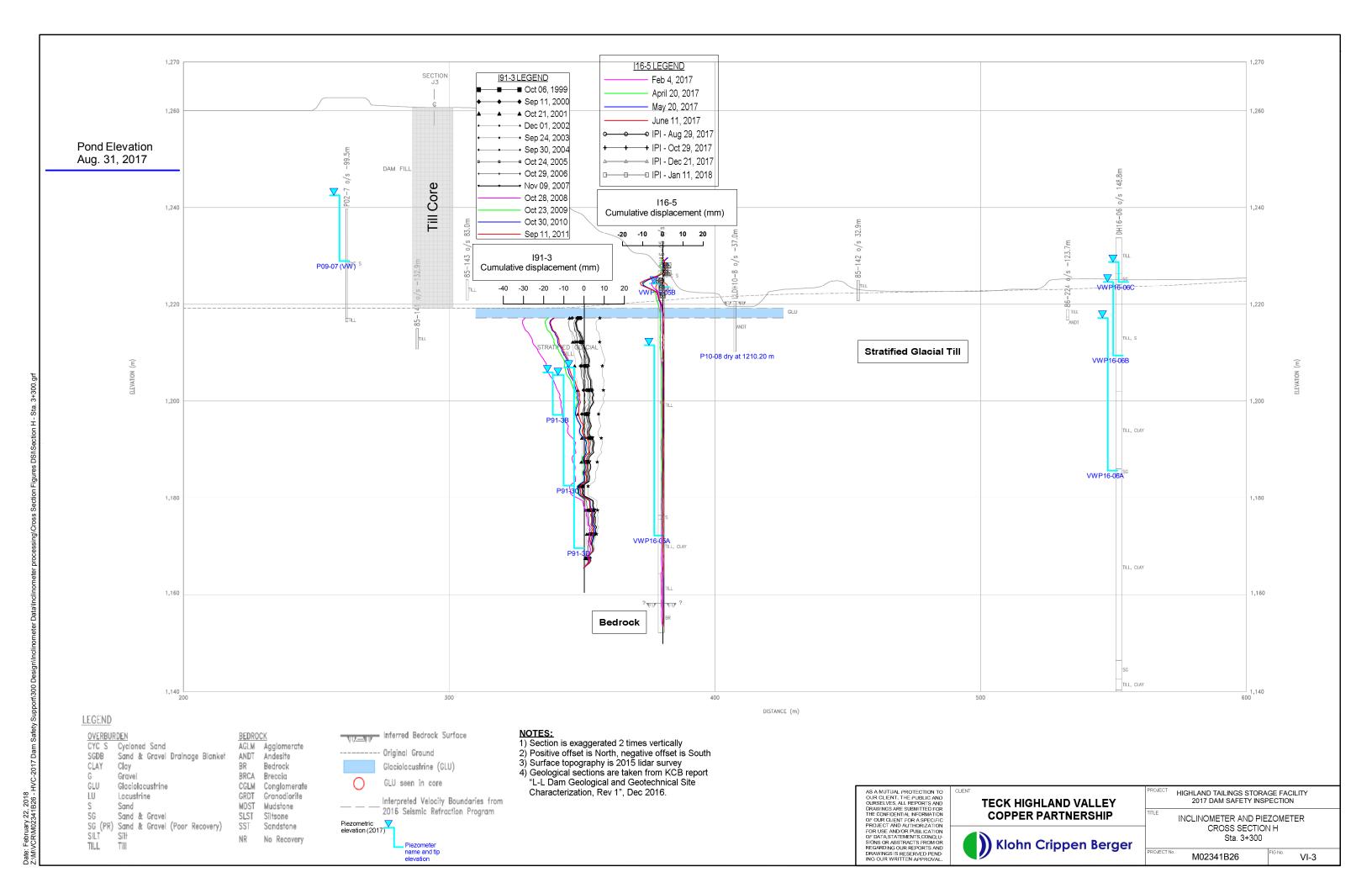
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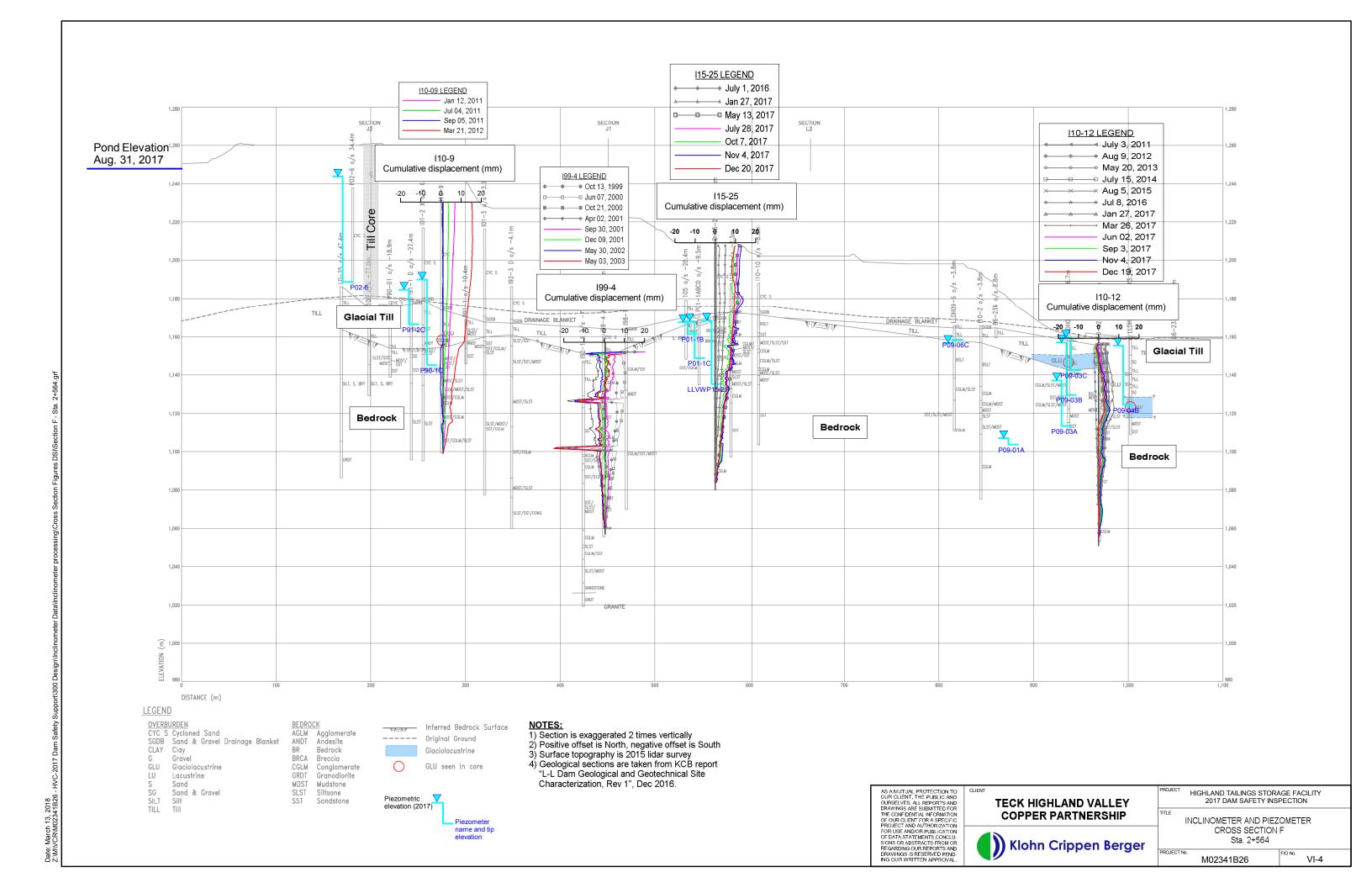
APPENDIX VI

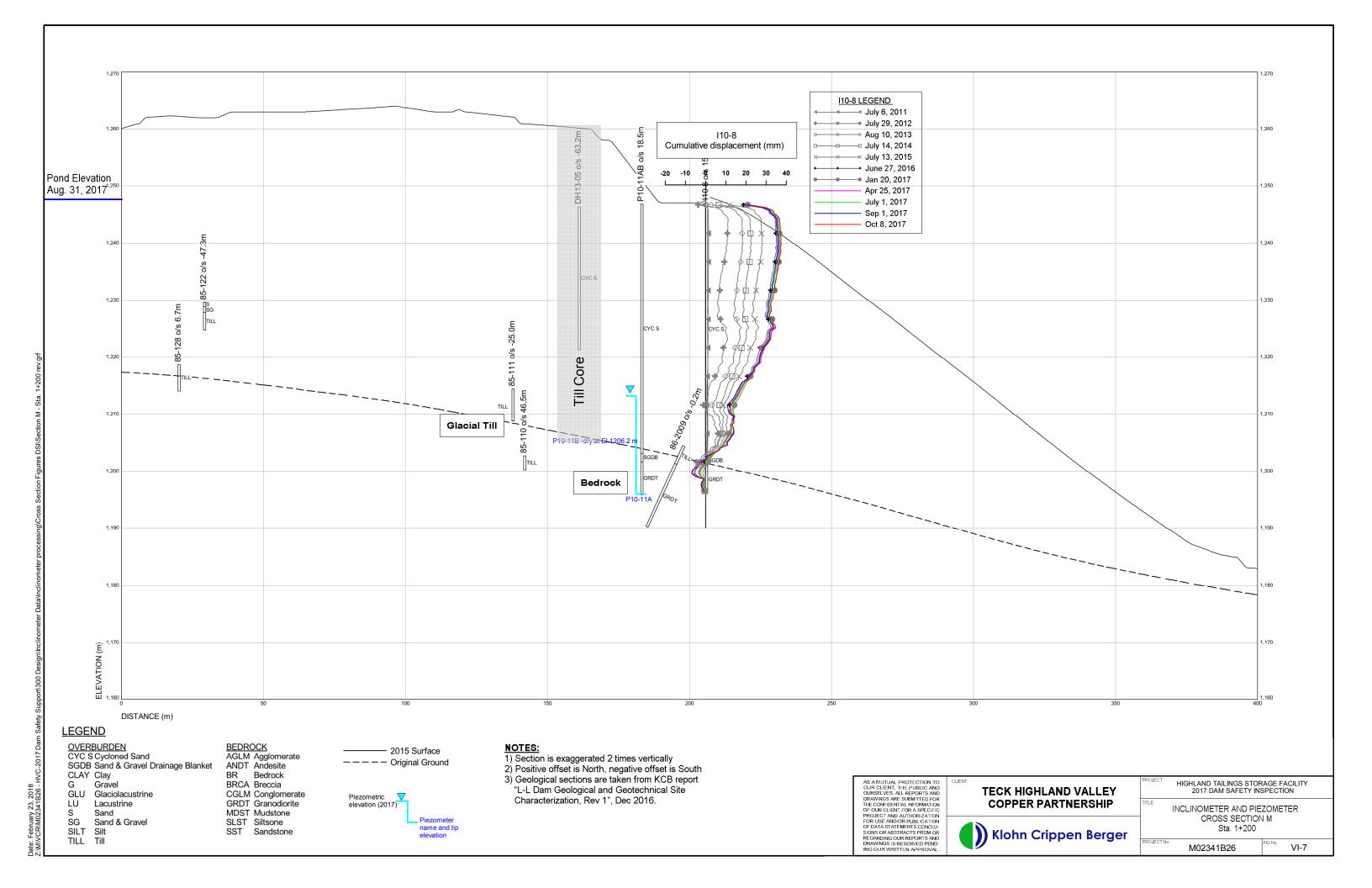
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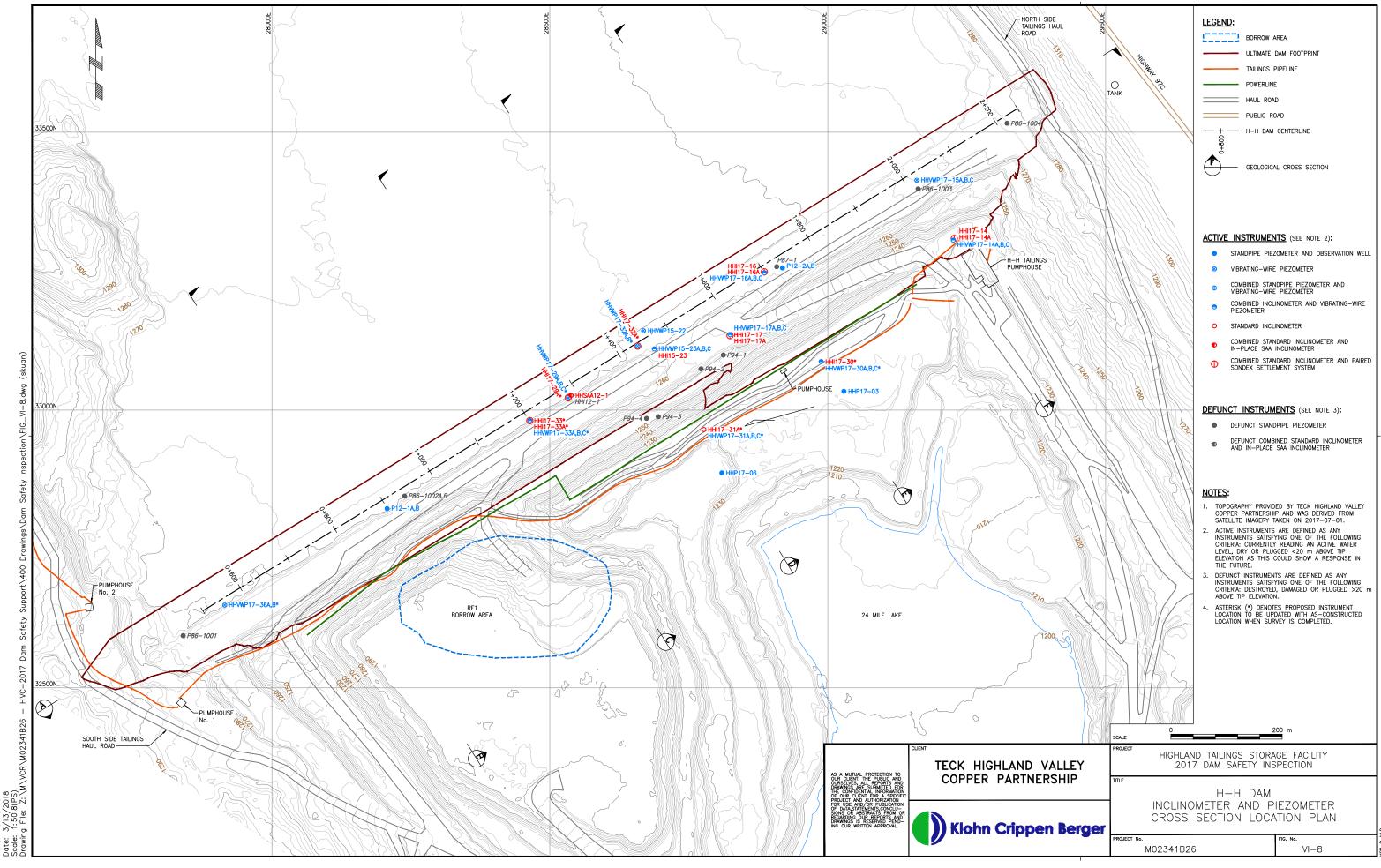


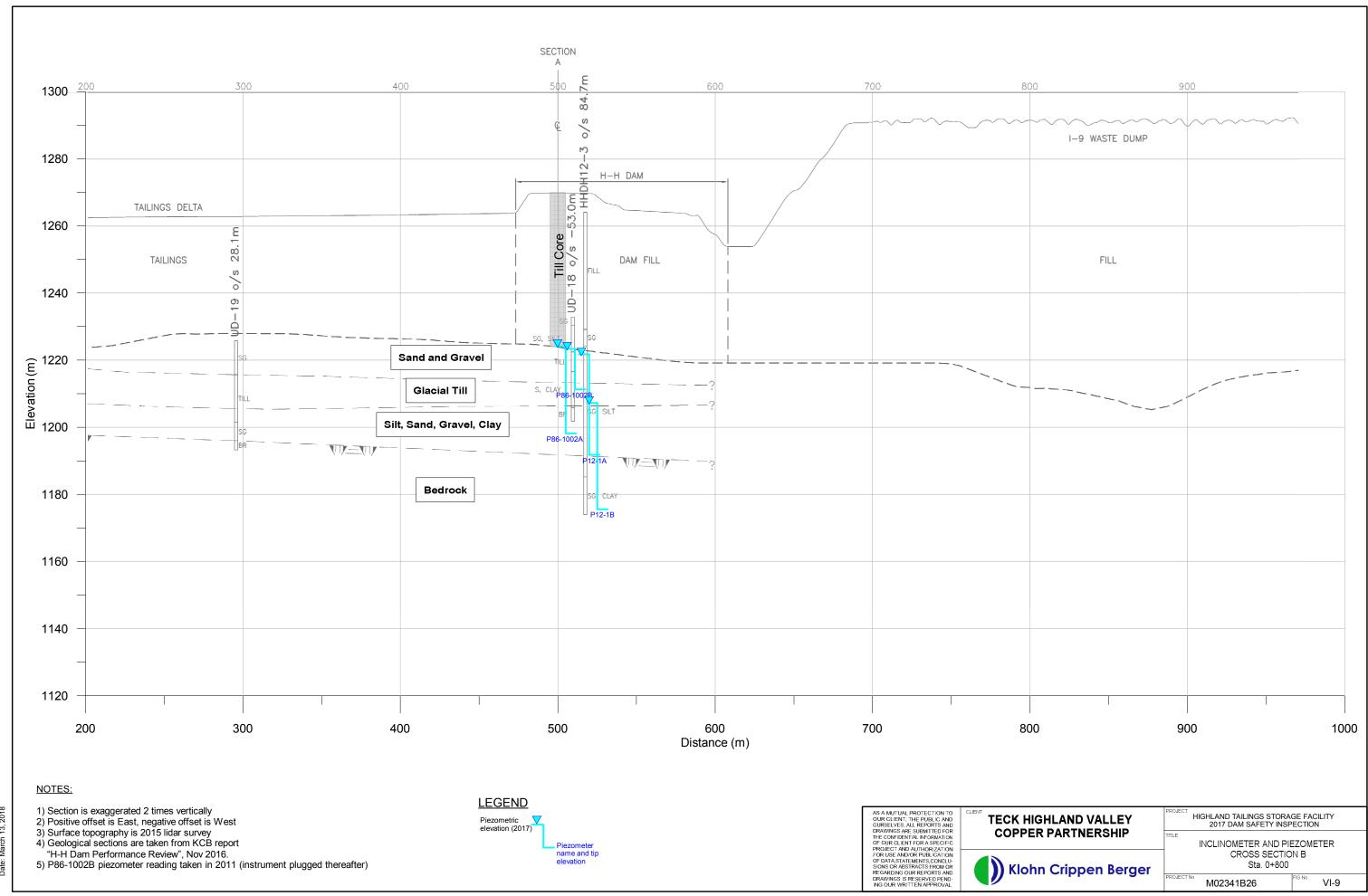
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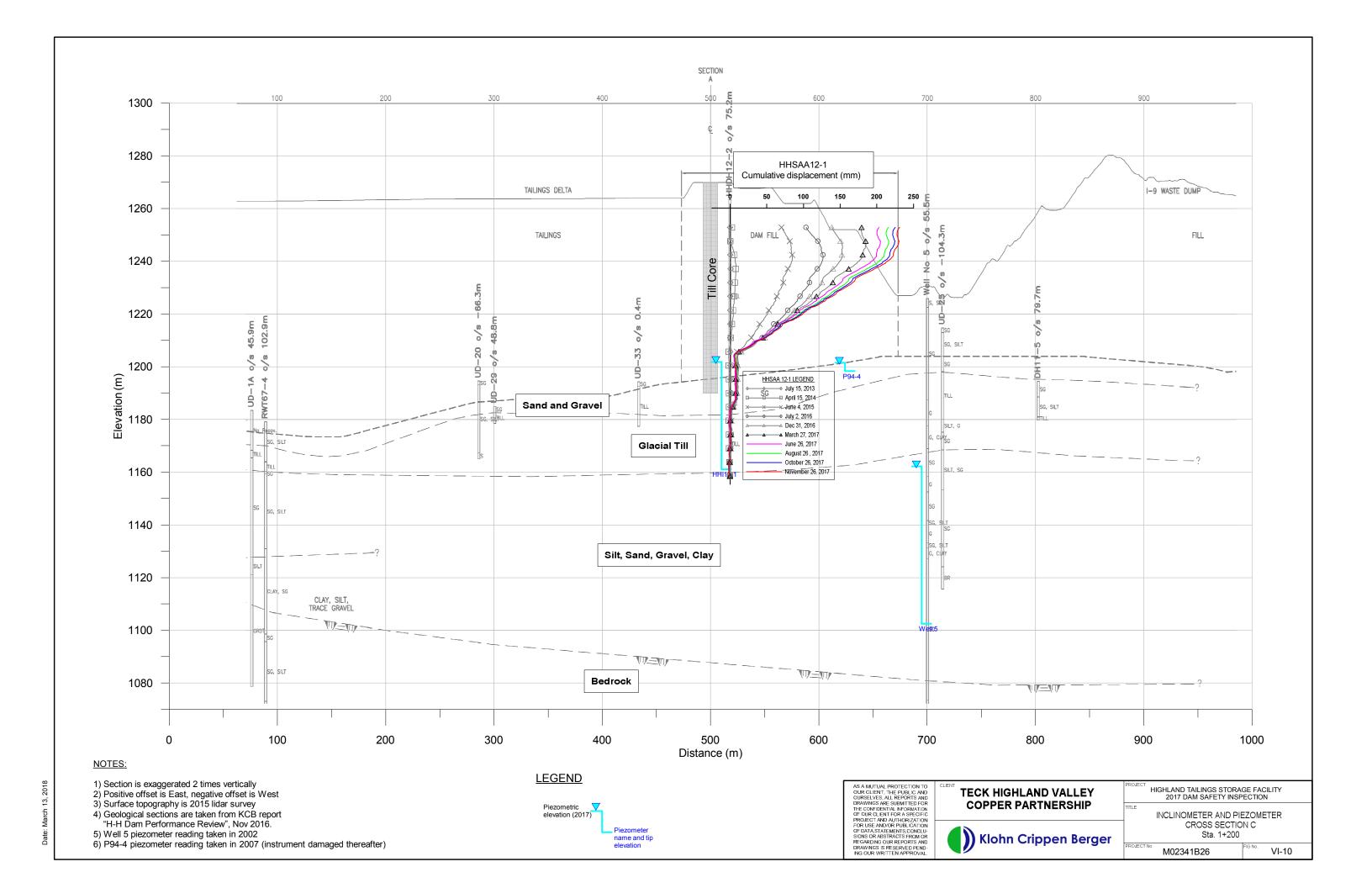


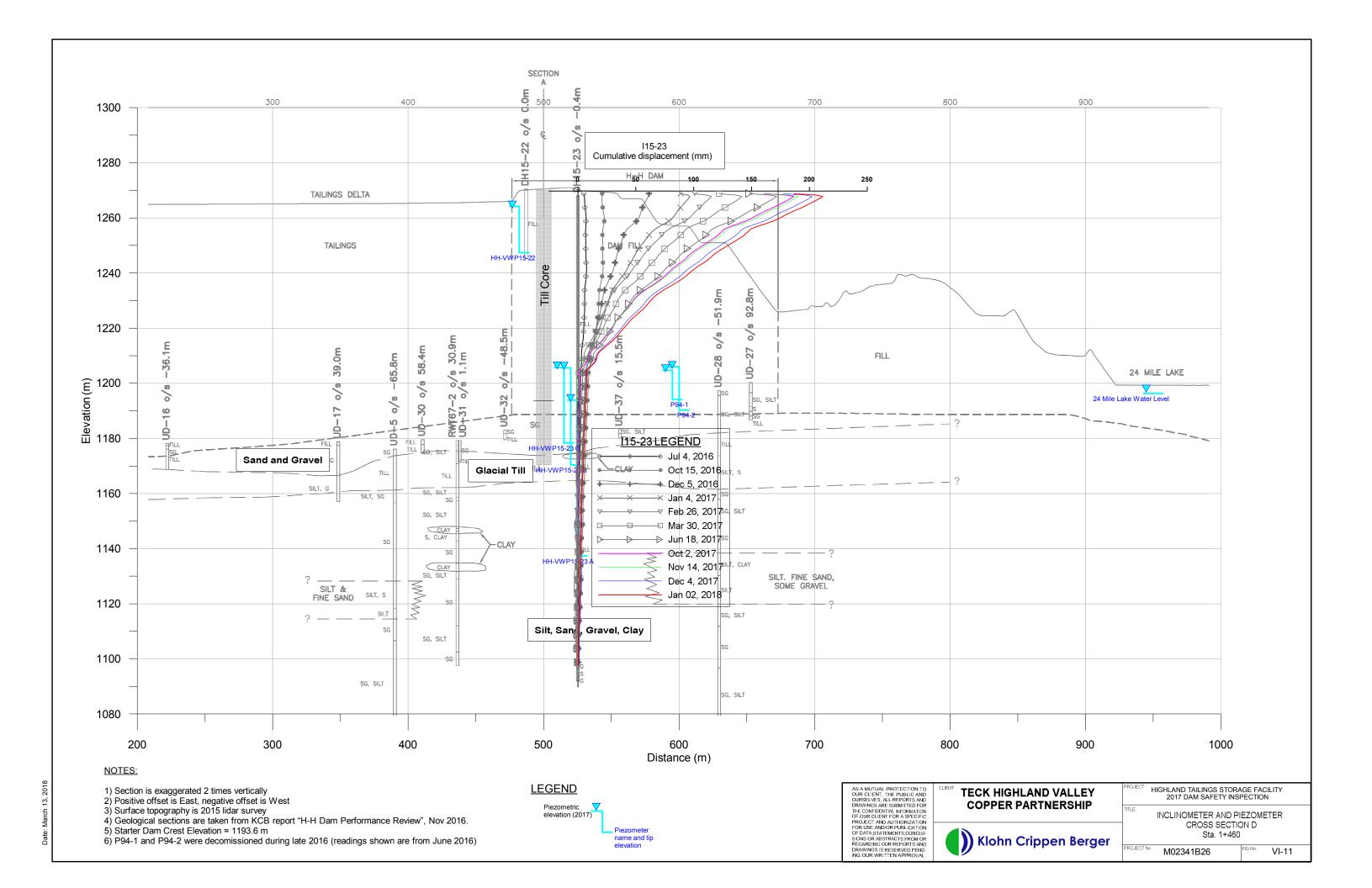


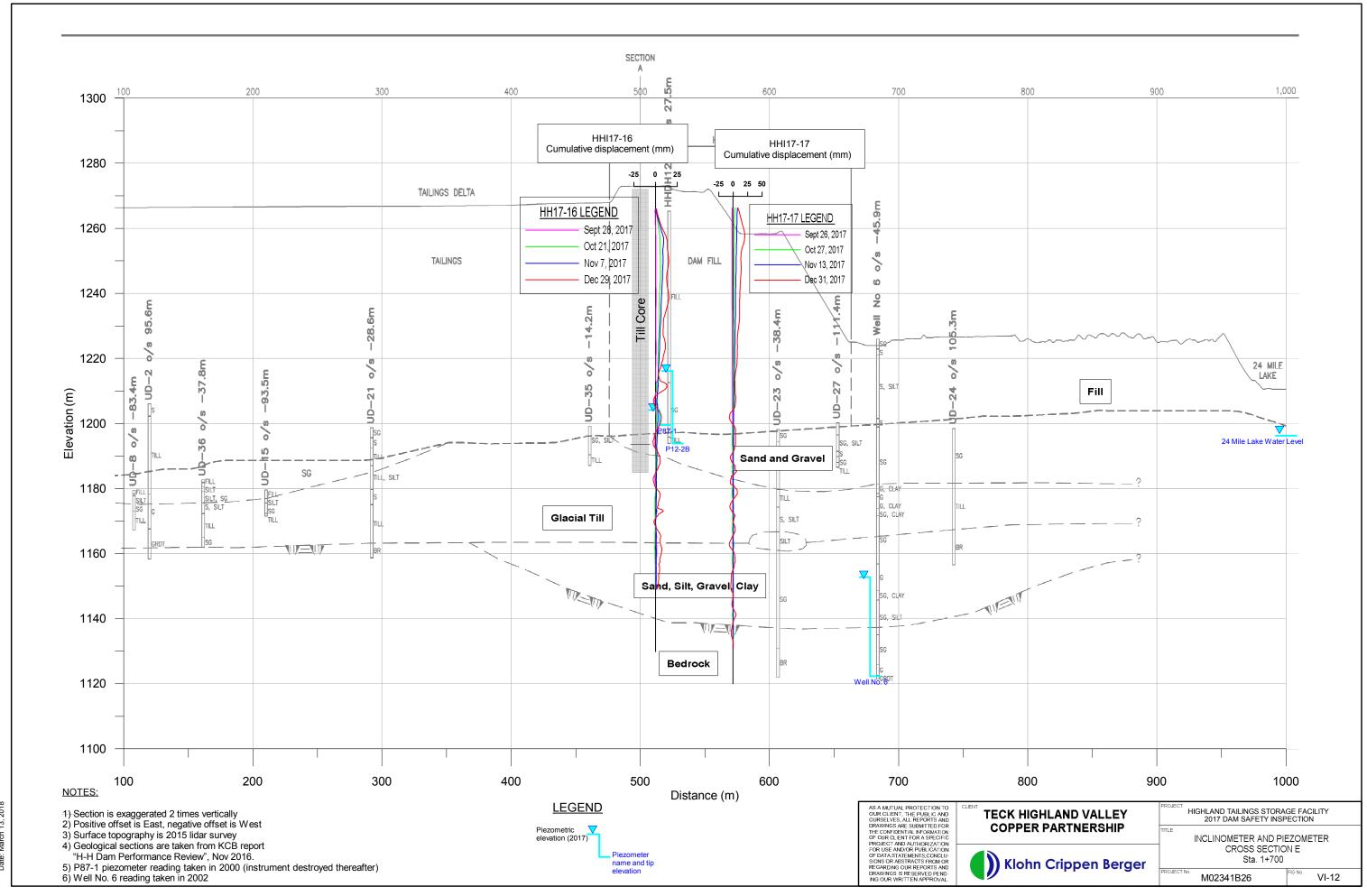


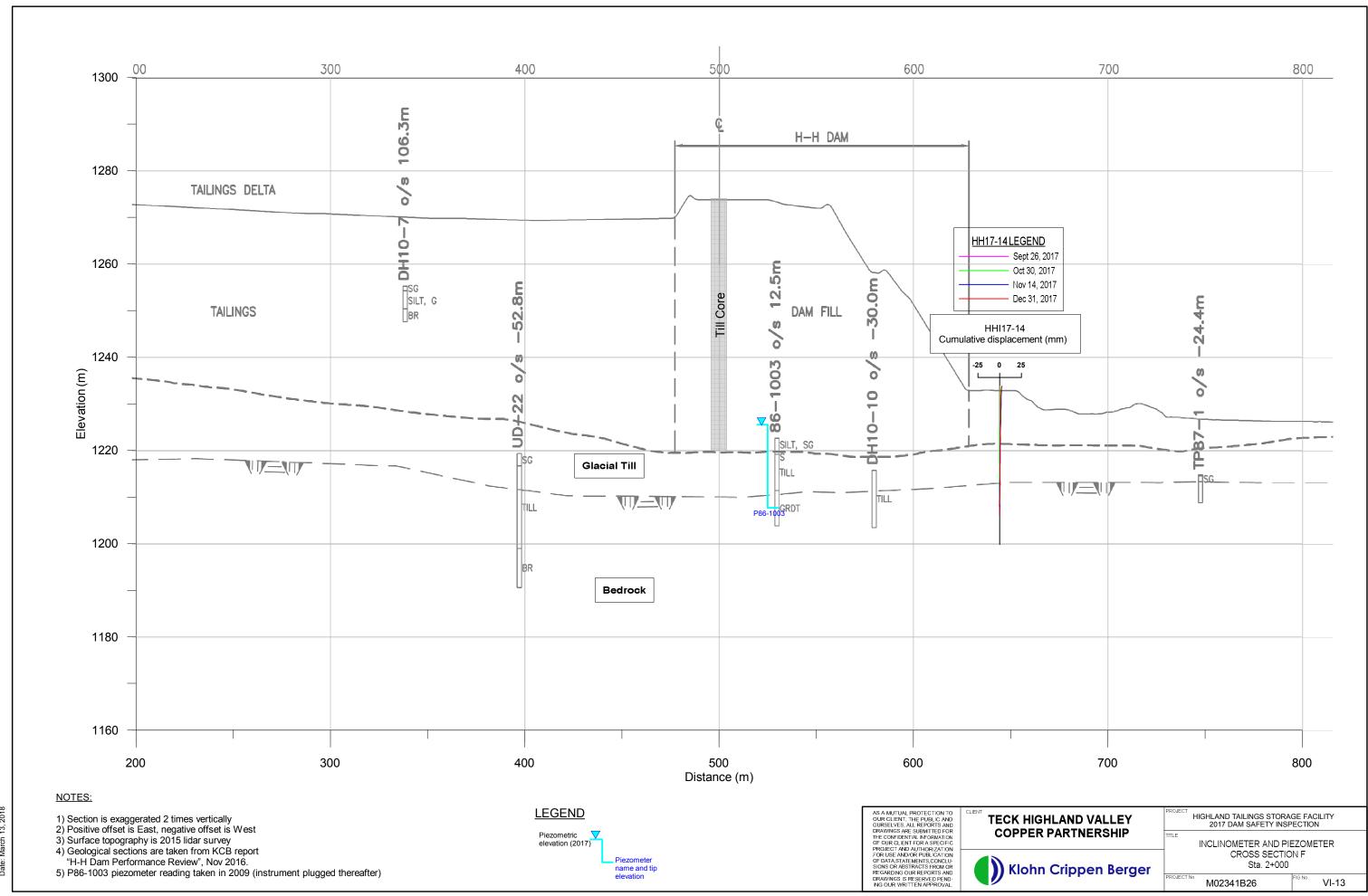






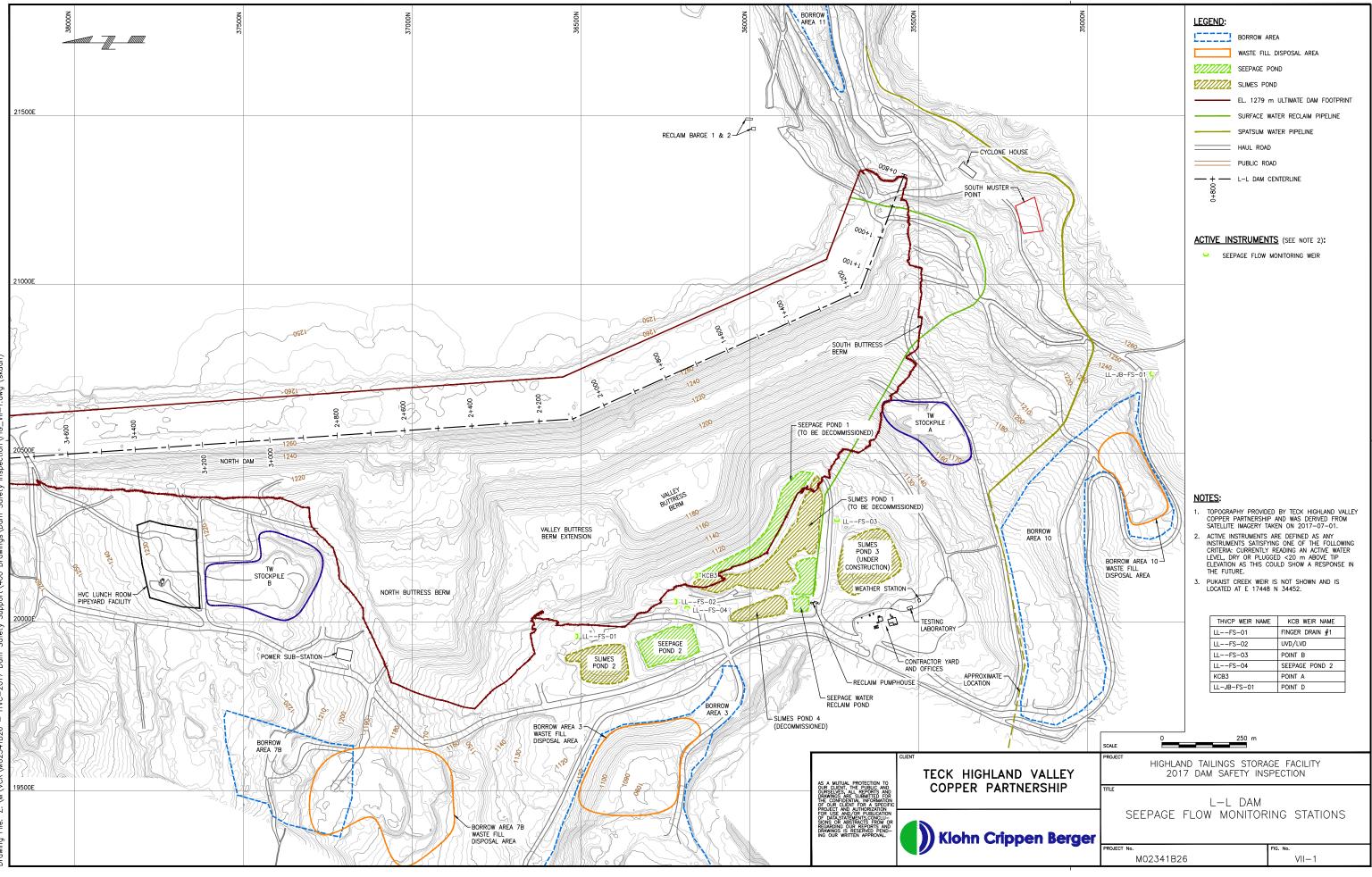




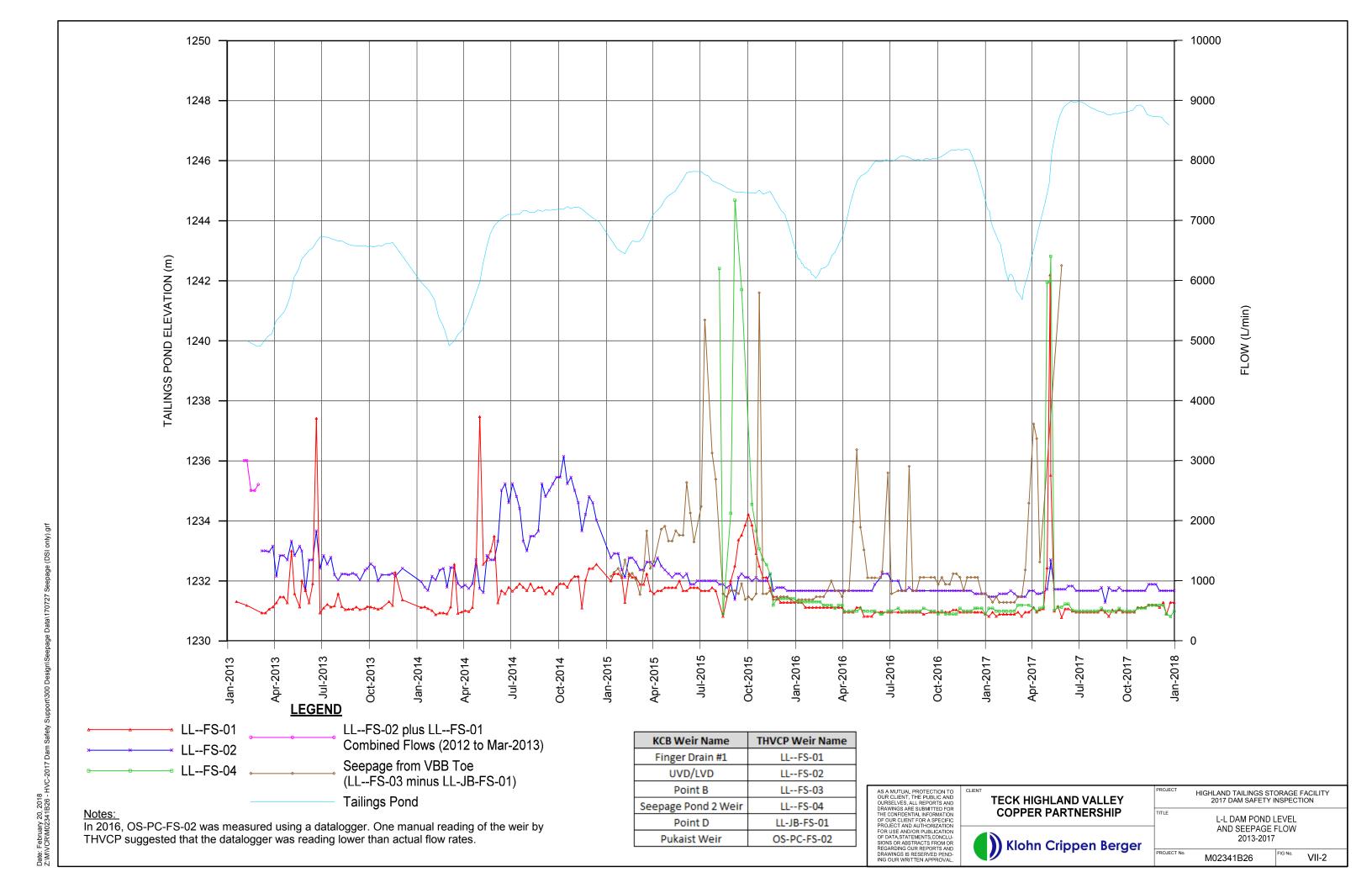


APPENDIX VII

Seepage Flow Data Plots



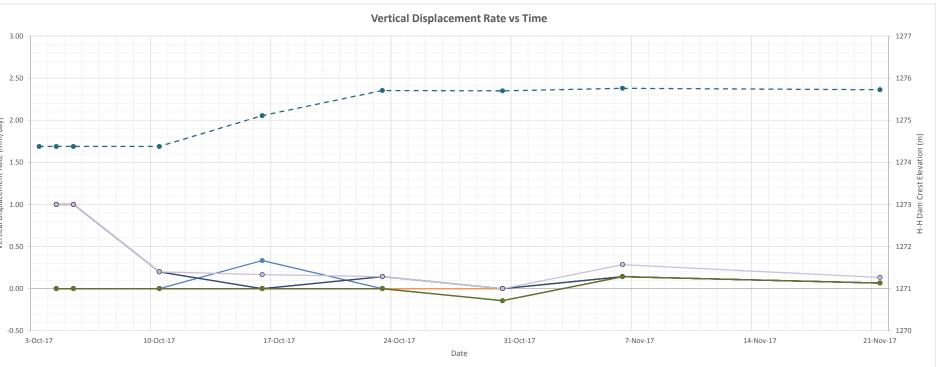
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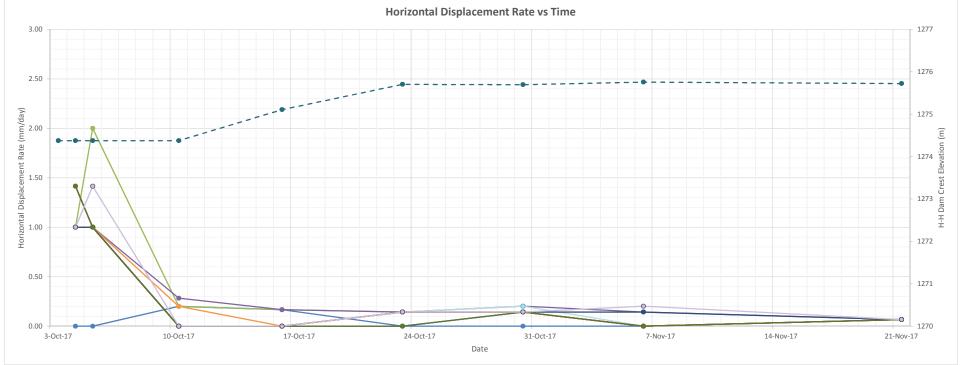
APPENDIX VIII

H-H Lock-Block Wall Monument Data Plots



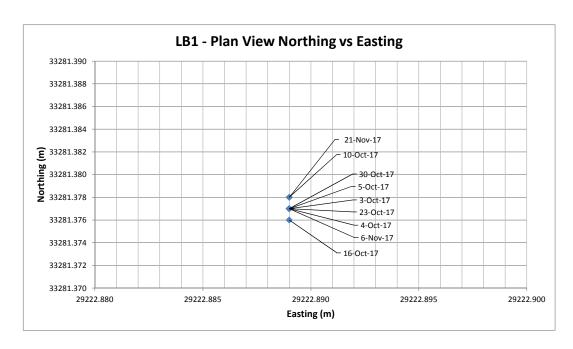


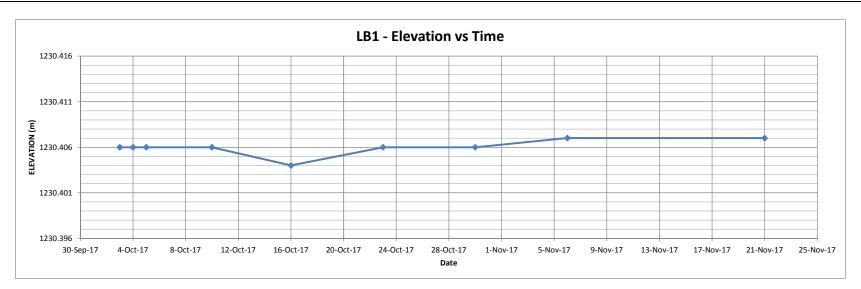


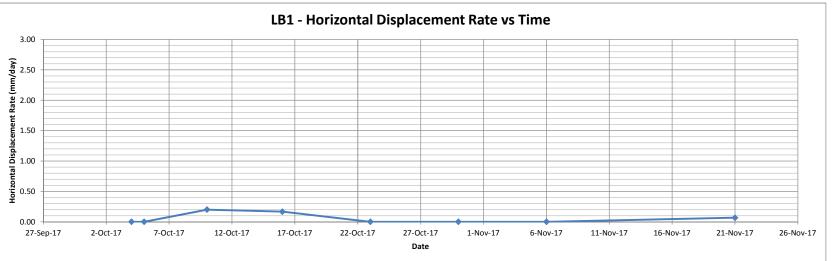


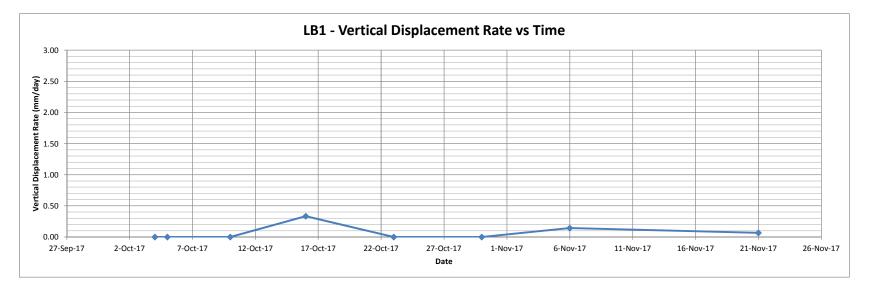






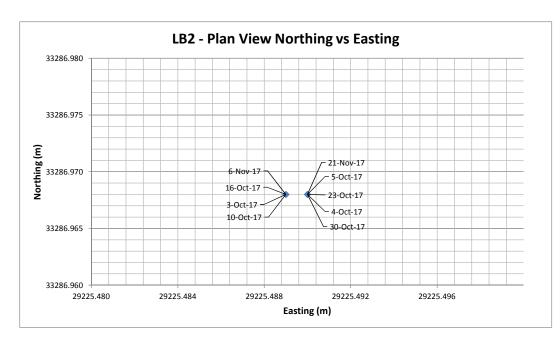


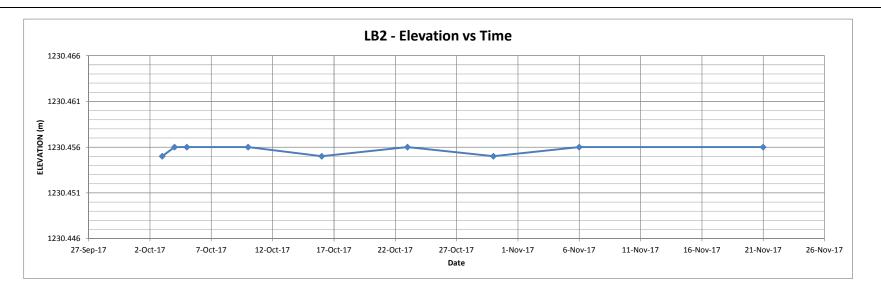


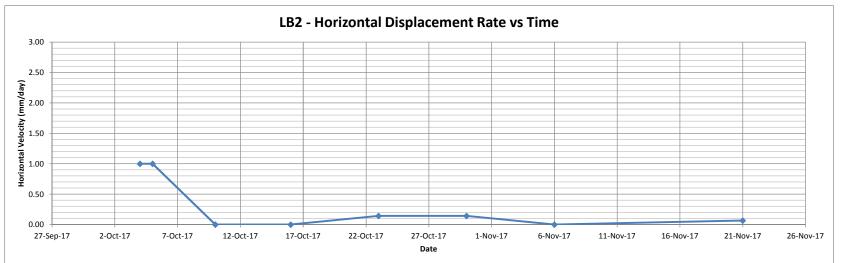


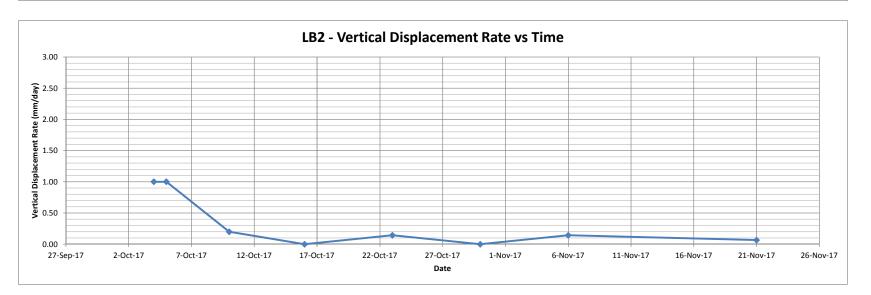












1. All data provided by THVCP.

TECK HIGHLAND VALLEY
COPPER PARTNERSHIP

TECK HIGHLAND VALLEY
COPPER PARTNERSHIP

THE H-H DAM LOCK-BLOCK RETAINING WALL SURVEY MONUMENT READINGS

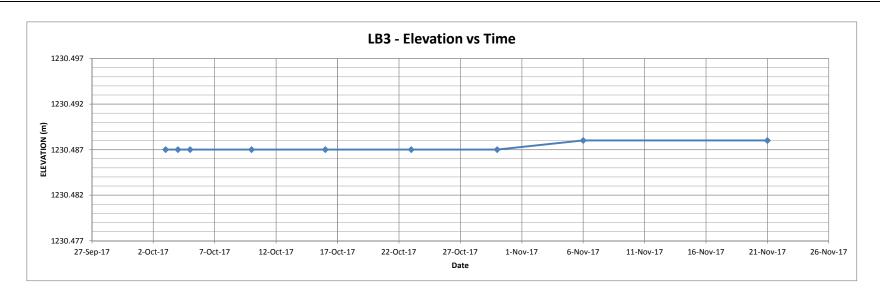
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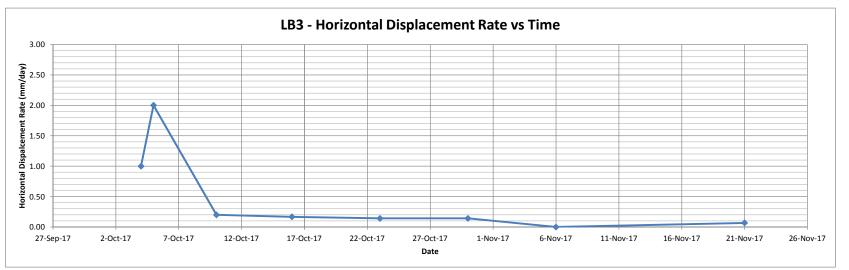
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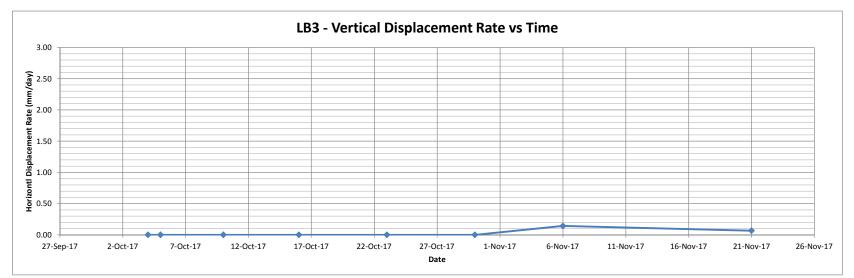
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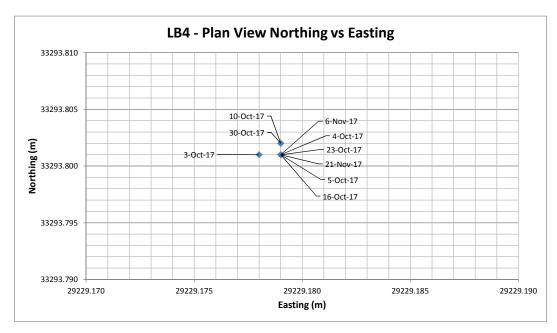


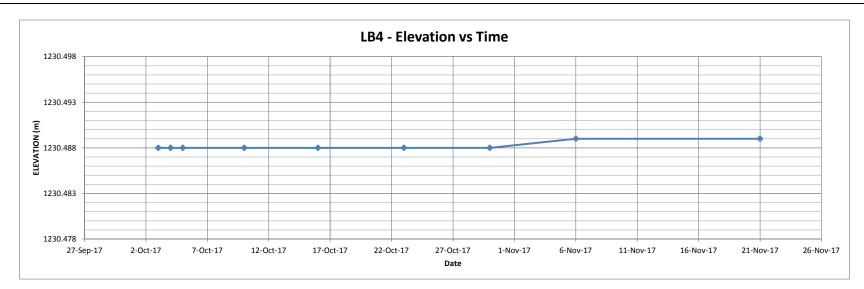


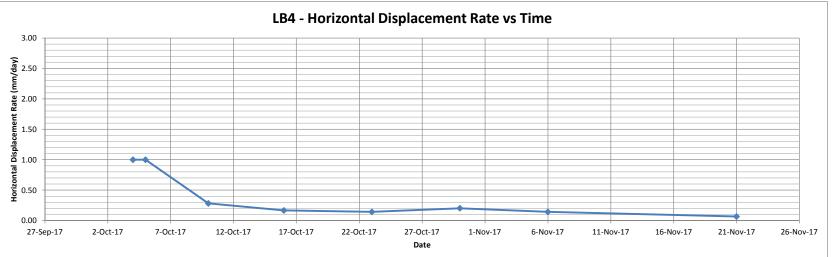


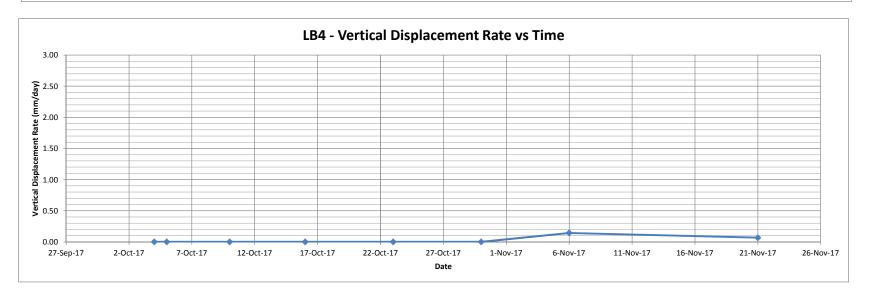










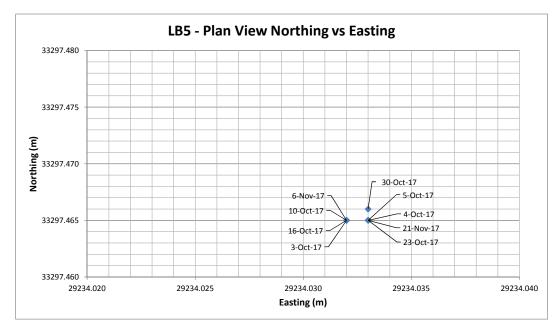


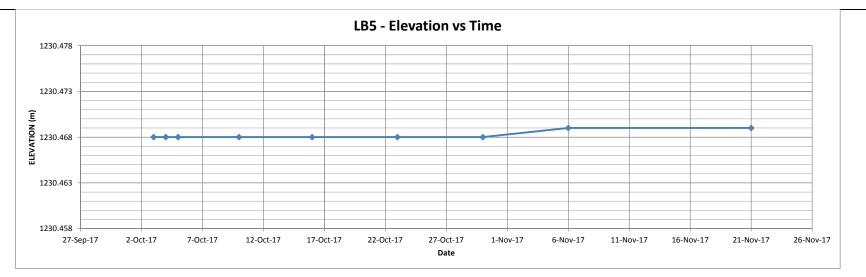
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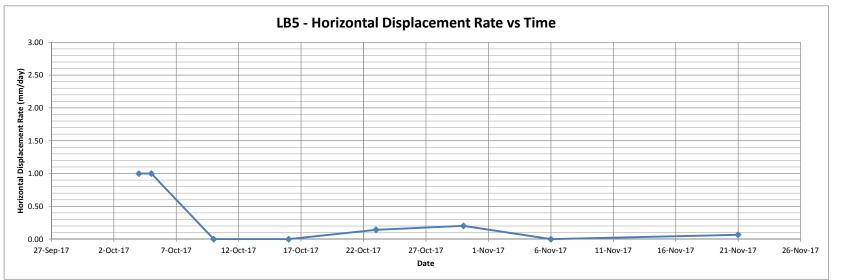
TECK HIGHLAND VALLEY
COPPER PARTNERSHIP

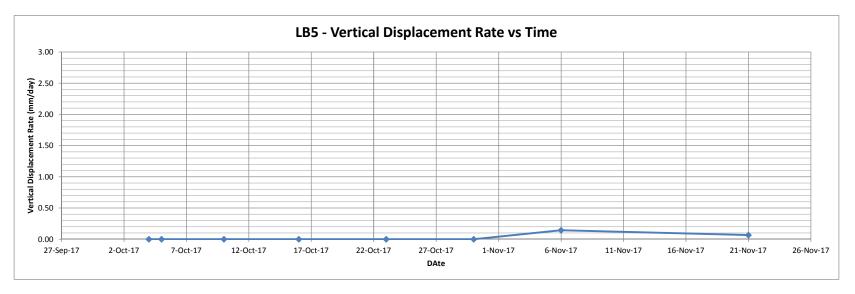
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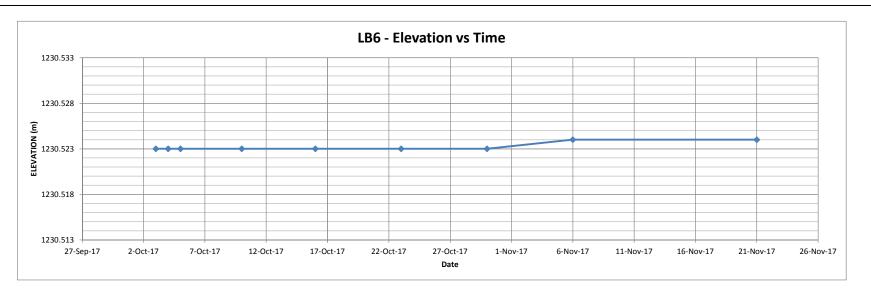


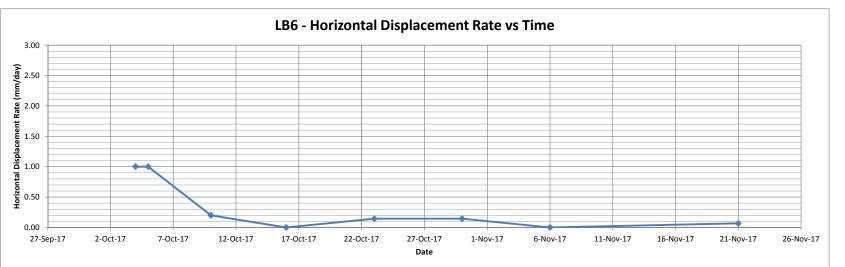


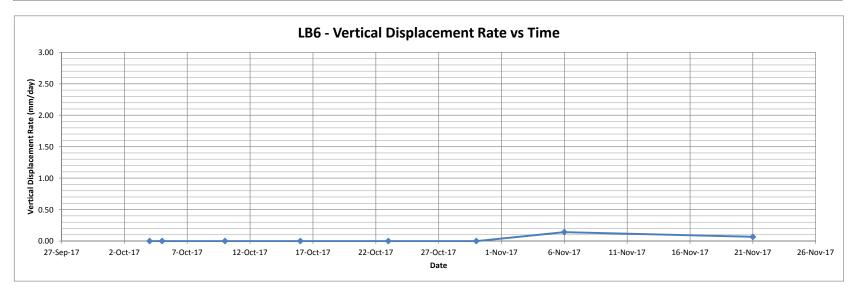












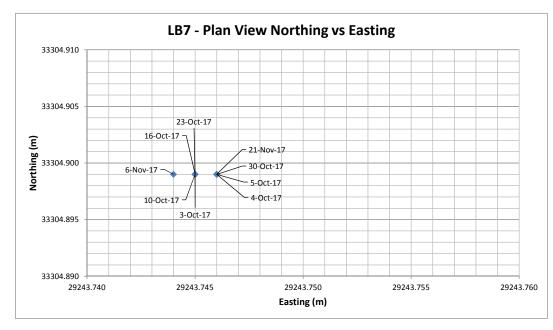
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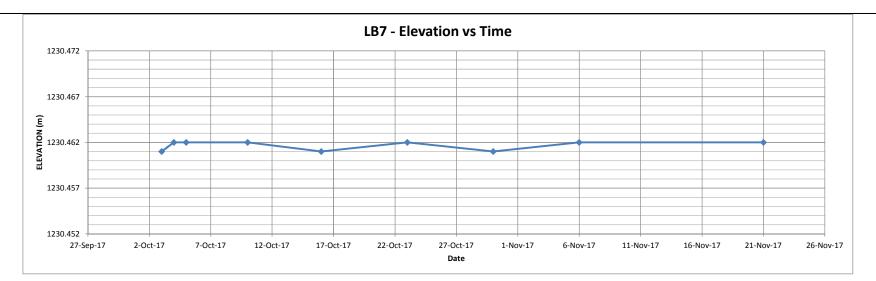
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COPPER PARTNERSHIP

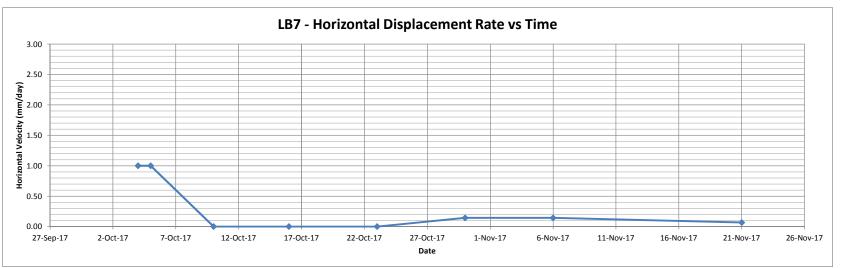
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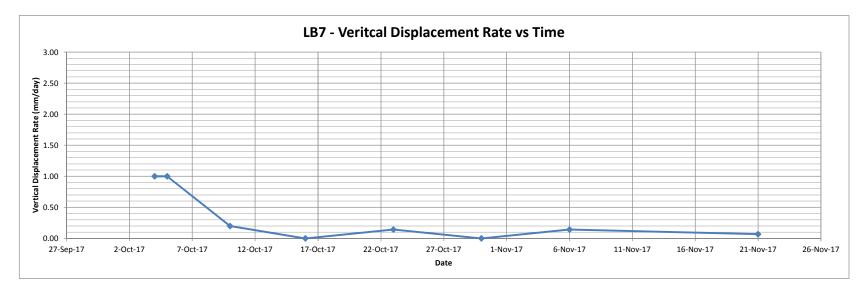
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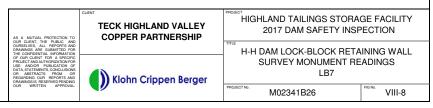






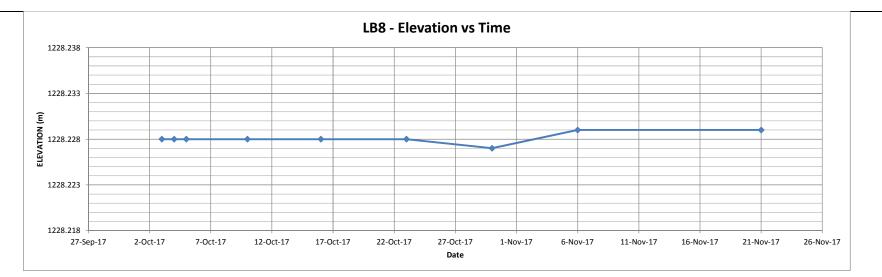


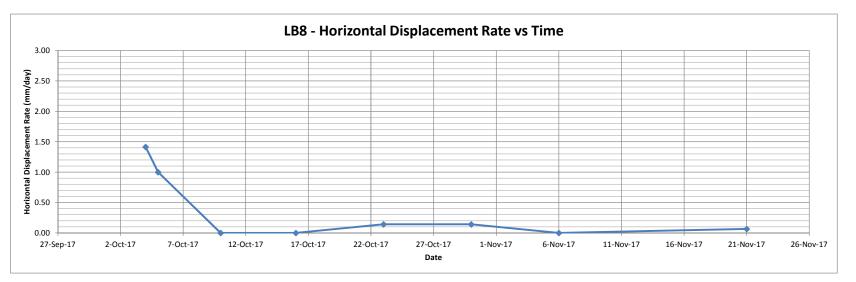


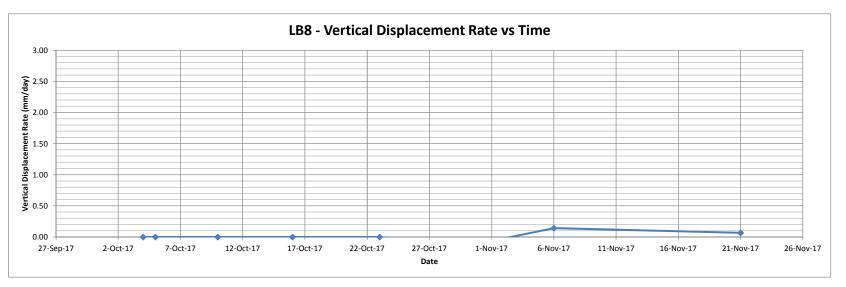


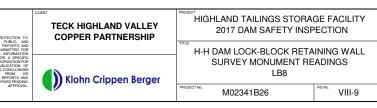




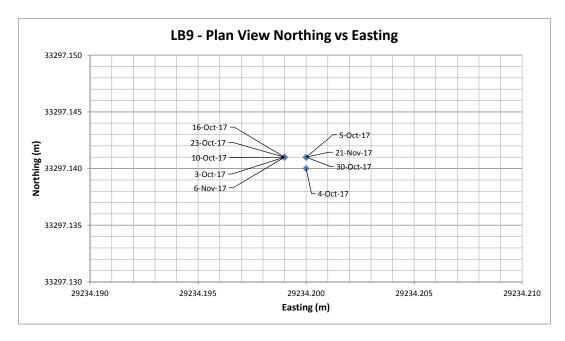


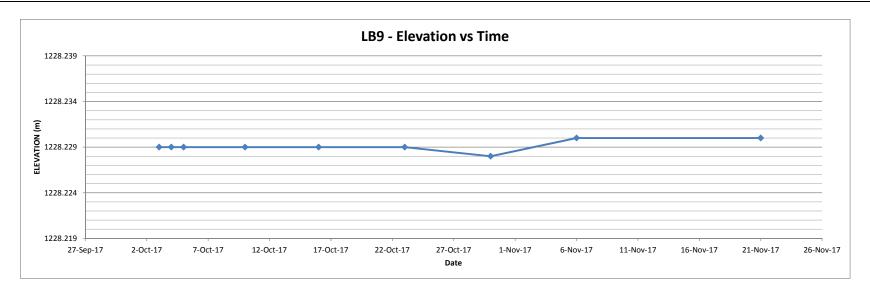


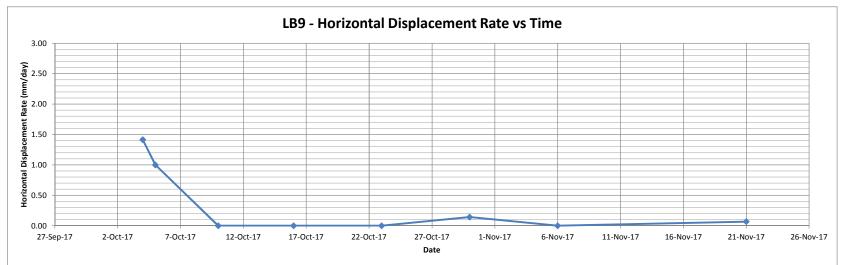


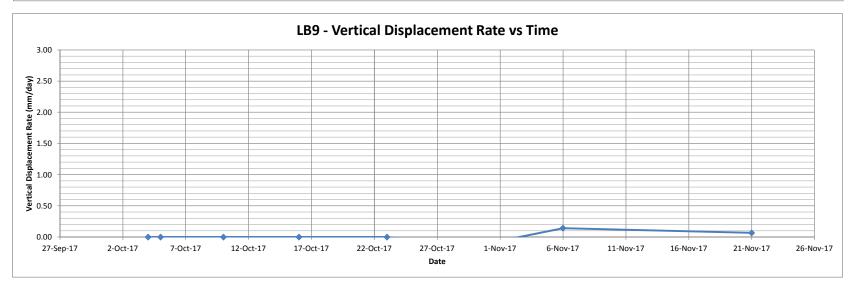








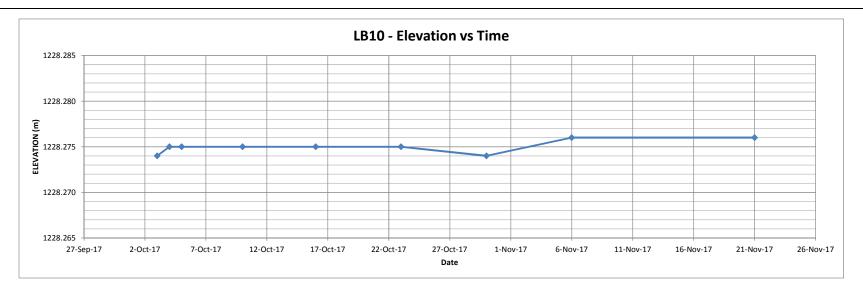


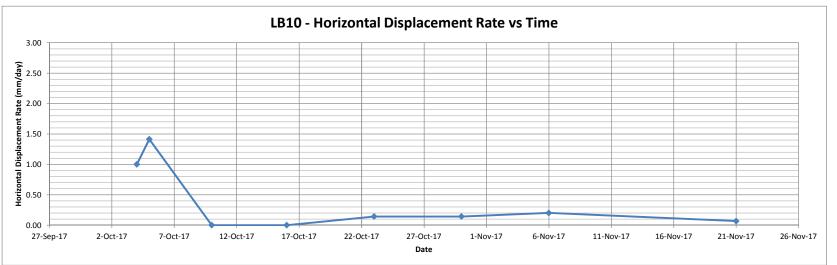


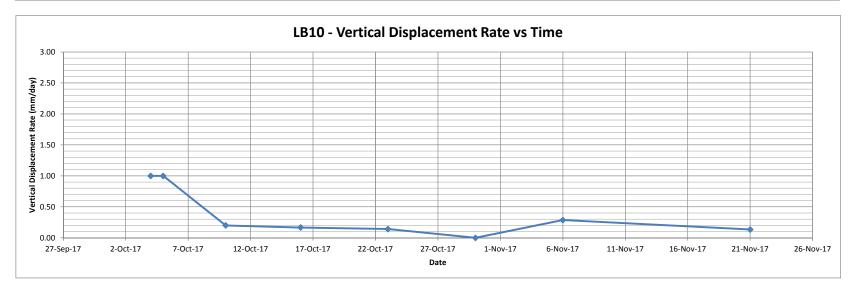


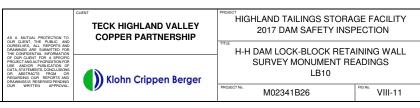












APPENDIX IX

Map of Water Quality Monitoring Points

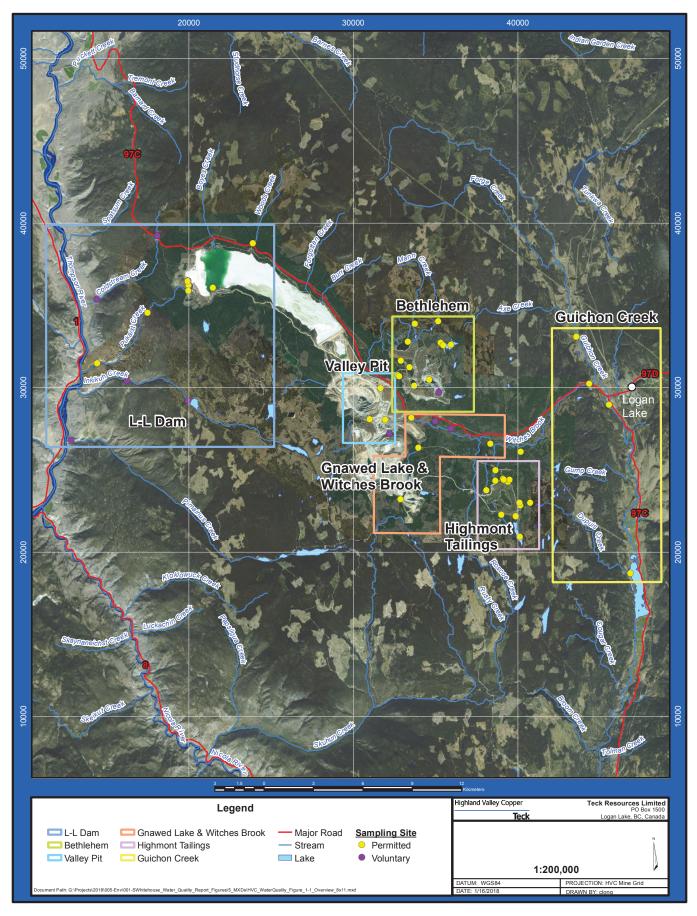


Figure 1-1 Water Quality Monitoring Sites Highland Valley Copper, 2017

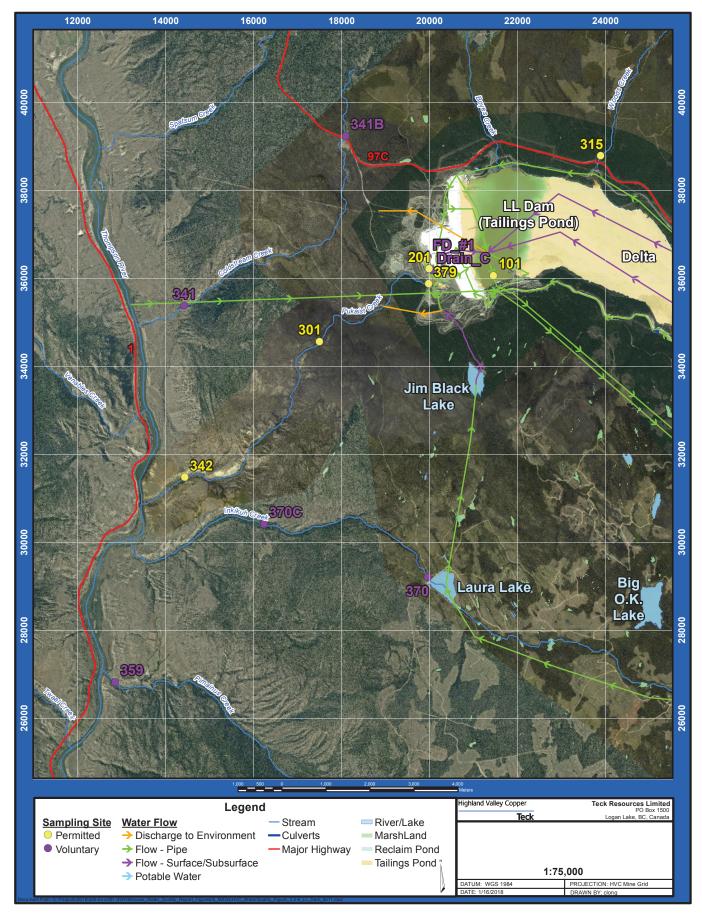


Figure 3.2-5 Water Quality Monitoring Sites in the in the LL Dam/Thompson Valley Area, Highland Valley Copper, 2017